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Lund Sand No 0

part 2

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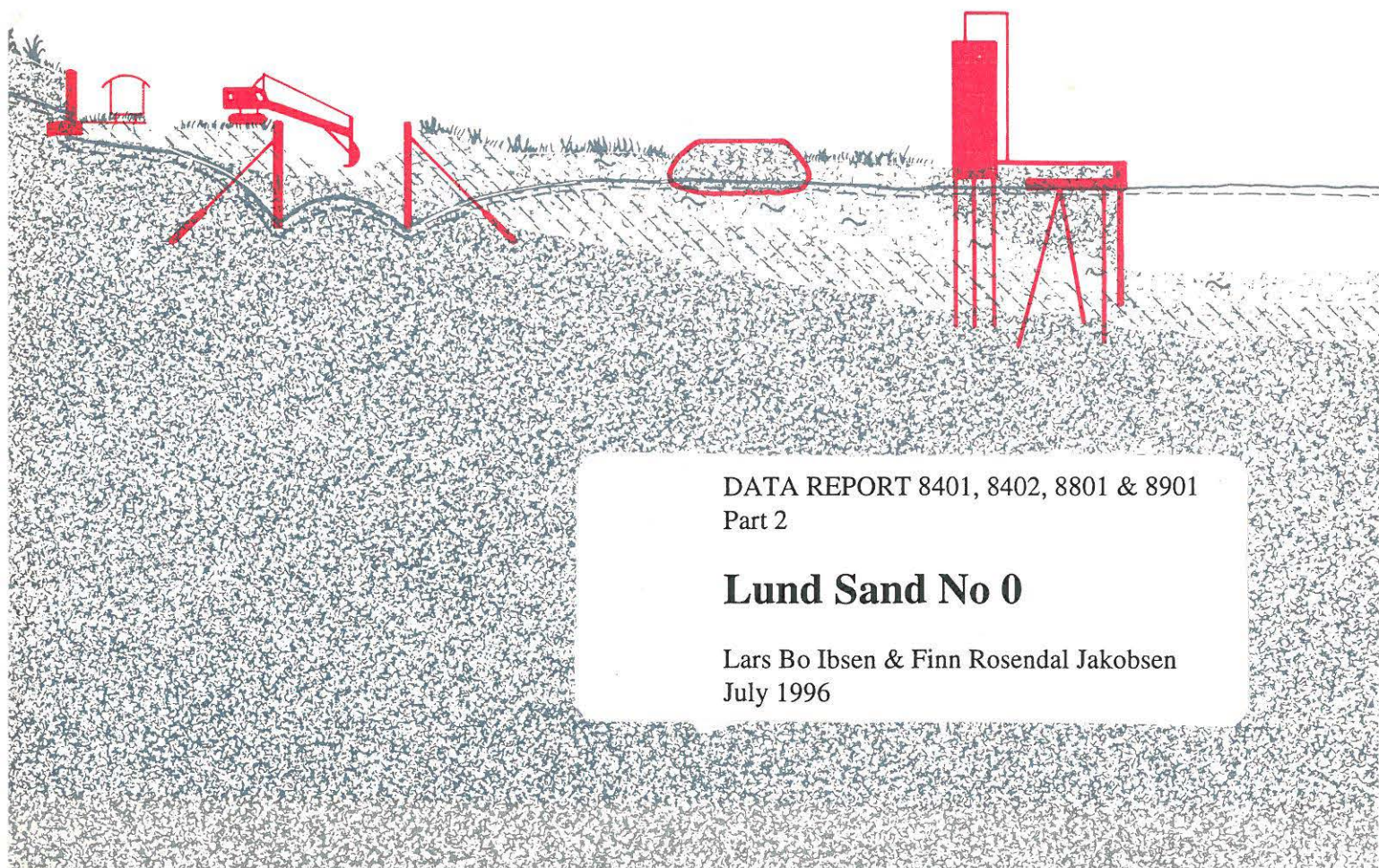
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DATA REPORT 8401, 8402, 8801 & 8901
Part 2

Lund Sand No 0

Lars Bo Ibsen & Finn Rosendal Jakobsen
July 1996

Aalborg University
Geotechnical Engineering Group
Sohngaardsholmsvej 57
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DATA REPORT 8401, 8402, 8801 & 8901
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List of symbols.

Latin letters

d	= diameter of grain
d_{10}	= 10% fractile
d_{50}	= 50% fractile
d_{60}	= 60% fractile
d_s	= grain density
e	= void ratio
e_0	= void ratio before test
e_f	= void ratio at failure
e_{max}	= maximum void ratio
e_{min}	= minimum void ratio
I_D	= density index
p	= mean stress = $1/3(\sigma_1 + 2\sigma_3)$
q	= deviatoric stress = $\sigma_1 - \sigma_3$
S_w	= degree of saturation
σ'	= effective stress

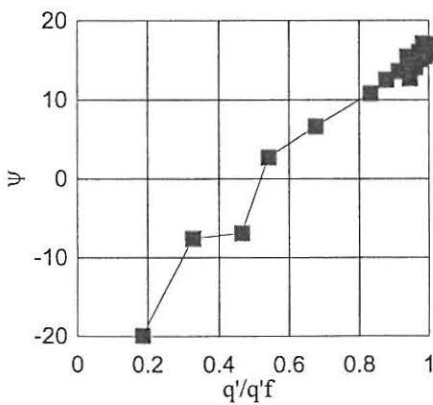
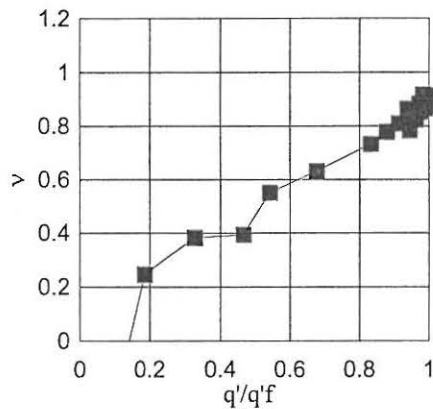
Greek letters

ϵ	= strain
ϵ_1	= vertical strain
ϵ_v	= volumetric strain = $\epsilon_1 + 2\epsilon_3$
ϵ_q	= shear strain = $2/3(\epsilon_1 - \epsilon_3)$
σ	= stress
σ_1	= vertical stress
σ_3	= confining pressure
ν	= Poisson's ratio = $\frac{\Delta\epsilon_1 - \Delta\epsilon_v}{2\Delta\epsilon_1}$
ψ	= angle of dilatation = $\sin^{-1}\left(\frac{\Delta\epsilon_v}{\Delta\epsilon_v - 2\Delta\epsilon_1}\right)$
σ'	= effective stress

Description of soil Lund No 0		Water content %	Before test	At failure
Calibration file kal_tri2	Date 21.09.88	Grain density	2.65	
		Void ratio	0.615	0.665
		Saturation		
		Dimension H mm D mm	71.5 69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-40 kPa
		ϵ_l	-0.062 %
		ϵ_v	-0.152 %
2. Drained compression.			
Deformation rate:		7.9 % ph	

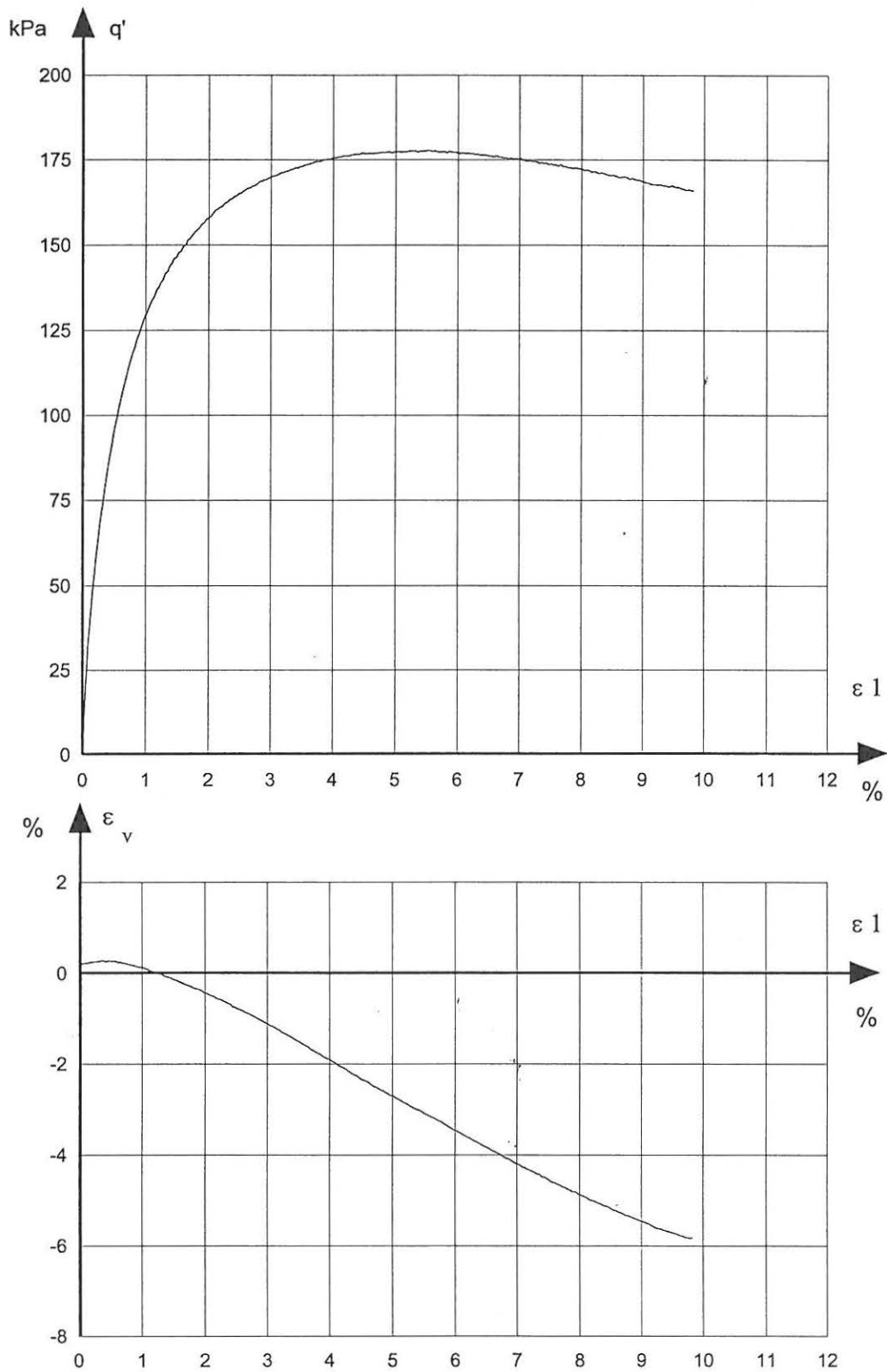
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	177.80	kPa	82.74	kPa
Mean normal stress	p'	100.10	kPa	68.92	kPa
Confining pressures	σ_3	40.83	kPa	41.34	kPa
Vertical strain	ϵ_l	5.54	%	0.37	%
Volumetric strain	ϵ_v	-3.10	%	0.28	%



q'	p'	ϵ_l	ϵ_v
0.45	39.58	0.00	0.00
6.18	42.51	0.01	0.16
10.56	44.61	0.02	0.18
32.87	51.92	0.08	0.22
58.14	59.45	0.20	0.24
82.74	68.92	0.37	0.28
96.30	71.91	0.49	0.27
120.29	82.46	0.82	0.18
147.93	89.25	1.52	-0.14
156.07	93.49	1.88	-0.34
162.30	93.91	2.25	-0.57
166.76	95.53	2.63	-0.84
170.15	97.04	3.00	-1.11
172.72	97.77	3.39	-1.40
174.62	99.55	3.77	-1.72
176.22	98.04	4.16	-2.04
176.93	98.28	4.54	-2.36
177.80	100.10	5.54	-3.10
177.04	100.23	6.08	-3.51
175.59	97.83	6.85	-4.07
173.24	98.32	7.62	-4.62
170.85	96.64	8.39	-5.11
167.94	97.58	9.16	-5.55
167.71	98.39	9.24	-5.60

Job: Lund No 0	Encl. No 1
Exc: LBI	Check: FRJ

Remark:
Preparation [%] $\Delta\epsilon_l$ 0.152
Error during saturation.

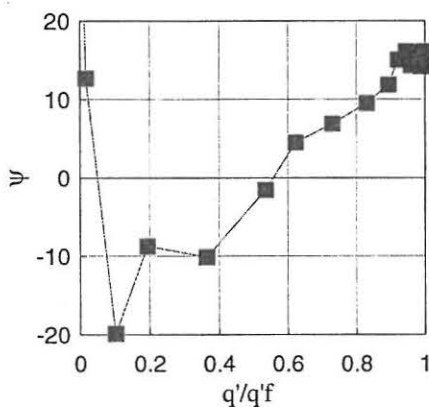
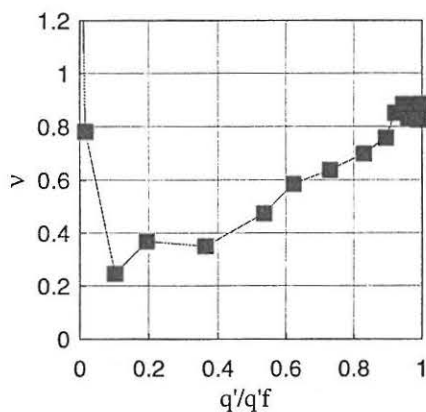


Job:	Encl. No
Lund No 0	2
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	25.3	
Calibration file	Date	Void ratio	0.615	0.667
kal_tri1	21.09.88	Saturation		
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-40 kPa
		ϵ_1	-0.048 %
		ϵ_v	0.000 %
	2. Drained compression.		
	Deformation rate:		7.8 % ph

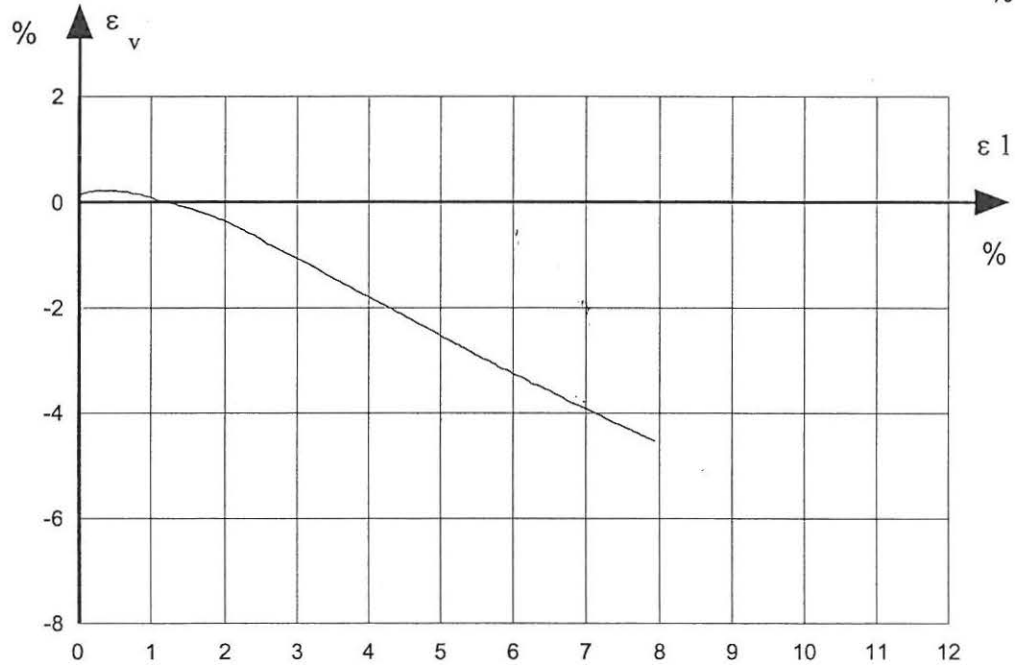
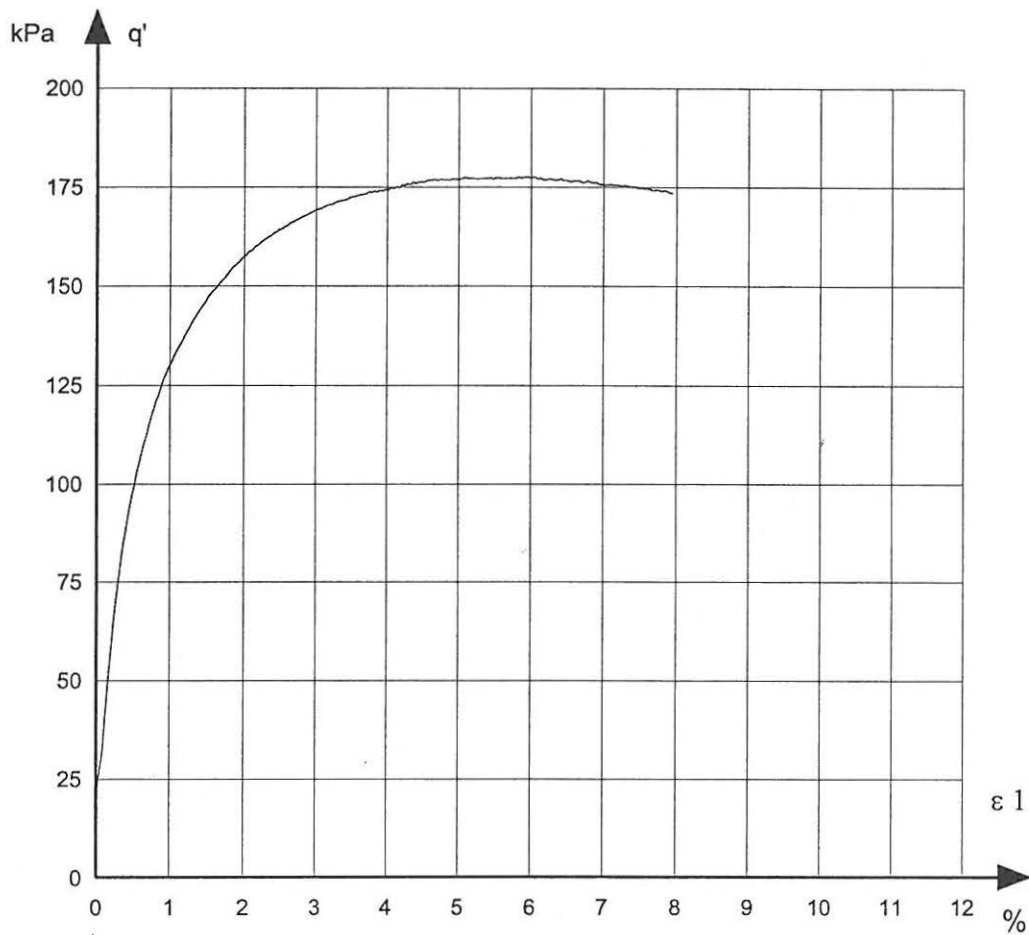
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	177.68 kPa	94.99 kPa
Mean normal stress	p'	100.44 kPa	74.41 kPa
Confining pressures	σ_3	41.21 kPa	42.75 kPa
Vertical strain	ϵ_1	5.94 %	0.45 %
Volumetric strain	ϵ_v	-3.18 %	0.24 %



q'	p'	ϵ_1	ϵ_v
0.00	40.83	0.00	0.00
1.81	42.84	-0.06	0.09
2.87	39.75	-0.10	0.12
17.99	47.21	-0.03	0.15
34.43	52.56	0.09	0.18
64.72	61.51	0.22	0.22
94.99	74.41	0.45	0.24
110.55	76.28	0.64	0.20
129.51	83.62	0.98	0.11
147.39	90.22	1.51	-0.10
158.80	94.02	2.06	-0.38
163.63	94.86	2.44	-0.65
167.74	95.98	2.82	-0.94
170.56	96.41	3.20	-1.19
172.98	97.22	3.58	-1.49
174.53	99.78	3.97	-1.76
176.32	99.73	4.36	-2.04
177.10	100.37	4.74	-2.32
177.26	99.79	5.13	-2.62
177.44	98.45	5.51	-2.90
177.68	100.44	5.94	-3.18
177.11	99.48	6.29	-3.45
175.61	98.86	7.06	-3.95
174.24	97.76	7.82	-4.46

Job:	Encl. No
Lund No 0	3
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1 = 0.099$
Error during saturation.

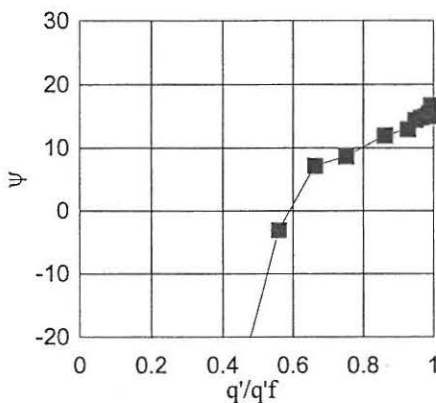
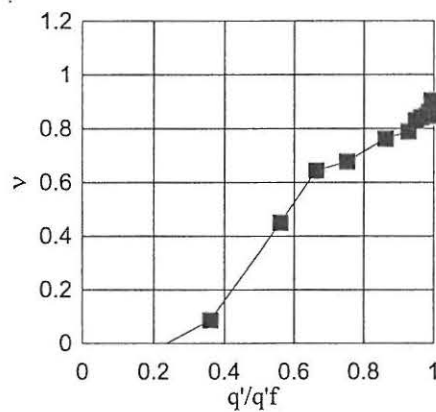


Job:	Encl. No
Lund No 0	4
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.3	
Calibration file	Date	Void ratio	0.618	0.663
kal_tri2	22.09.88	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-40 kPa
		ϵ_1	-0.048 %
		ϵ_v	0.100 %
	2. Drained compression.		
Deformation rate:		8.8 % ph	

		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	184.84 kPa	103.57 kPa
Mean normal stress	p'	101.04 kPa	76.38 kPa
Confining pressures	σ_3	39.43 kPa	41.85 kPa
Vertical strain	ϵ_1	5.18 %	0.44 %
Volumetric strain	ϵ_v	-2.72 %	0.19 %

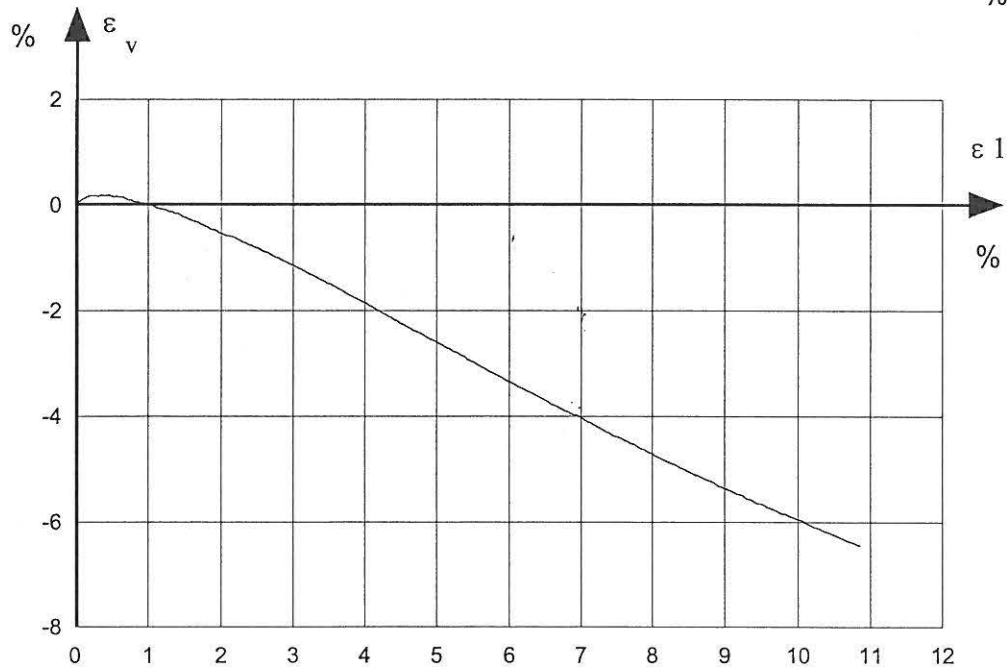
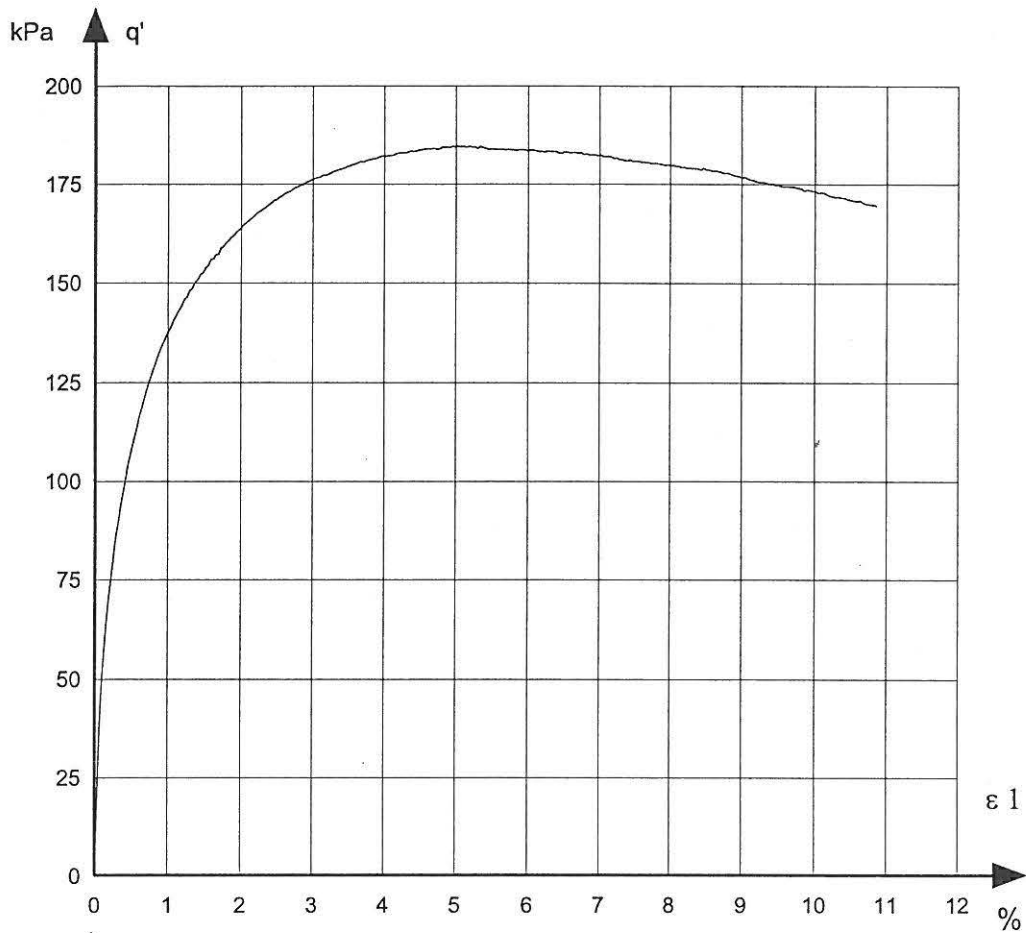


q'	p'	ϵ_1	ϵ_v
0.30	40.81	0.00	0.00
12.98	44.90	0.02	0.03
36.98	53.67	0.06	0.07
66.55	61.99	0.16	0.16
103.57	76.38	0.44	0.19
122.74	80.98	0.69	0.12
139.04	85.39	1.03	-0.00
159.24	93.27	1.75	-0.38
171.23	98.16	2.49	-0.81
175.08	97.15	2.87	-1.05
177.99	100.80	3.25	-1.31
180.55	100.63	3.63	-1.57
182.17	101.17	4.02	-1.85
183.40	102.48	4.41	-2.17
184.42	102.69	4.80	-2.44
184.84	101.04	5.18	-2.72
183.98	102.16	5.56	-3.02
183.37	102.85	6.34	-3.59
182.14	100.02	7.12	-4.11
180.16	101.14	7.88	-4.62
178.44	100.82	8.66	-5.13
175.22	100.00	9.42	-5.62
172.18	97.84	10.20	-6.08
171.85	97.48	10.28	-6.11

Job:	Encl. No
Lund No 0	5
Exc:	Check:
LBI	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1$ 0.159

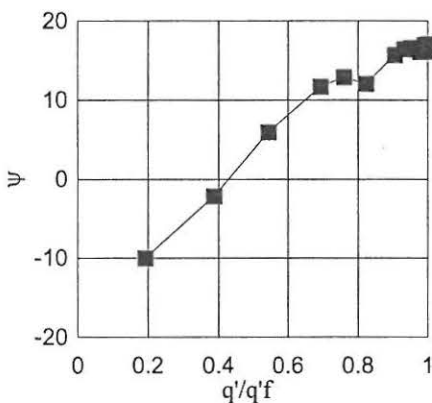
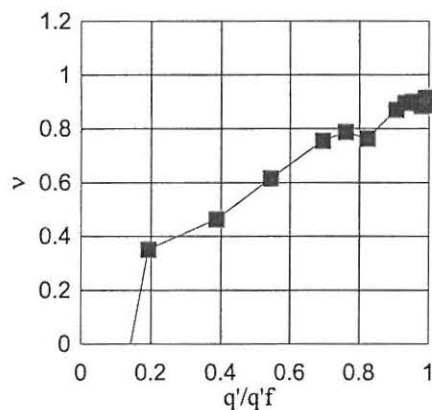


Job:	Encl. No
Lund No 0	6
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	24.6	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.628		
kal_tri2	26.09.88	Saturation	1.04		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.		σ_3	100-20 kPa
			ϵ_1	-0.030 %
			ϵ_v	-0.080 %
	2. Drained compression.		Deformation rate: 8.5 % ph	

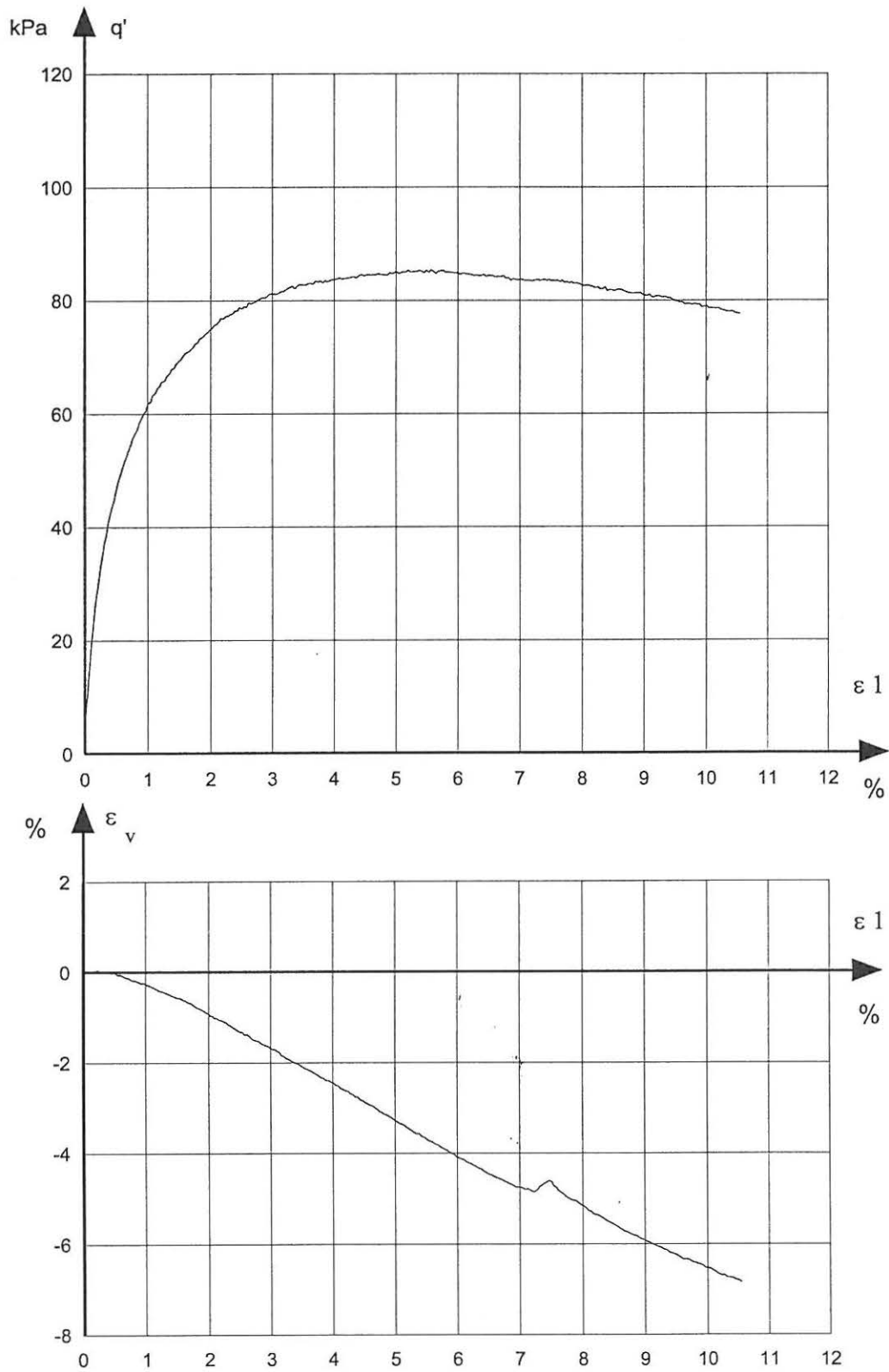
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	85.48	kPa	33.09	kPa
Mean normal stress	p'	52.61	kPa	30.04	kPa
Confining pressures	σ_3	24.12	kPa	19.01	kPa
Vertical strain	ϵ_1	5.56	%	0.24	%
Volumetric strain	ϵ_v	-3.73	%	0.05	%



q'	p'	ϵ_1	ϵ_v
-0.15	22.79	0.00	0.00
7.08	22.14	0.01	0.02
16.42	25.38	0.08	0.04
33.09	30.04	0.24	0.05
46.49	36.17	0.49	-0.00
59.32	40.06	0.90	-0.21
64.99	43.61	1.18	-0.38
70.36	44.12	1.56	-0.58
77.35	47.35	2.28	-1.11
79.58	46.05	2.65	-1.40
81.20	49.02	3.02	-1.69
82.51	48.81	3.39	-1.99
83.52	49.15	3.77	-2.28
83.93	50.18	4.16	-2.58
84.58	50.01	4.55	-2.90
84.82	49.46	4.93	-3.21
85.48	52.61	5.56	-3.73
84.85	50.49	6.06	-4.12
83.56	48.01	6.85	-4.68
83.59	48.53	7.60	-4.80
82.37	48.00	8.36	-5.46
80.77	48.49	9.12	-5.99
79.37	47.00	9.89	-6.44
78.98	47.38	9.96	-6.52

Job:	Encl. No
Lund No 0	7
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.093
Void ratio too large.

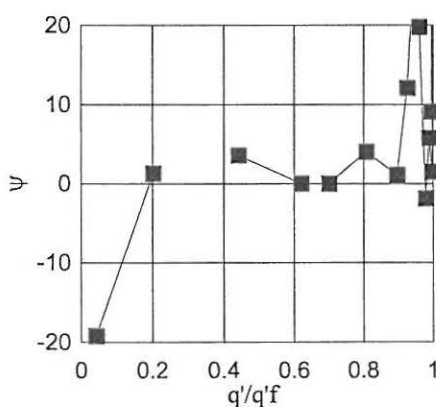
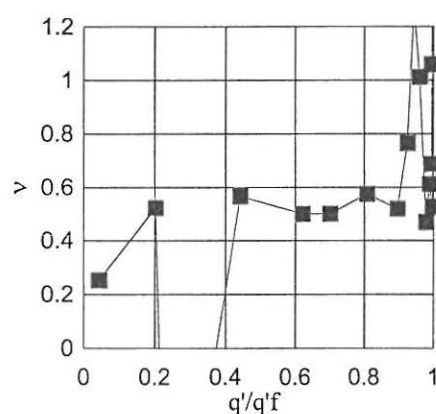


Job:	Encl. No
Lund No 0	8
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.2	
Calibration file	Date	Void ratio	0.616	0.646
kal_tri2	27.09.88	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-20 kPa
		ϵ_1	-0.059 %
		ϵ_v	-0.024 %
	2. Drained compression.		
	Deformation rate:		5.8 % ph

		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	106.47 kPa	25.35 kPa
Mean normal stress	p'	56.29 kPa	29.51 kPa
Confining pressures	σ_3	20.80 kPa	21.05 kPa
Vertical strain	ϵ_1	4.68 %	0.14 %
Volumetric strain	ϵ_v	-1.83 %	0.05 %



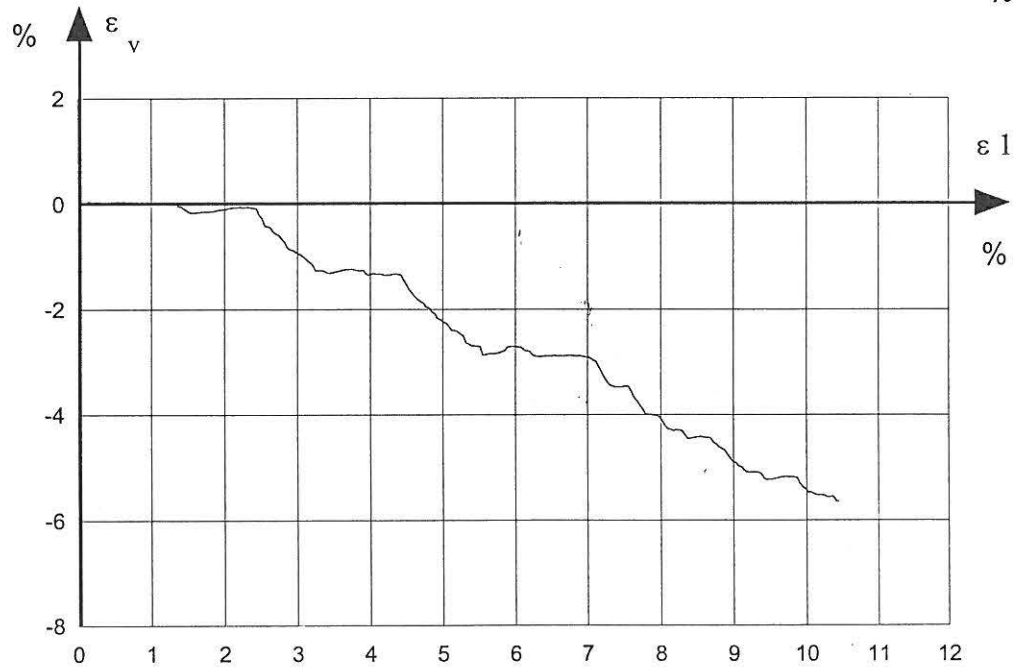
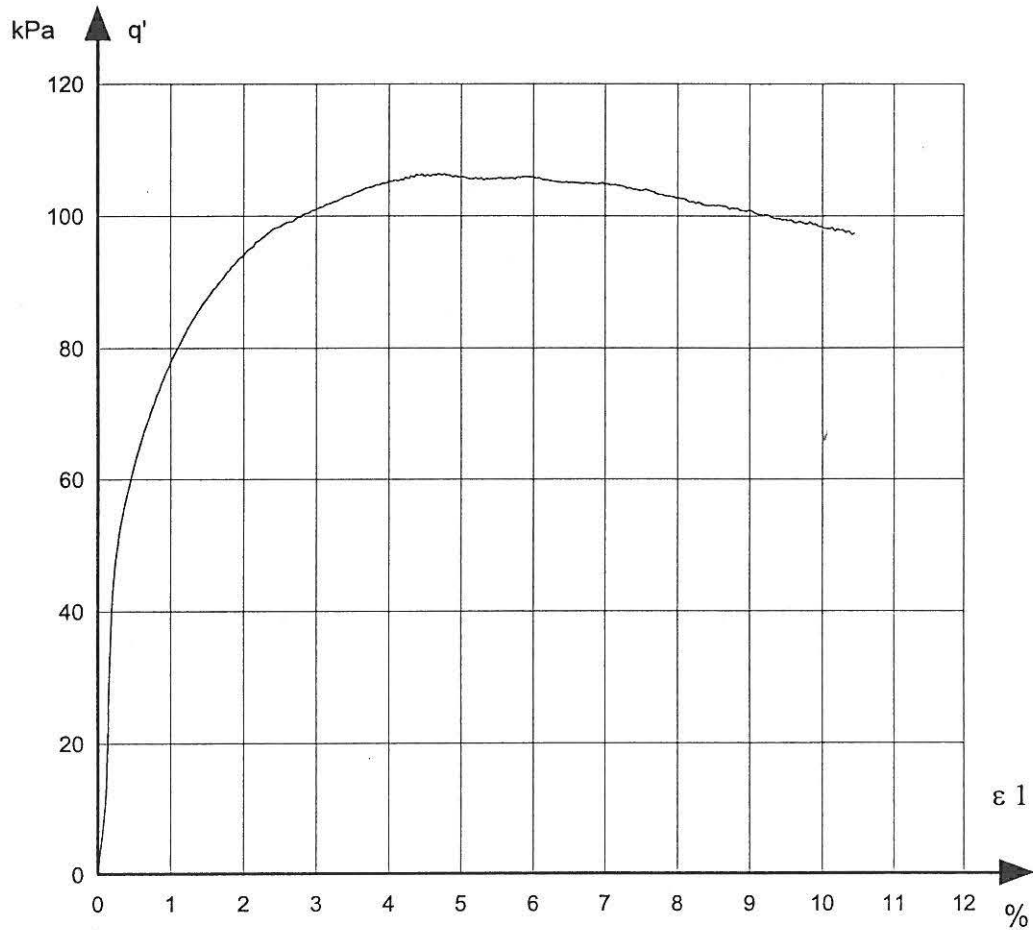
q'	p'	ϵ_1	ϵ_v
0.00	21.05	0.00	0.00
4.53	22.56	0.04	0.02
21.58	28.25	0.13	0.02
25.35	29.51	0.14	0.05
47.04	36.61	0.23	0.04
66.10	43.09	0.59	0.04
74.60	45.92	0.86	0.04
86.05	49.61	1.40	-0.04
95.44	52.74	2.13	-0.07
98.55	53.78	2.51	-0.27
100.49	54.42	2.90	-0.86
102.30	54.90	3.28	-1.26
104.22	55.66	3.67	-1.23
105.29	55.90	4.04	-1.32
106.14	56.18	4.42	-1.34
106.47	56.29	4.68	-1.83
105.85	56.21	5.20	-2.42
105.96	55.86	5.96	-2.70
105.05	55.31	6.73	-2.87
103.88	54.79	7.49	-3.46
101.94	54.27	8.25	-4.29
100.88	53.66	9.02	-4.90
98.87	52.99	9.79	-5.17
98.76	52.95	9.87	-5.19

Job:	Encl. No
Lund No 0	9
Exc:	Check:
LBI	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1$ -0.043

The volumetric strain is not measured correctly.

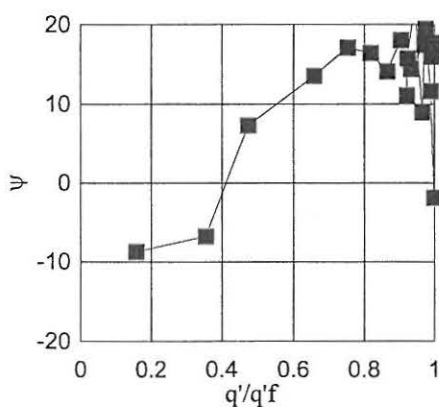
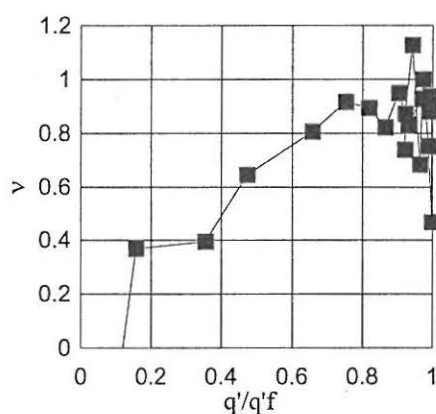


Job:	Encl. No
Lund No 0	10
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.2	
Calibration file	Date	Void ratio	0.611	0.665
kal_tri3	28.09.88	Saturation	1.05	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-20 kPa
		ϵ_1	-0.089 %
		ϵ_v	-0.232 %
	2. Drained compression.		
	Deformation rate:		8.6 % ph

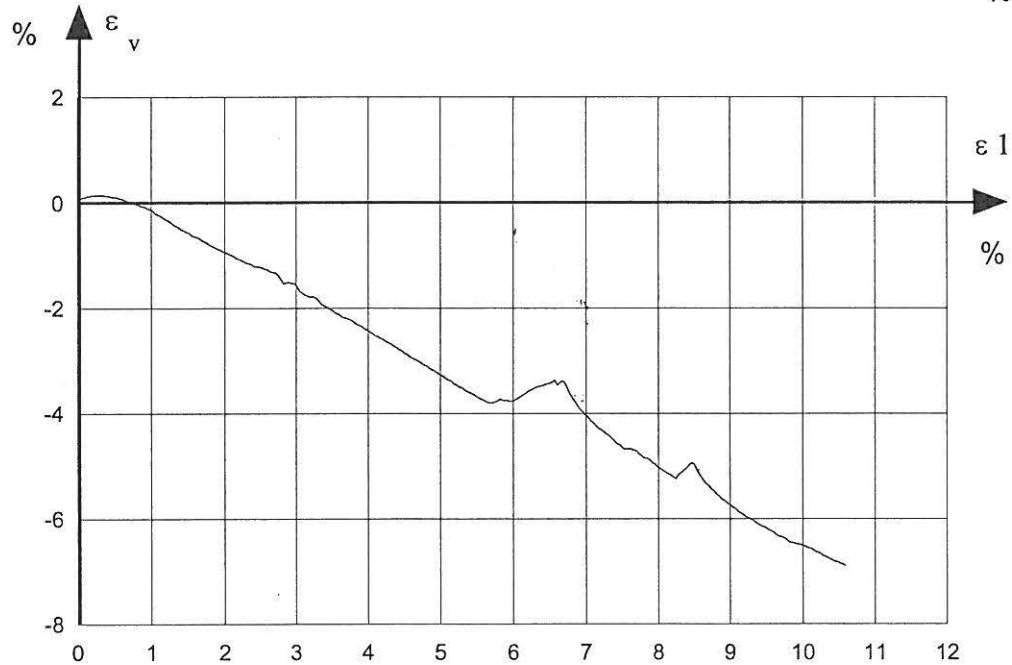
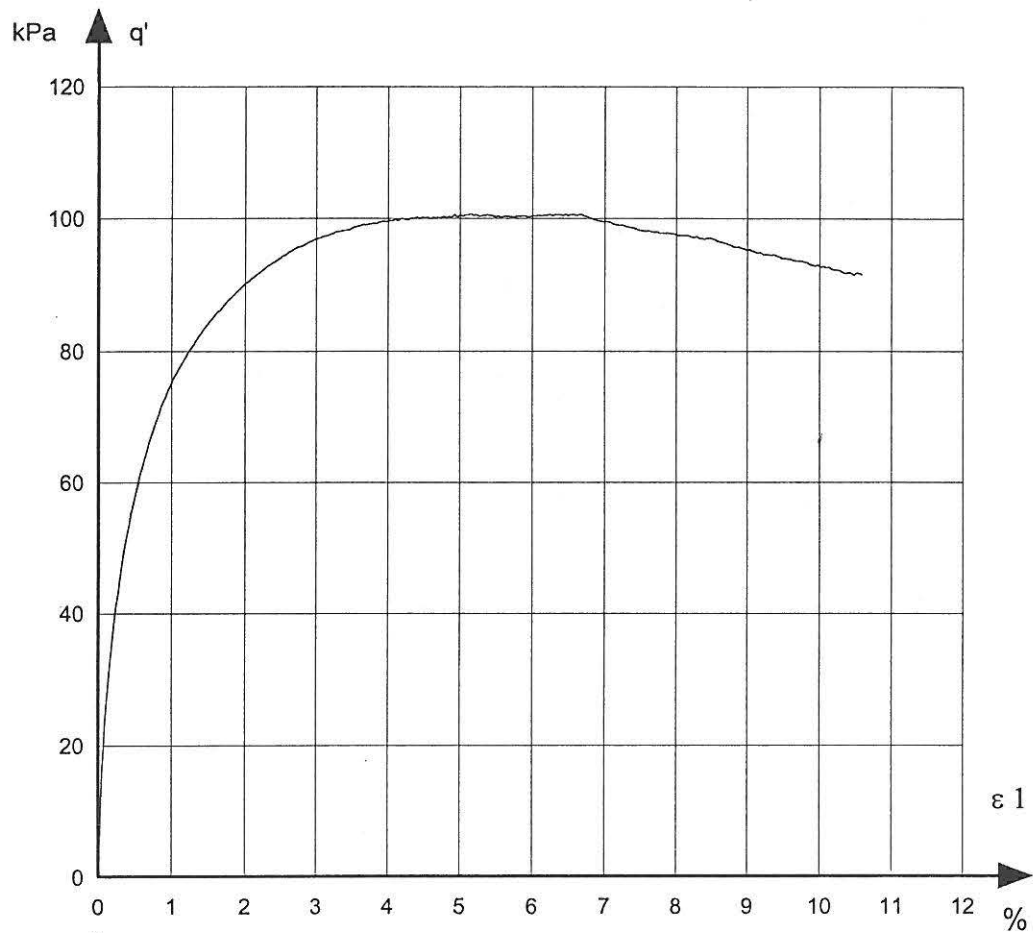
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	100.75 kPa	47.67 kPa
Mean normal stress	p'	54.76 kPa	36.94 kPa
Confining pressures	σ_3	21.18 kPa	21.05 kPa
Vertical strain	ϵ_1	5.14 %	0.32 %
Volumetric strain	ϵ_v	-3.38 %	0.16 %



q'	p'	ϵ_1	ϵ_v
0.45	21.33	0.00	0.00
6.93	26.46	0.05	0.10
15.82	30.12	0.11	0.11
35.83	36.94	0.32	0.16
47.67	42.32	0.62	0.07
66.29	46.95	1.09	-0.22
75.92	48.64	1.37	-0.45
82.38	50.23	1.72	-0.73
87.14	52.32	2.45	-1.20
90.99	53.19	2.82	-1.53
93.79	53.62	3.20	-1.78
97.70	54.01	3.57	-2.10
99.62	54.26	3.95	-2.38
100.08	54.54	4.32	-2.67
100.21	54.46	4.70	-3.00
100.75	54.76	5.14	-3.38
100.47	54.54	5.45	-3.63
100.54	54.57	6.19	-3.58
99.65	54.27	6.94	-3.95
98.01	53.85	7.69	-4.70
97.11	53.55	8.43	-4.97
94.94	52.83	9.18	-5.91
92.89	52.40	9.94	-6.47
92.78	51.98	10.01	-6.51

Job:	Encl. No
Lund No 0	11
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.070

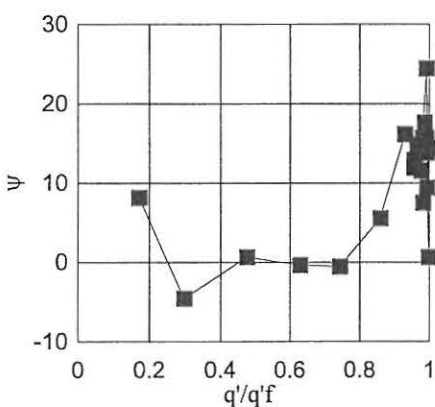
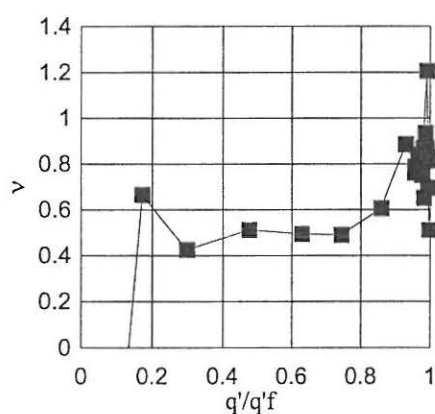


Job:	Encl. No
Lund No 0	12
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.1	
Calibration file	Date	Void ratio	0.621	0.665
kal_tri4	4.10.88	Saturation	1.03	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-10 kPa
		ϵ_1	-0.002 %
		ϵ_v	0.016 %
	2. Drained compression.		
	Deformation rate:		8.5 % ph

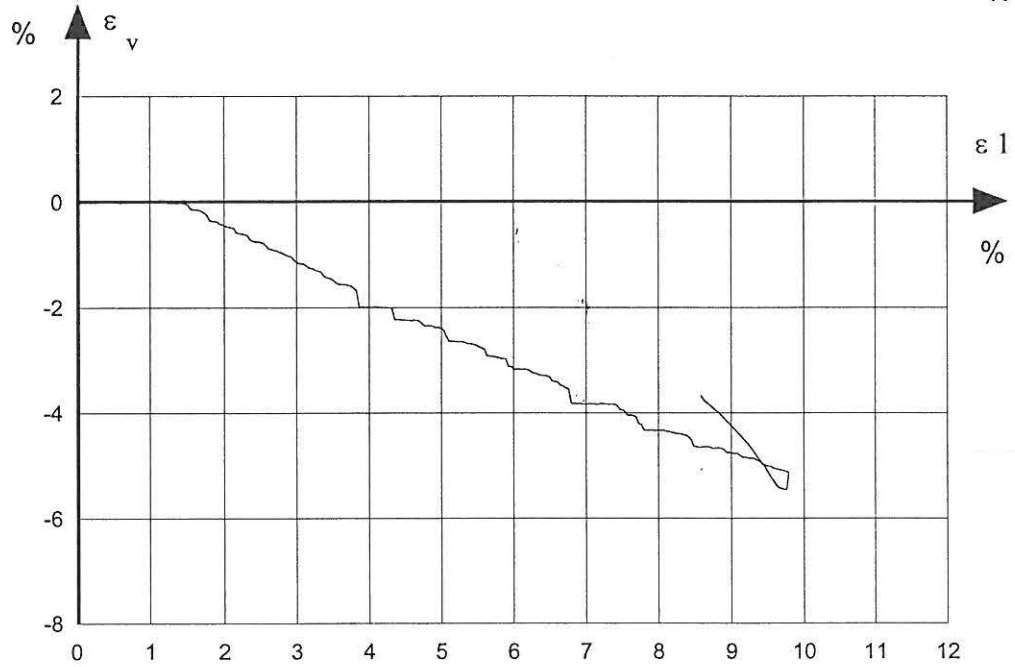
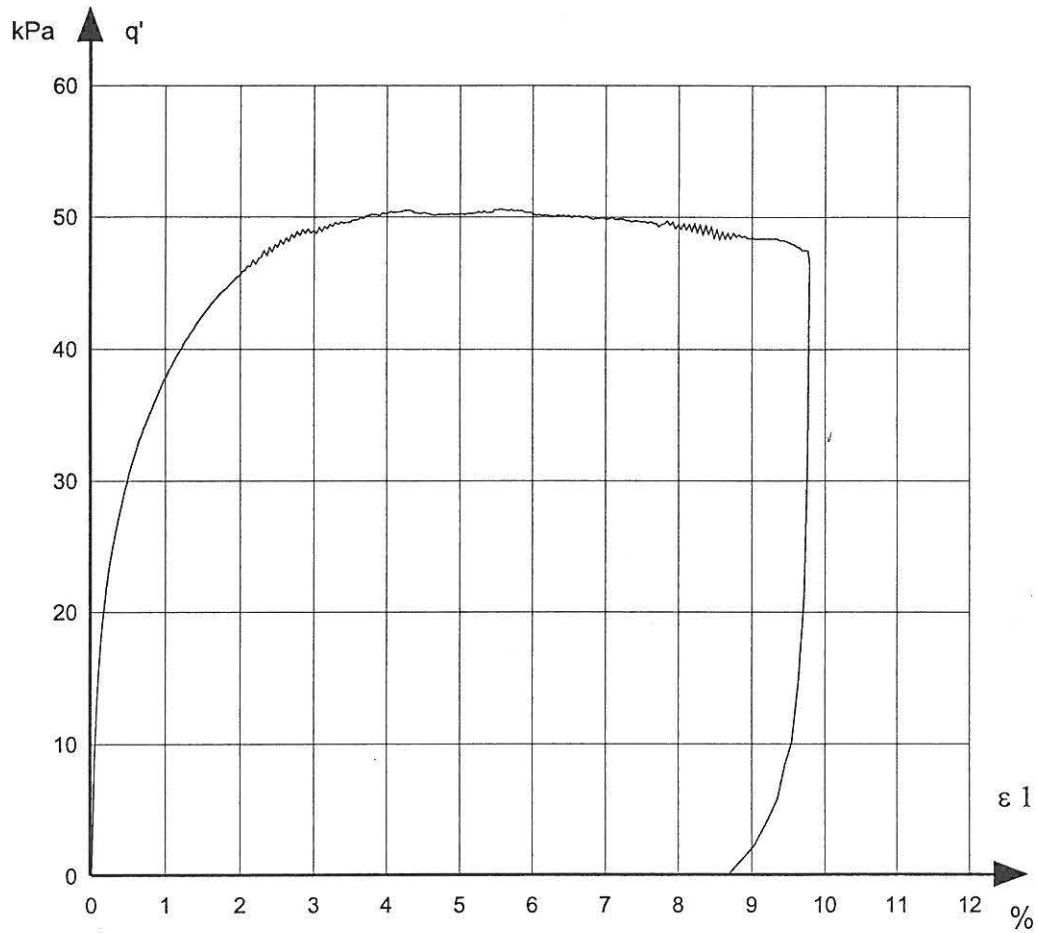
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	50.65 kPa	37.69 kPa
Mean normal stress	p'	26.33 kPa	22.13 kPa
Confining pressures	σ_3	9.44 kPa	9.57 kPa
Vertical strain	ϵ_1	5.55 %	0.97 %
Volumetric strain	ϵ_v	-2.75 %	0.00 %



q'	p'	ϵ_1	ϵ_v
-0.05	9.30	0.00	0.00
0.00	9.31	-0.00	-0.00
8.67	12.33	0.04	-0.02
15.08	14.47	0.09	-0.01
24.10	17.48	0.26	-0.01
31.86	19.93	0.57	-0.01
37.69	22.13	0.97	0.00
43.50	23.94	1.62	-0.14
47.07	25.00	2.35	-0.70
48.43	25.59	2.73	-0.92
48.89	25.87	3.10	-1.17
49.61	25.98	3.48	-1.45
50.20	26.18	3.86	-1.99
50.52	26.54	4.23	-2.00
50.20	26.18	4.61	-2.23
50.26	26.19	4.99	-2.38
50.65	26.33	5.55	-2.75
50.15	26.29	6.11	-3.17
49.94	26.09	6.86	-3.81
49.70	26.01	7.61	-4.04
49.42	25.66	8.37	-4.42
48.39	25.32	9.12	-4.81
21.17	16.63	9.71	-5.44
10.17	12.20	9.54	-5.22

Job:	Encl. No
Lund No 0	13
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ -0.041

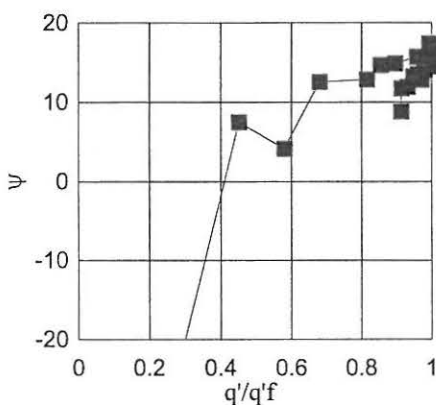
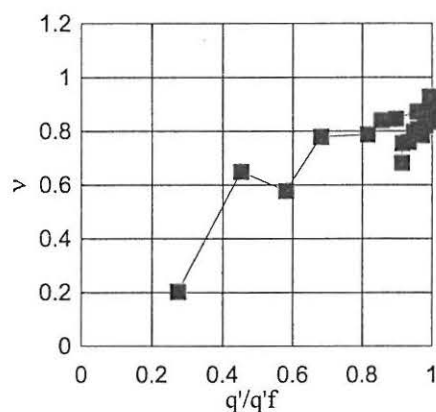


Job:	Encl. No
Lund No 0	14
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.9	
Calibration file	Date	Void ratio	0.612	0.669
kal_tri4	6.10.88	Saturation	1.03	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-10 kPa
		ϵ_1	0.003 %
		ϵ_v	-0.004 %
	2.Drained compression.		
	Deformation rate:		8.8 % ph

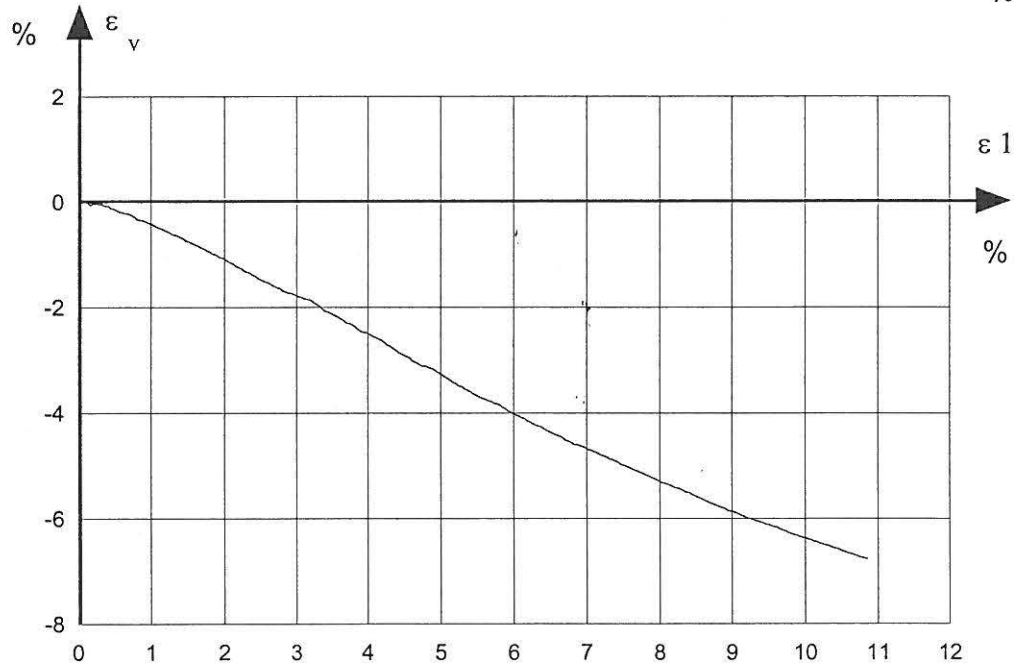
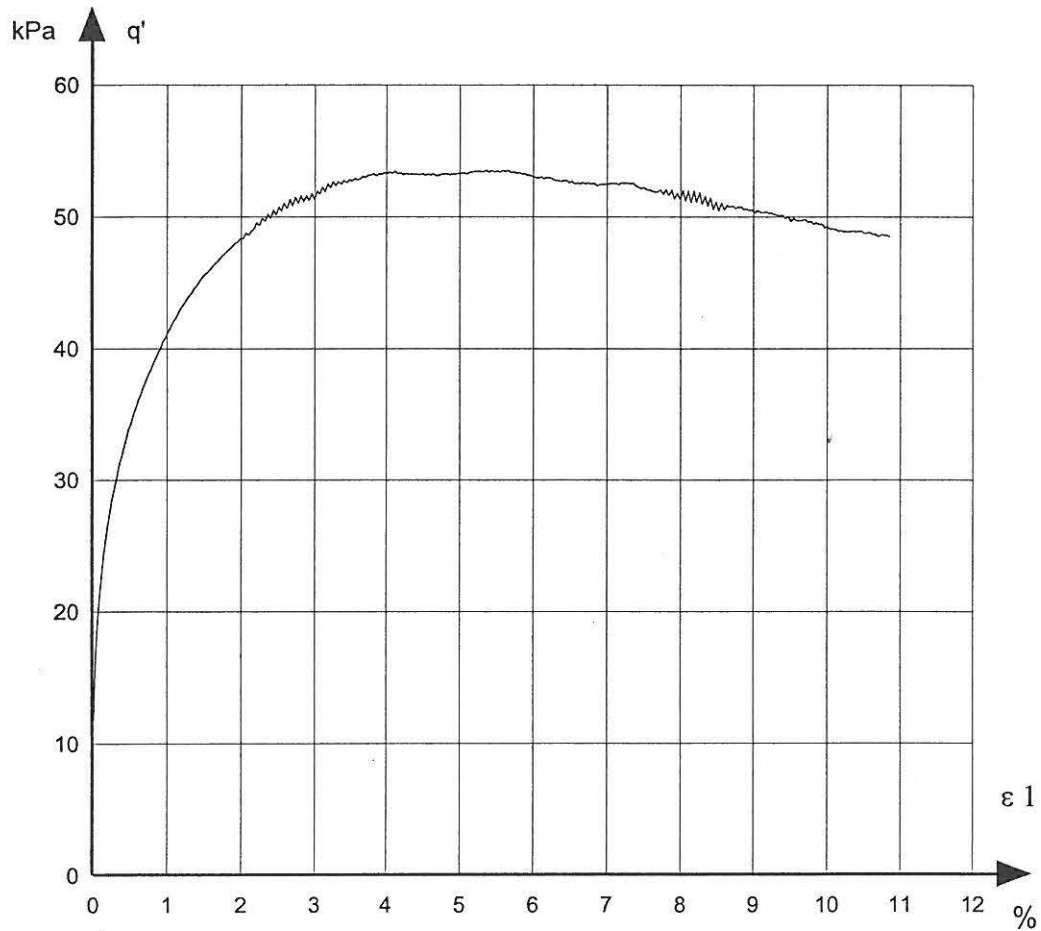
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	53.56	kPa	14.68	kPa
Mean normal stress	p'	28.06	kPa	15.36	kPa
Confining pressures	σ_3	10.21	kPa	10.46	kPa
Vertical strain	ϵ_1	5.36	%	0.01	%
Volumetric strain	ϵ_v	-3.55	%	0.01	%



q'	p'	ϵ_1	ϵ_v
11.87	14.29	0.00	0.00
14.68	15.36	0.01	0.01
24.20	18.40	0.13	-0.03
31.08	20.70	0.34	-0.06
36.45	22.61	0.63	-0.22
43.67	24.76	1.25	-0.58
45.81	25.48	1.54	-0.77
48.00	26.34	1.91	-1.03
51.32	27.44	2.65	-1.58
51.97	27.66	3.03	-1.79
52.81	27.94	3.40	-2.06
53.18	28.06	3.78	-2.32
53.30	28.10	4.15	-2.60
53.20	27.69	4.53	-2.92
53.28	27.84	4.90	-3.17
53.56	28.06	5.36	-3.55
53.56	28.06	5.66	-3.76
52.78	27.80	6.40	-4.29
52.46	27.70	7.16	-4.77
51.37	27.08	7.91	-5.23
50.77	27.13	8.67	-5.69
49.91	26.84	9.43	-6.08
48.97	26.40	10.19	-6.47
48.91	26.39	10.27	-6.49

Job:	Encl. No
Lund No 0	15
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.036

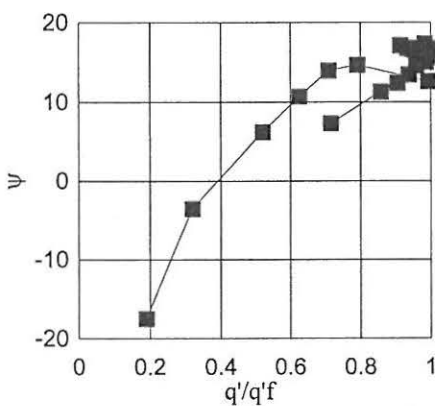
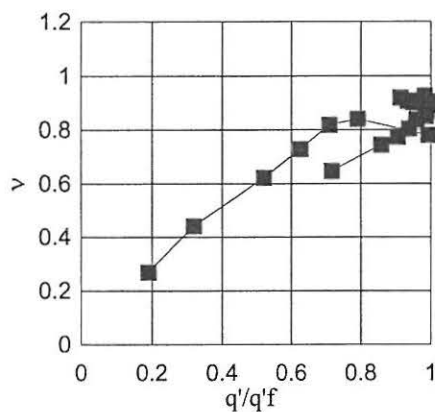


Job:	Encl. No
Lund No 0	16
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.1	
Calibration file	Date	Void ratio	0.617	0.675
kal_tri4	07.10.88	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-5 kPa
		ϵ_1	-0.006 %
		ϵ_v	0.000 %
	2. Drained compression.		
	Deformation rate:		8.6 % ph

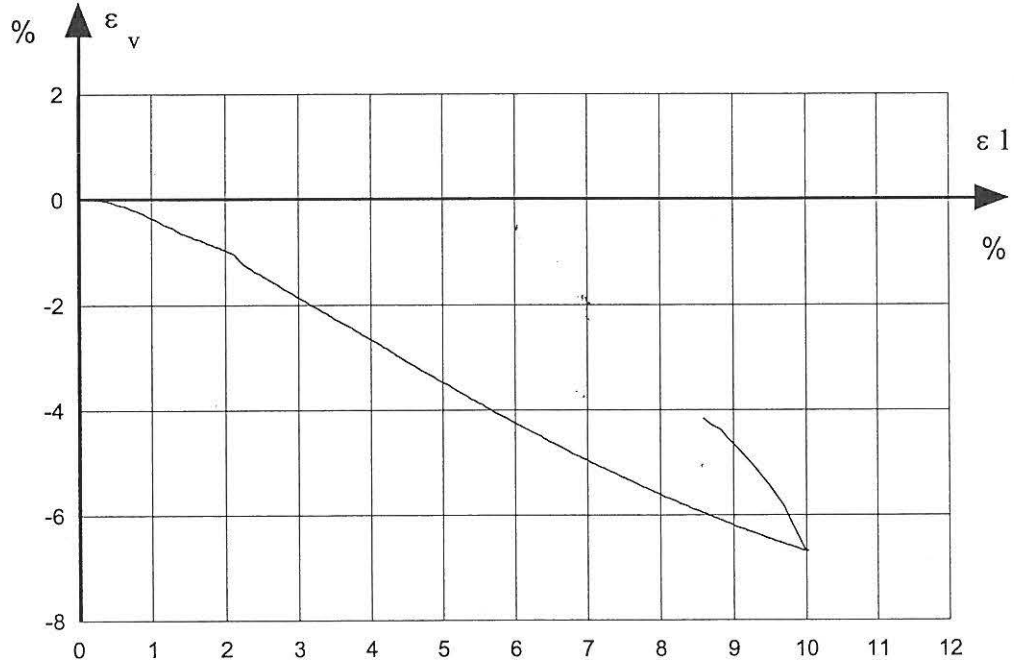
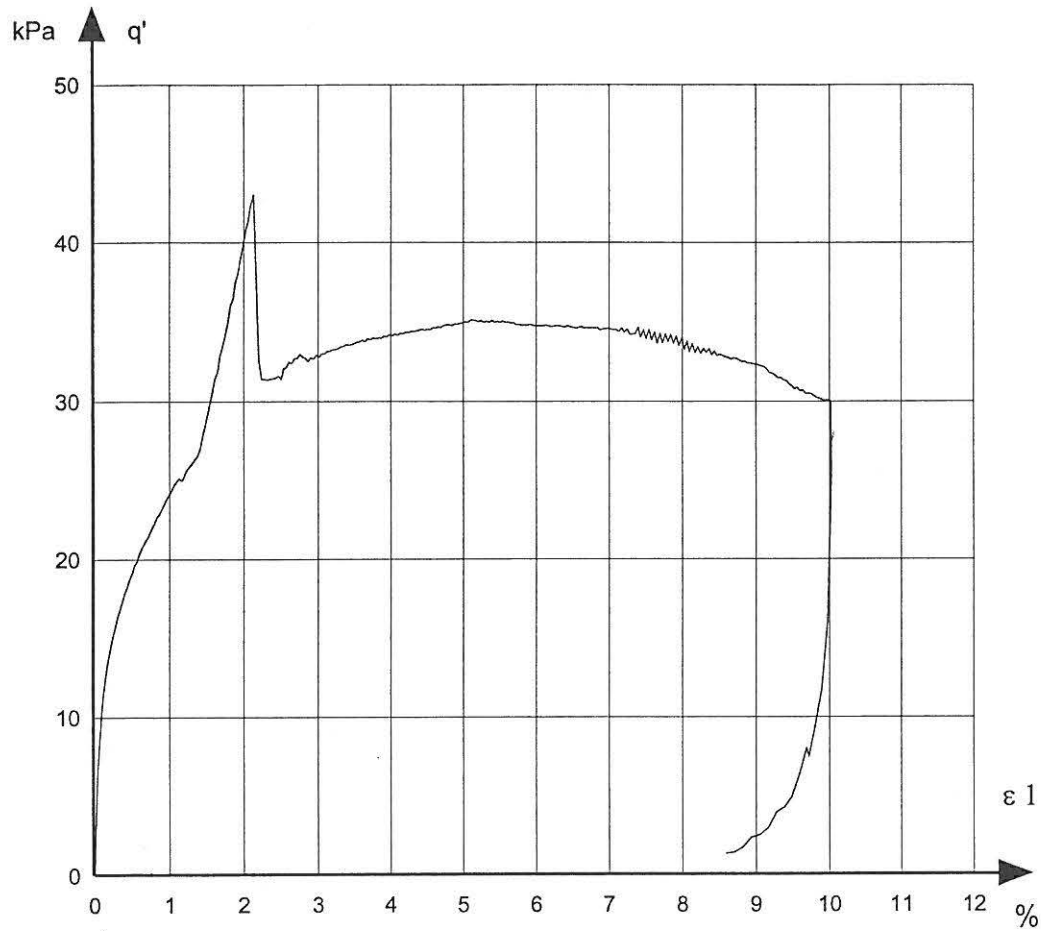
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	35.14 kPa	11.22 kPa
Mean normal stress	p'	16.69 kPa	8.72 kPa
Confining pressures	σ_3	4.98 kPa	4.98 kPa
Vertical strain	ϵ_1	5.13 %	0.11 %
Volumetric strain	ϵ_v	-3.57 %	0.03 %



q'	p'	ϵ_1	ϵ_v
0.00	4.98	0.00	0.00
6.67	7.20	0.04	0.02
11.22	8.72	0.11	0.03
18.27	11.06	0.43	-0.05
22.00	12.31	0.75	-0.20
24.94	13.42	1.09	-0.41
27.79	14.50	1.44	-0.65
34.87	18.64	1.78	-0.84
32.08	15.80	2.53	-1.47
32.77	16.03	2.90	-1.77
33.31	16.08	3.27	-2.06
33.75	16.23	3.64	-2.36
34.13	16.35	4.01	-2.66
34.50	16.48	4.39	-2.98
34.82	16.58	4.76	-3.28
35.14	16.69	5.13	-3.57
34.99	16.77	5.50	-3.87
34.76	16.56	6.24	-4.42
34.63	16.52	6.98	-4.94
33.66	16.20	7.73	-5.44
32.90	16.07	8.47	-5.89
31.78	15.44	9.22	-6.30
30.13	15.15	9.98	-6.67
25.17	13.37	10.02	-6.68

Job:	Encl. No
Lund No 0	17
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ -0.055

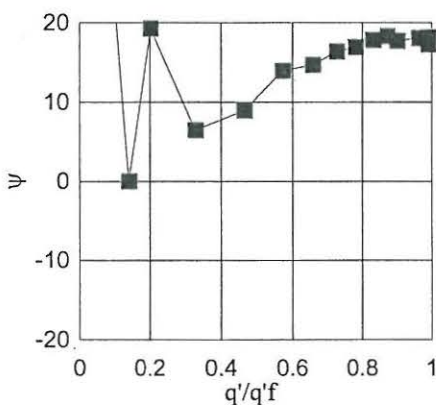
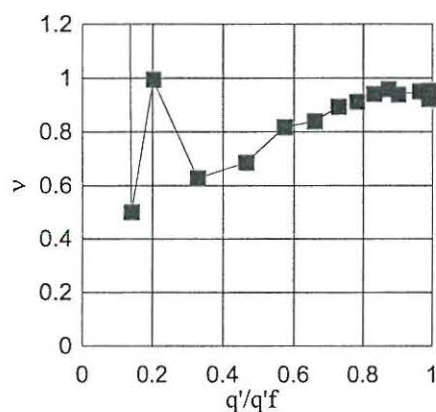


Job:	Lund No 0	Encl. No	18
Exc:	LBI	Check:	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.9	
Calibration file	Date	Void ratio	0.606	0.690
kal_tri4	10.10.88	Saturation	1.05	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-5 kPa
		ϵ_1	-0.115 %
		ϵ_v	-0.785 %
	2. Drained compression.		
	Deformation rate:		8.4 % ph

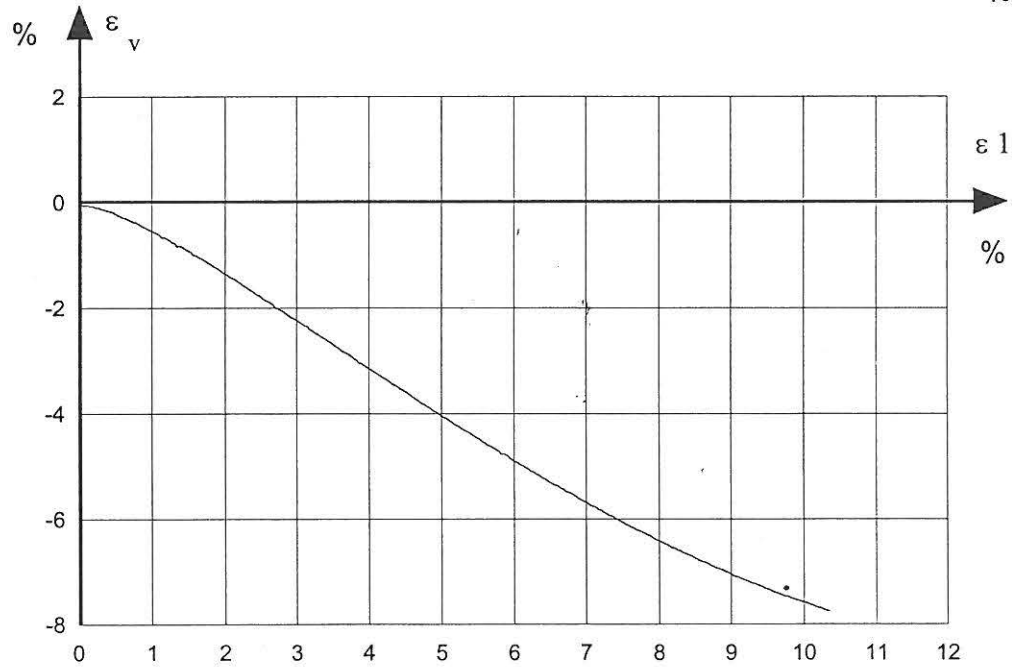
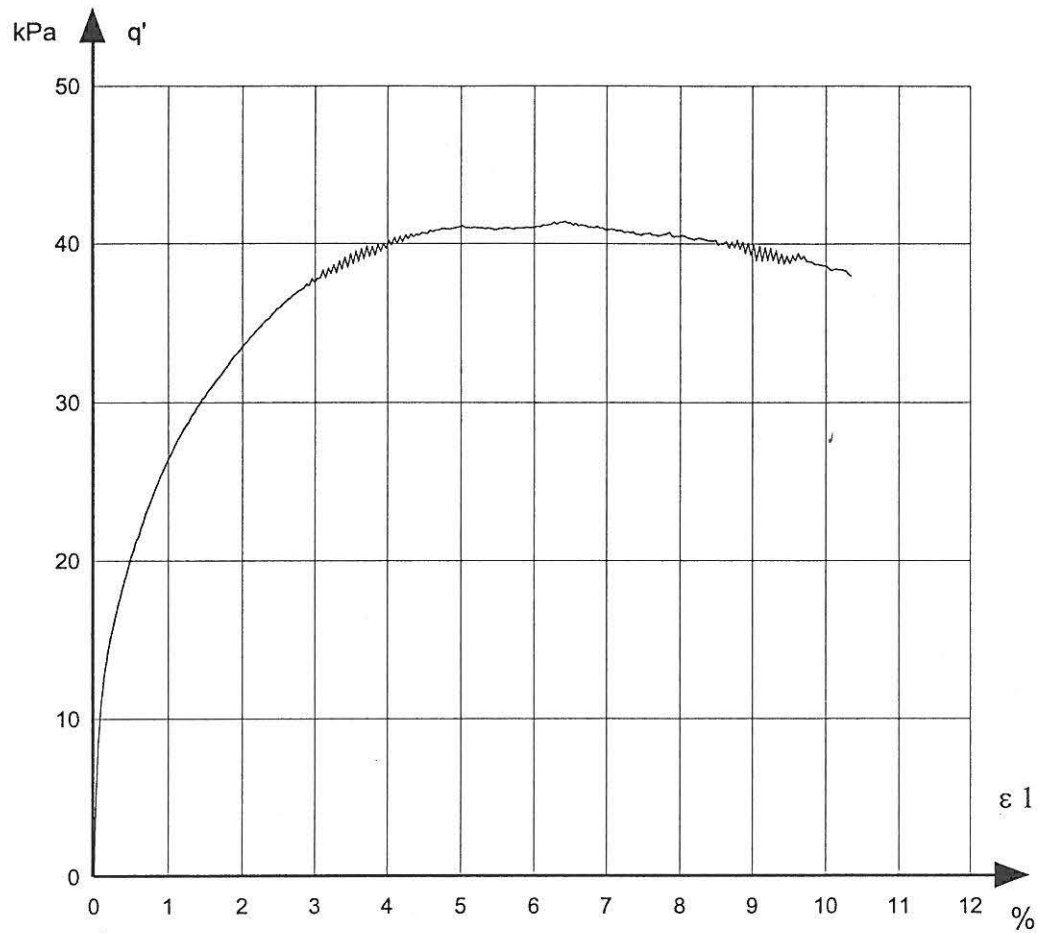
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	41.47 kPa	0.05 kPa
Mean normal stress	p'	19.06 kPa	4.87 kPa
Confining pressures	σ_3	5.23 kPa	4.85 kPa
Vertical strain	ϵ_1	6.42 %	0.00 %
Volumetric strain	ϵ_v	-5.23 %	0.01 %



q'	p'	ϵ_1	ϵ_v
0.05	4.87	0.00	0.01
0.45	5.13	0.00	-0.02
5.77	7.03	0.03	-0.02
8.40	7.78	0.06	-0.05
13.65	9.53	0.17	-0.08
19.28	11.40	0.44	-0.18
23.76	13.02	0.75	-0.38
27.34	14.09	1.09	-0.60
30.18	15.04	1.44	-0.88
32.40	15.90	1.81	-1.18
34.48	16.60	2.17	-1.50
36.18	17.17	2.55	-1.84
37.35	17.55	2.92	-2.17
39.94	18.42	4.04	-3.18
40.57	18.75	4.42	-3.52
40.96	18.89	4.80	-3.85
41.08	18.80	5.17	-4.19
41.04	18.91	5.93	-4.83
41.47	19.06	6.42	-5.23
40.64	18.78	7.43	-6.00
40.28	18.66	8.18	-6.53
40.07	18.59	8.94	-7.00
39.25	18.31	9.70	-7.43
38.91	18.33	9.77	-7.46

Job:	Encl. No
Lund No 0	19
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.064

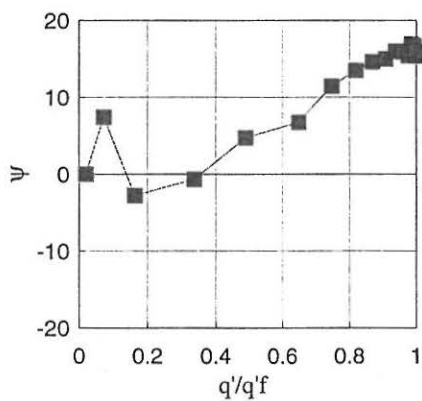
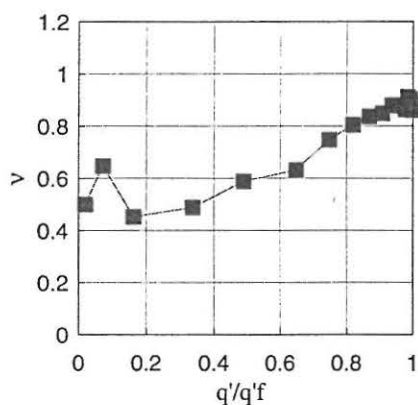


Job:	Encl. No
Lund No 0	20
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	24.1	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.612		
kal_tri3	03.10.88	Saturation	1.04		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-40 kPa
		ϵ_1	0.000 %
		ϵ_v	0.000 %
	2. Drained compression.		
Deformation rate:			10.4 % ph

		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	173.29	kPa	31.78	kPa
Mean normal stress	p'	97.83	kPa	50.66	kPa
Confining pressures	σ_3	40.07	kPa	40.07	kPa
Vertical strain	ϵ_1	5.62	%	0.22	%
Volumetric strain	ϵ_v	-3.48	%	0.00	%

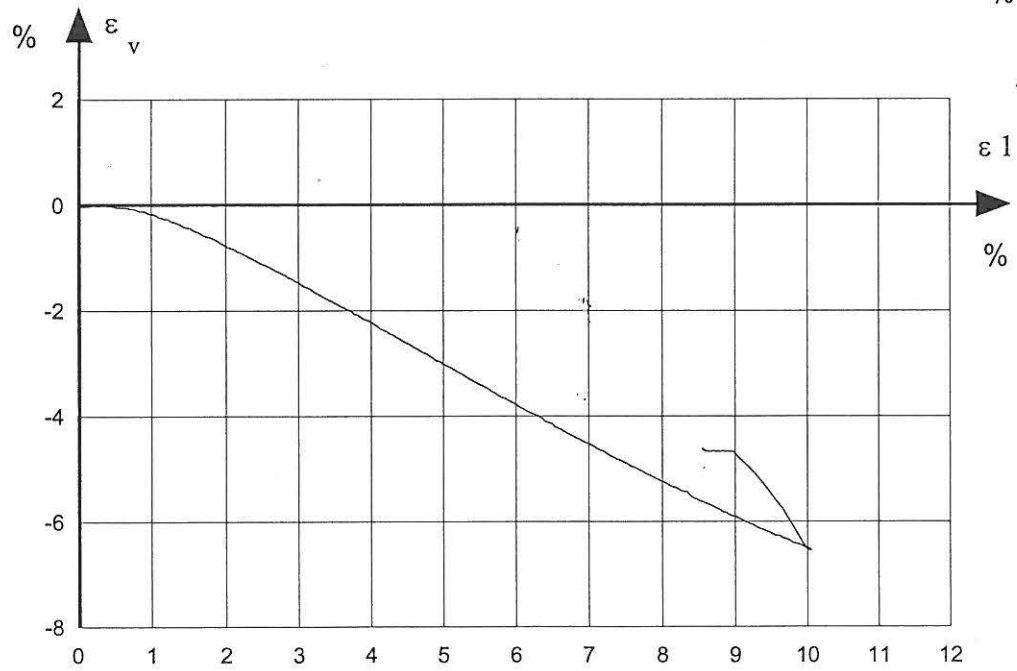
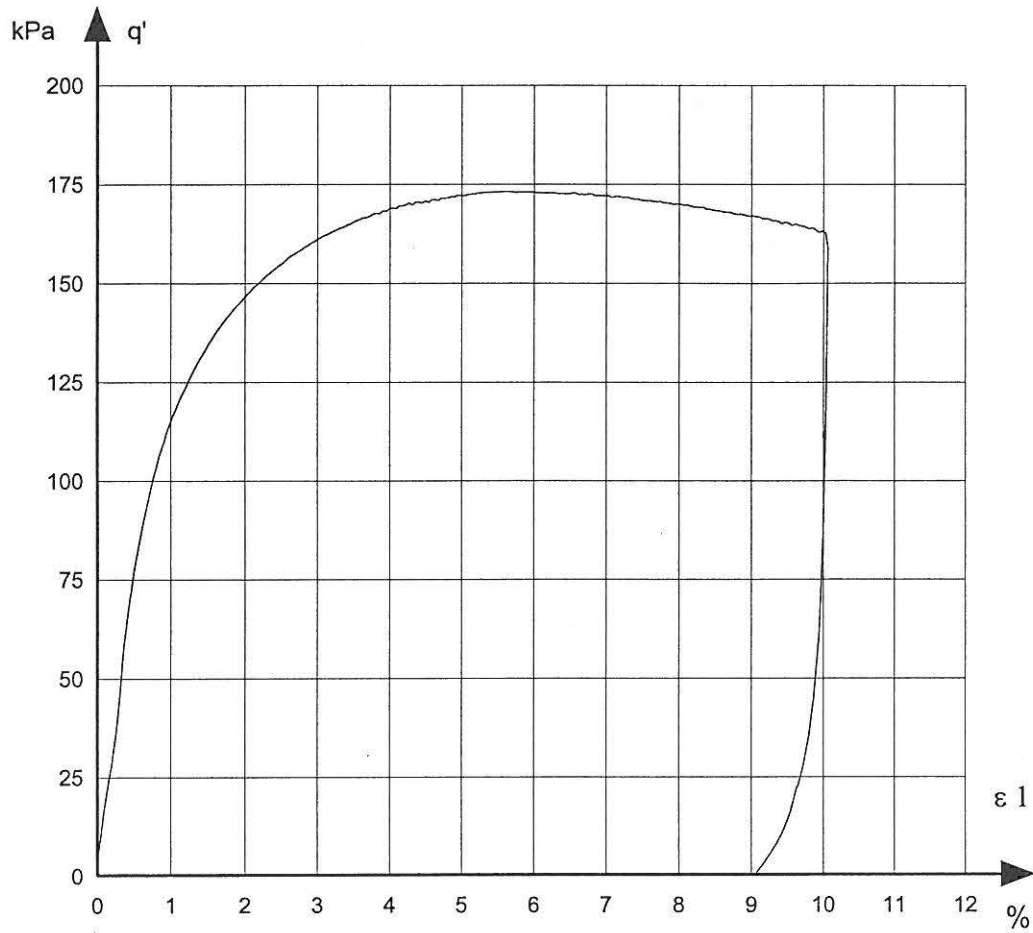


q'	p'	ϵ_1	ϵ_v
3.47	41.10	-0.01	0.00
12.52	44.11	0.06	-0.02
28.02	49.41	0.18	-0.01
58.34	59.51	0.36	-0.00
84.60	68.01	0.56	-0.04
112.26	77.36	0.94	-0.14
129.54	83.12	1.34	-0.34
141.77	87.45	1.76	-0.60
150.39	90.07	2.20	-0.89
157.10	92.43	2.64	-1.20
162.19	94.13	3.09	-1.54
166.04	95.28	3.54	-1.88
168.98	96.27	3.99	-2.21
170.39	96.99	4.44	-2.58
171.91	97.37	4.89	-2.94
173.01	97.61	5.35	-3.29
173.29	97.83	5.62	-3.48
172.83	97.55	6.25	-3.96
172.49	97.44	6.70	-4.31
172.06	97.42	7.16	-4.64
170.97	96.93	7.61	-4.96
168.52	96.11	8.51	-5.61
165.18	95.00	9.42	-6.17
27.79	49.46	9.71	-5.90

Job:	Encl. No
Lund No 0	21
Exc:	Check:
LBI	FRJ

Remark:

Preparation [%] $\Delta\epsilon_1 = 0.012$
 Deformation during isotropic
 consolidation not measured.

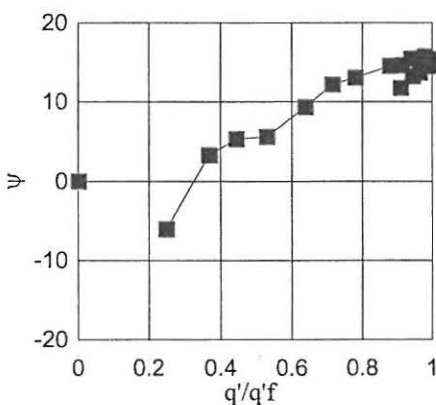
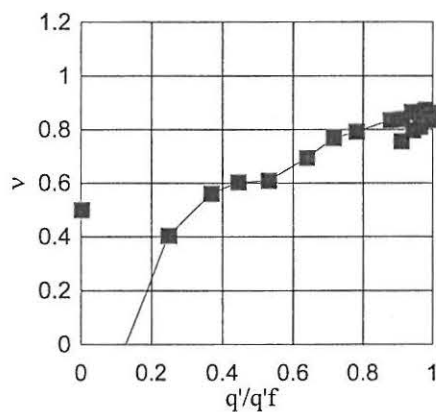


Job:	Lund No 0	Encl. No	22
Exc:	LBI	Check:	FRJ

Description of soil Lund No 0		Water content %	24.3	At failure 0.675
		Grain density	2.65	
Calibration file	Date	Void ratio	0.618	
		Saturation	1.04	
kal_tri4	12.10.88	Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-5 kPa
		ϵ_1	-0.125 %
		ϵ_v	-0.433 %
	2. Drained compression.	Deformation rate: 8.4 % ph	

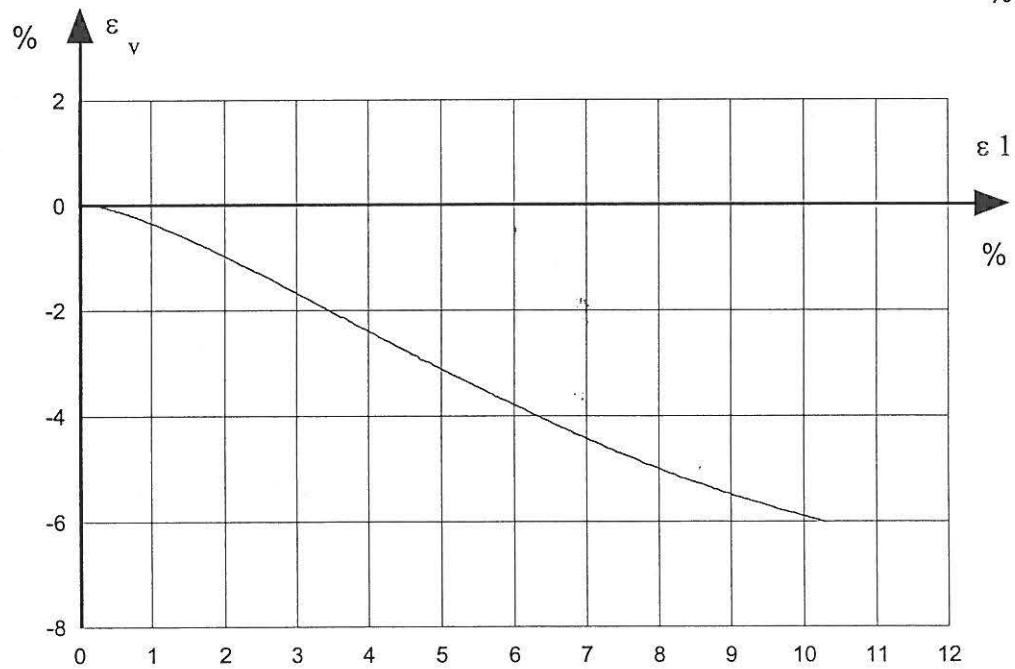
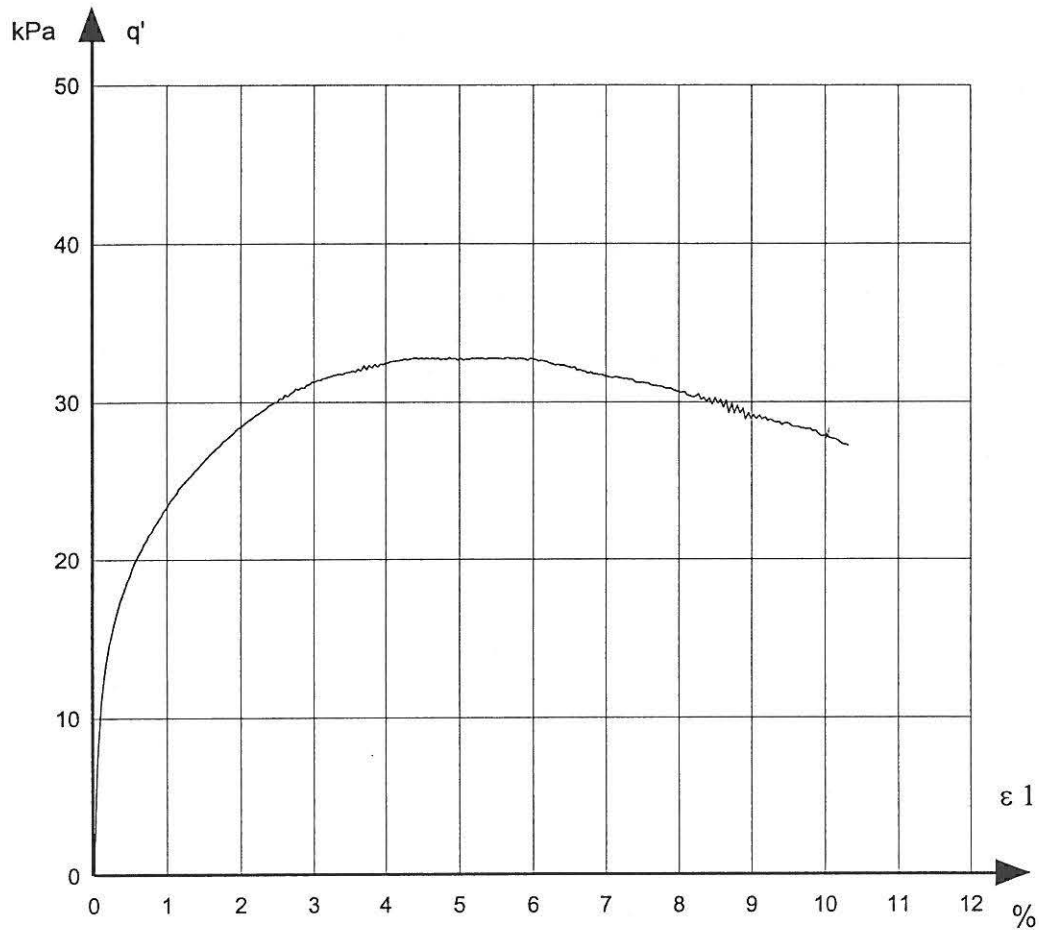
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	32.85 kPa	8.18 kPa
Mean normal stress	p'	15.92 kPa	7.70 kPa
Confining pressures	σ_3	4.98 kPa	4.98 kPa
Vertical strain	ϵ_1	5.66 %	0.06 %
Volumetric strain	ϵ_v	-3.56 %	0.02 %



q'	p'	ϵ_1	ϵ_v
0.05	4.87	0.00	0.00
0.10	4.88	0.00	-0.01
3.00	5.97	0.02	0.01
8.18	7.70	0.06	0.02
12.12	8.89	0.13	0.01
14.60	9.84	0.20	-0.00
17.41	10.65	0.35	-0.04
20.99	11.97	0.67	-0.16
23.54	12.82	1.02	-0.34
25.66	13.40	1.38	-0.55
28.88	14.47	2.11	-1.05
29.99	15.10	2.49	-1.30
30.90	15.28	2.86	-1.57
31.64	15.52	3.24	-1.85
32.14	15.69	3.61	-2.13
32.49	15.81	3.99	-2.38
32.79	15.91	4.37	-2.66
32.72	15.88	4.75	-2.93
32.73	15.89	5.13	-3.20
32.85	15.92	5.66	-3.56
32.46	15.80	6.26	-3.96
31.66	15.53	7.01	-4.43
31.01	15.18	7.77	-4.88
29.90	14.82	8.53	-5.26

Job:	Encl. No
Lund No 0	23
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.066

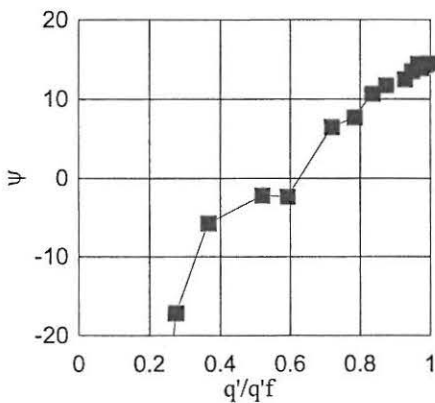
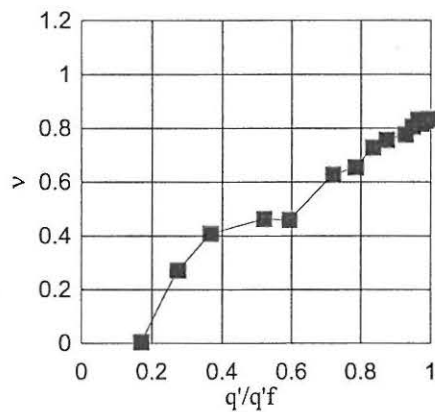


Job:	Encl. No
Lund No 0	24
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.5	
Calibration file	Date	Void ratio	0.617	0.663
kal_tri4	13.10.88	Saturation	1.05	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-80 kPa
		ϵ_1	-0.011 %
		ϵ_v	-0.248 %
	2. Drained compression.		
	Deformation rate:		8.1 % ph

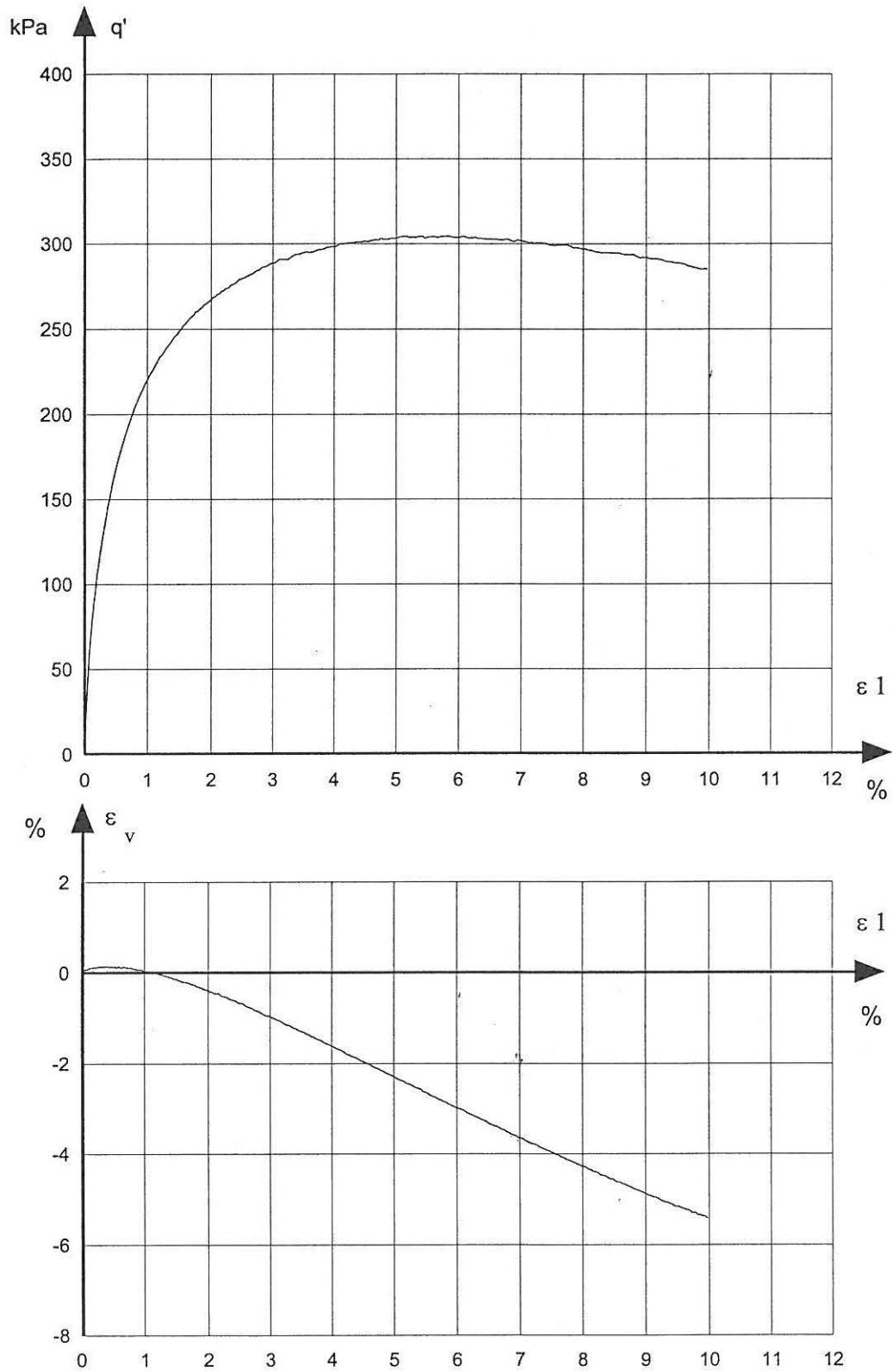
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	304.72	kPa	180.68	kPa
Mean normal stress	p'	182.47	kPa	141.00	kPa
Confining pressures	σ_3	80.90	kPa	80.77	kPa
Vertical strain	ϵ_1	5.82	%	0.58	%
Volumetric strain	ϵ_v	-2.84	%	0.15	%



q'	p'	ϵ_1	ϵ_v
0.00	80.77	0.00	0.00
9.10	83.68	0.01	0.02
30.14	90.69	0.03	0.05
51.49	97.80	0.06	0.08
83.54	108.62	0.13	0.11
111.59	117.97	0.21	0.12
158.46	133.72	0.43	0.14
180.68	141.00	0.58	0.15
219.34	153.88	0.98	0.05
239.24	160.65	1.29	-0.04
254.44	165.58	1.62	-0.19
266.26	169.27	1.96	-0.36
282.57	174.83	2.66	-0.75
288.81	176.91	3.01	-0.97
293.95	179.01	3.37	-1.20
296.56	179.62	3.73	-1.43
300.18	181.09	4.09	-1.67
302.82	181.58	4.83	-2.16
304.72	182.47	5.82	-2.84
303.14	181.43	6.31	-3.17
301.36	180.97	7.05	-3.67
298.71	180.47	7.80	-4.15
294.78	179.16	8.55	-4.62
289.81	177.25	9.31	-5.05

Job:	Encl. No
Lund No 0	25
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.037

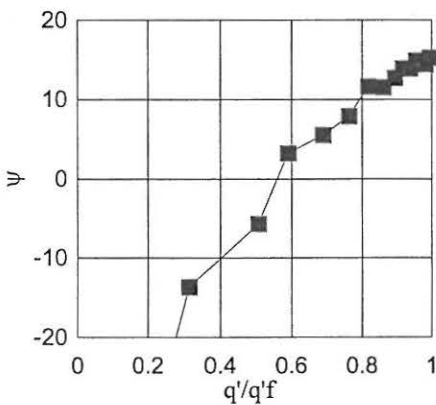
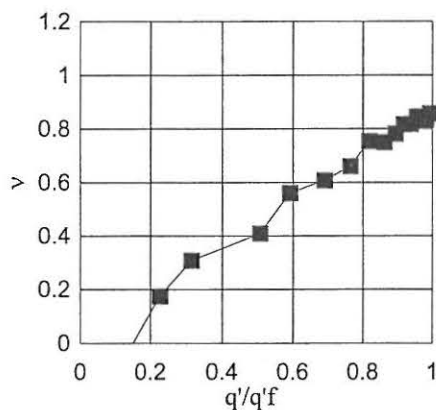


Job:	Encl. No
Lund No 0	26
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
Calibration file	Date	Grain density	2.65	0.654
		Void ratio	0.613	
		Saturation		
		Dimension H mm	71.5	
kal_tri4	17.10.88	D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-80 kPa
		ϵ_1	-0.014 %
		ϵ_v	-0.080 %
	2. Drained compression.	Deformation rate: 7.9 % ph	

		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	297.51	kPa	150.92	kPa
Mean normal stress	p'	179.43	kPa	130.57	kPa
Confining pressures	σ_3	80.26	kPa	80.26	kPa
Vertical strain	ϵ_1	5.19	%	0.47	%
Volumetric strain	ϵ_v	-2.52	%	0.20	%

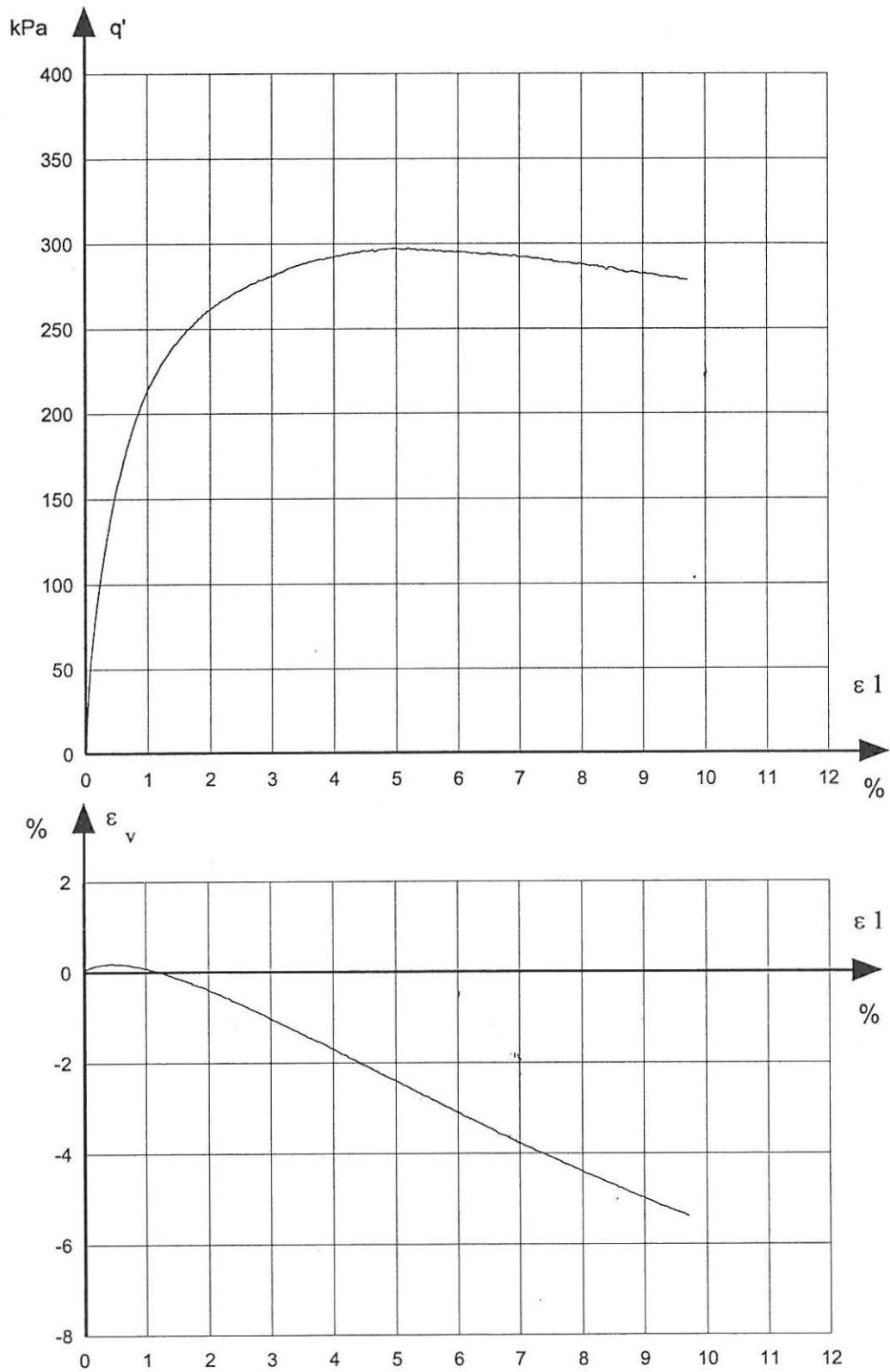


q'	p'	ϵ_1	ϵ_v
0.60	80.21	0.00	0.00
9.66	83.35	0.01	0.03
39.04	93.02	0.05	0.08
67.66	102.43	0.12	0.12
93.48	111.17	0.20	0.15
150.92	130.57	0.47	0.20
175.95	138.78	0.63	0.18
205.12	148.51	0.90	0.12
227.34	155.91	1.18	0.03
243.41	161.14	1.49	-0.12
256.33	165.58	1.82	-0.28
265.85	168.62	2.15	-0.47
272.83	170.82	2.49	-0.69
278.77	172.80	2.84	-0.91
284.17	174.73	3.20	-1.15
288.73	176.25	3.56	-1.39
291.88	177.17	3.92	-1.63
295.47	178.24	4.65	-2.16
297.51	179.43	5.19	-2.52
295.08	178.24	6.11	-3.17
292.69	177.44	6.85	-3.67
289.69	176.44	7.59	-4.13
286.68	175.69	8.33	-4.59
282.27	173.97	9.07	-5.02

Job:	Encl. No
Lund No 0	27
Exc:	Check:
LBI	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1$ 0.074
Saturation not measured.

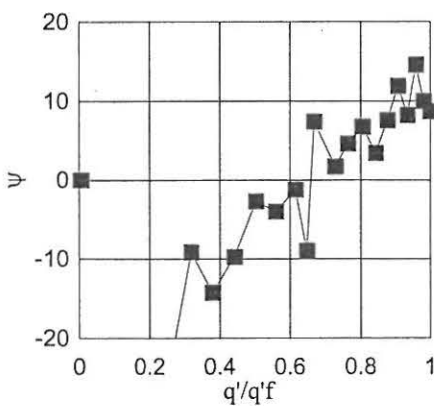
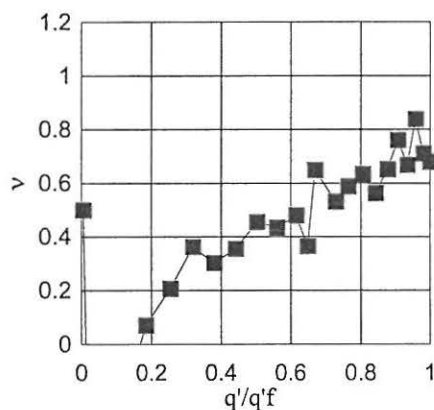


Job:	Encl. No
Lund No 0	28
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	25.0	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.610		
kal_tri4	18.10.88	Saturation	1.09		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.041 %
		ϵ_v	0.072 %
	2. Drained compression.		
		Deformation rate:	8.4 % ph

		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'		331.12 kPa
Mean normal stress	p'		270.77 kPa
Confining pressures	σ_3		160.39 kPa
Vertical strain	ϵ_1		0.73 %
Volumetric strain	ϵ_v		0.29 %



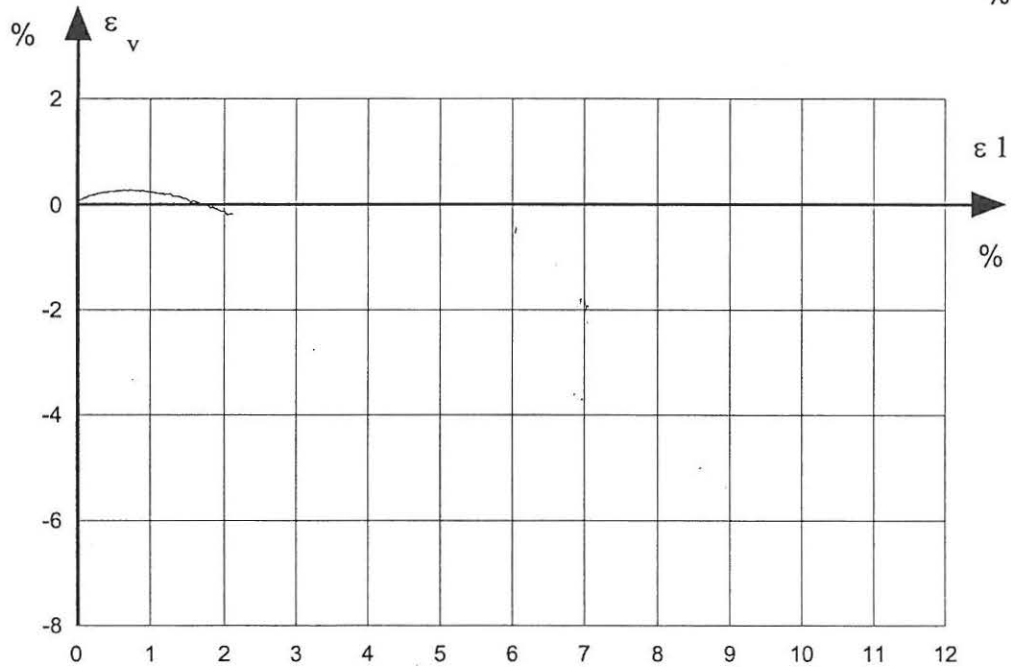
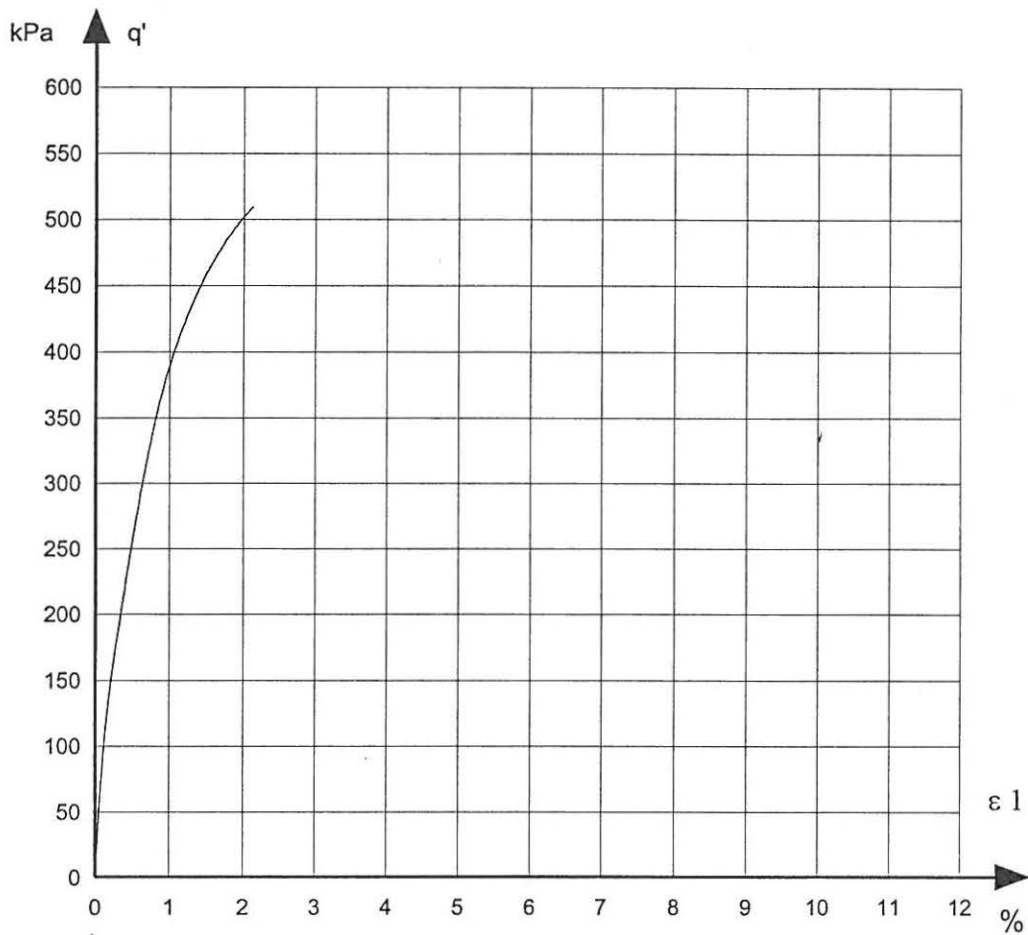
q'	p'	ϵ_1	ϵ_v
-0.05	160.12	0.00	0.00
2.60	161.13	0.00	0.00
16.53	165.65	0.01	0.04
55.43	178.49	0.05	0.09
93.81	191.41	0.10	0.14
129.75	203.52	0.16	0.17
162.48	214.17	0.23	0.19
193.73	224.71	0.32	0.22
225.42	235.53	0.40	0.25
255.98	245.59	0.49	0.26
285.48	255.68	0.58	0.27
314.25	265.02	0.68	0.27
331.12	270.77	0.73	0.29
341.81	274.20	0.78	0.28
371.69	284.04	0.91	0.27
389.98	290.26	1.00	0.25
410.83	297.21	1.12	0.22
429.66	303.36	1.25	0.20
446.92	309.24	1.38	0.16
462.12	314.30	1.52	0.09
475.85	318.63	1.66	0.04
488.47	322.96	1.81	-0.06
499.95	326.92	1.96	-0.12
509.78	330.07	2.12	-0.18

Job:	Encl. No
Lund No 0	29
Exc:	Check:
LBI	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1$ 0.072

The test was stopped before failure.

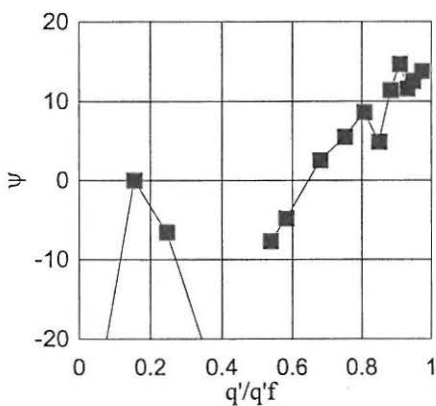
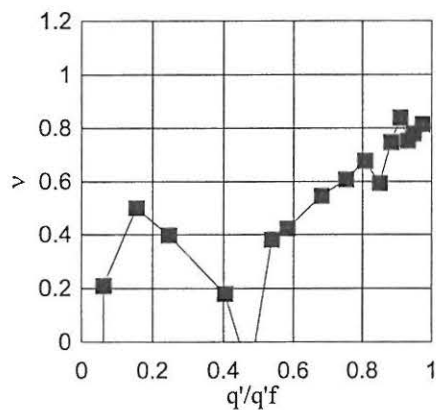


Job:	Encl. No
Lund No 0	30
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.4	
Calibration file	Date	Void ratio	2.65	
		Saturation	0.614	
		Dimension H mm	1.05	
kal_tri3	19.10.88	D mm	71.5	
			69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.036 %
		ϵ_v	0.168 %
	2. Drained compression.	Deformation rate: 7.3 % ph	

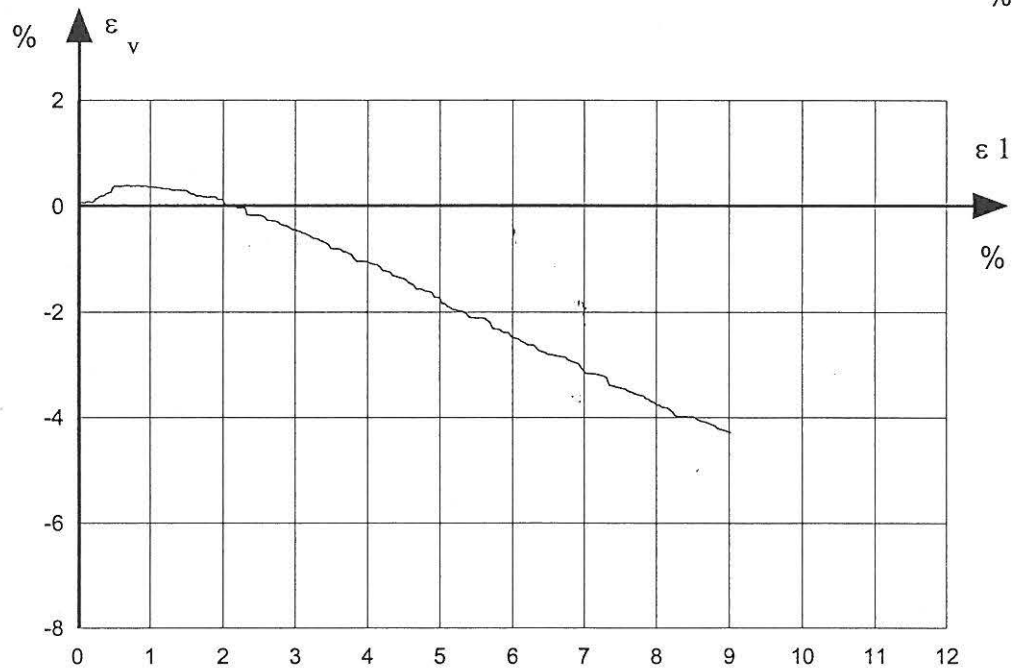
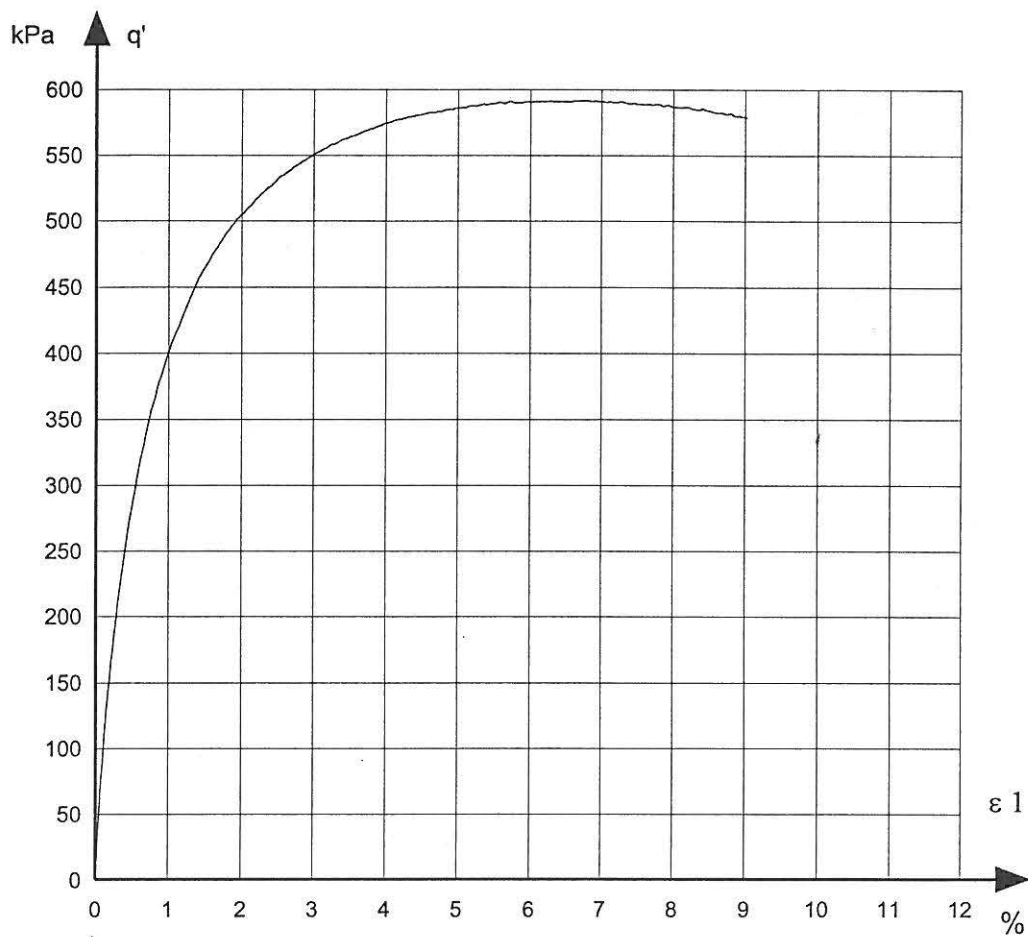
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	592.09	kPa	345.10	kPa
Mean normal stress	p'	357.63	kPa	275.81	kPa
Confining pressures	σ_3	160.27	kPa	160.78	kPa
Vertical strain	ϵ_1	6.76	%	0.70	%
Volumetric strain	ϵ_v	-2.90	%	0.40	%



q'	p'	ϵ_1	ϵ_v
-1.21	159.99	0.00	0.00
2.57	161.25	0.00	0.05
36.41	171.51	0.03	0.07
91.05	192.02	0.09	0.07
145.47	209.39	0.17	0.08
239.47	241.11	0.37	0.21
281.02	255.21	0.48	0.35
318.43	267.81	0.60	0.38
345.10	275.81	0.70	0.40
403.36	294.97	1.01	0.37
445.38	309.87	1.31	0.30
477.83	320.94	1.63	0.19
502.93	328.68	1.96	0.13
522.16	335.59	2.30	-0.04
537.89	341.22	2.64	-0.27
550.76	343.21	2.99	-0.44
560.74	346.79	3.34	-0.64
575.66	353.17	4.06	-1.09
582.83	356.07	4.59	-1.45
589.84	356.50	5.50	-2.11
591.40	357.14	6.22	-2.61
592.09	357.63	6.76	-2.90
589.27	357.07	7.67	-3.52
585.93	356.47	8.40	-3.97

Job:	Encl. No
Lund No 0	31
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.067

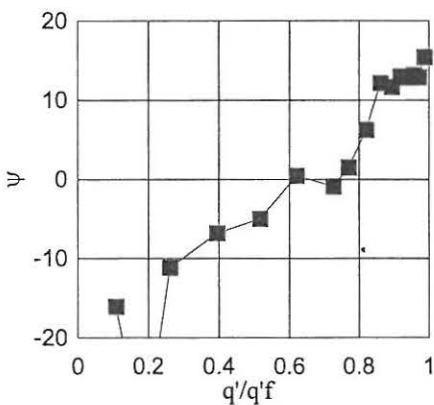
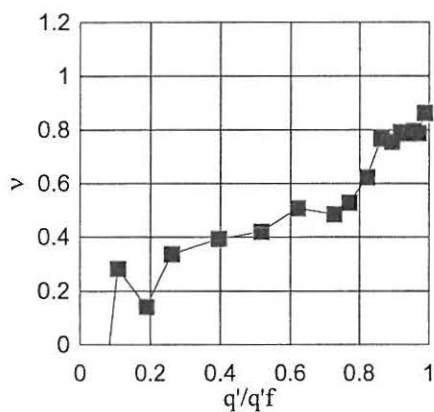


Job:	Encl. No
Lund No 0	32
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.1	
Calibration file	Date	Void ratio	0.614	0.649
kal_tri3	19.10.88	Saturation	1.05	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.034 %
		ϵ_v	0.120 %
	2. Drained compression.		
Deformation rate:		8.0 % ph	

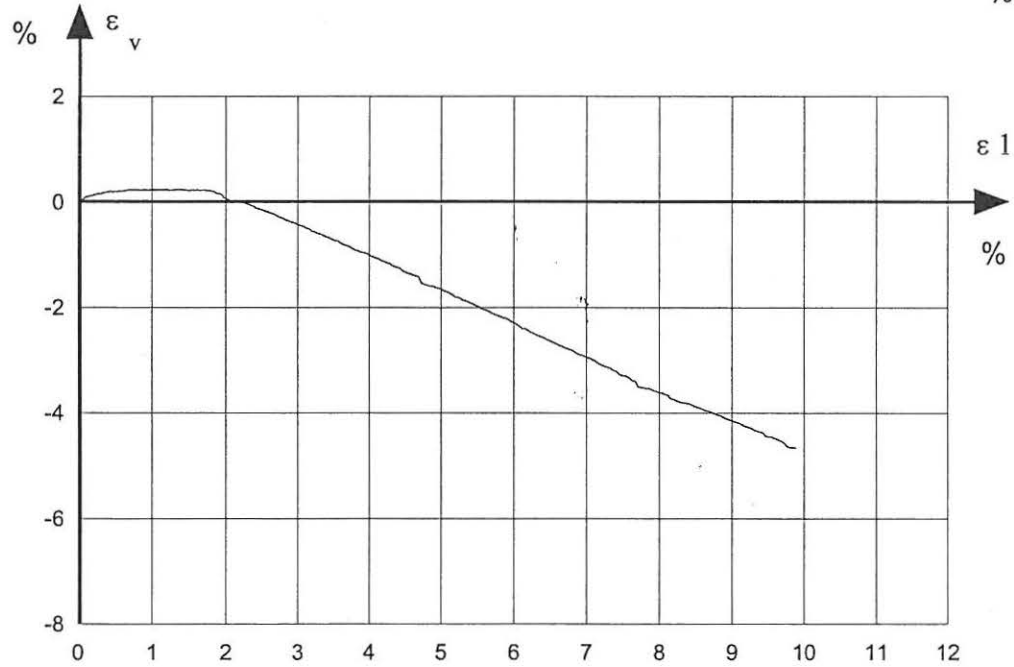
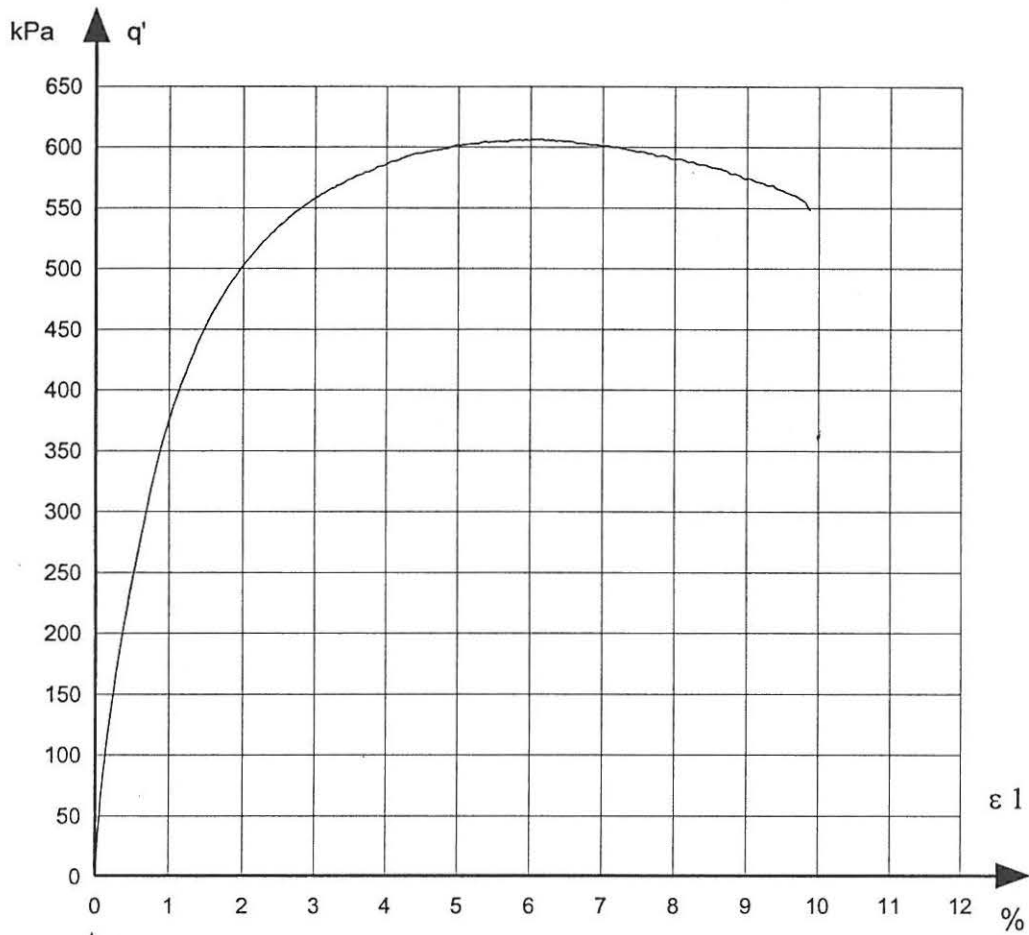
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	606.84 kPa	441.19 kPa
Mean normal stress	p'	363.31 kPa	308.22 kPa
Confining pressures	σ_3	161.03 kPa	161.16 kPa
Vertical strain	ϵ_1	6.15 %	1.39 %
Volumetric strain	ϵ_v	-2.39 %	0.24 %



q'	p'	ϵ_1	ϵ_v
0.00	160.90	0.00	0.00
17.96	166.89	0.01	0.03
65.19	182.38	0.07	0.06
114.21	198.98	0.15	0.12
159.22	213.85	0.25	0.15
239.91	241.00	0.48	0.20
313.45	265.26	0.73	0.24
376.83	286.51	1.00	0.23
441.19	308.22	1.39	0.24
467.83	316.85	1.61	0.23
498.50	327.07	1.94	0.15
523.15	335.54	2.28	-0.03
542.23	341.65	2.63	-0.20
557.39	346.83	2.98	-0.41
569.42	350.58	3.34	-0.61
580.24	354.57	3.70	-0.83
588.60	357.49	4.06	-1.03
599.04	360.71	4.79	-1.56
604.98	362.56	5.52	-1.98
606.84	363.31	6.15	-2.39
601.00	361.11	7.00	-2.94
596.67	359.67	7.55	-3.30
584.14	355.36	8.48	-3.86
570.25	350.99	9.23	-4.27

Job:	Encl. No
Lund No 0	33
Exc:	Check:
LBI	FRJ

Remark:	
Preparation [%]	$\Delta \epsilon_1$ 0.085

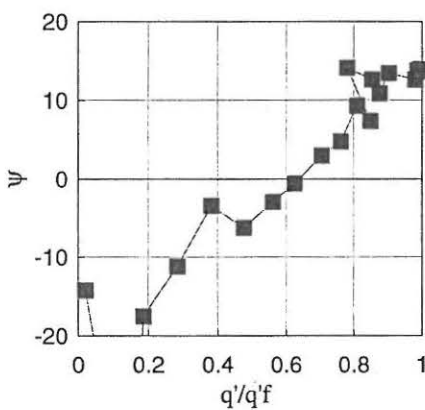
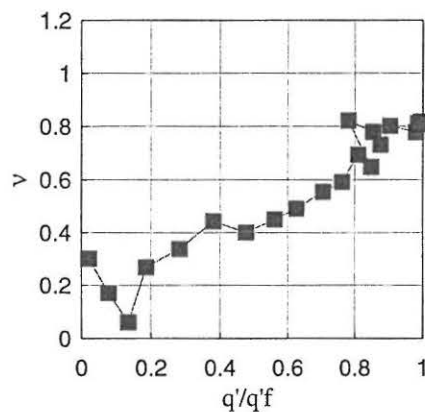


Job:	Encl. No
Lund No 0	34
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.6	0.649
Calibration file	Date	Void ratio	2.65	
kal_tri3	21.10.88	Saturation	0.609	
		Dimension H mm	1.07	
		D mm	71.5	
			69.7	

TEST-PROGRAM	Drained compression.			
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-320 kPa	
		ϵ_1	0.127 %	
		ϵ_v	0.028 %	
	2. Drained compression.			
Deformation rate:			7.4 % ph	

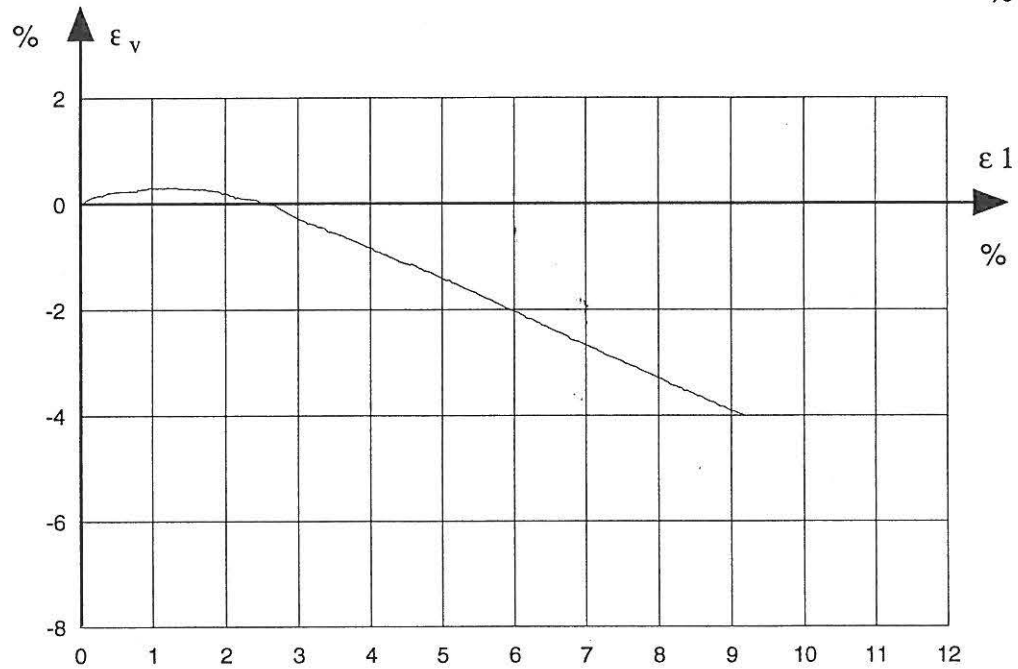
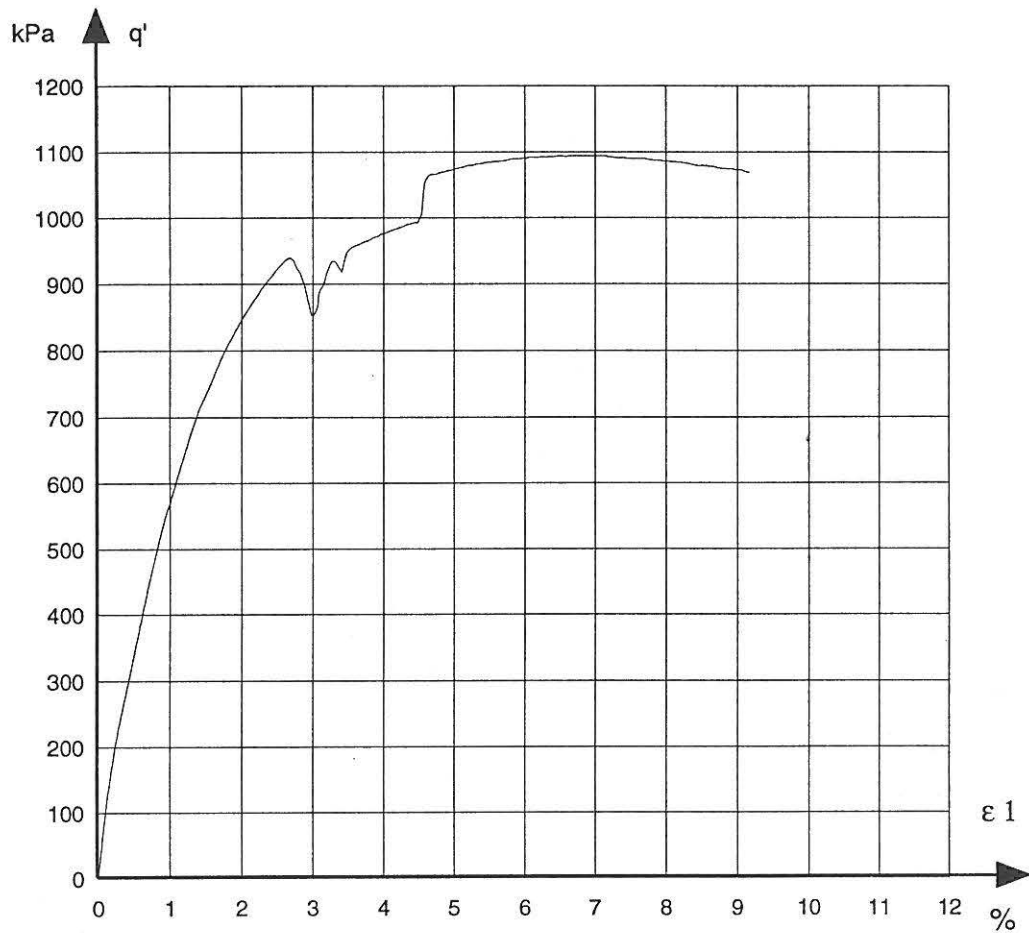
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 094.33	kPa	683.83	kPa
Mean normal stress	p'	683.91	kPa	546.81	kPa
Confining pressures	σ_3	319.13	kPa	318.87	kPa
Vertical strain	ϵ_1	6.74	%	1.32	%
Volumetric strain	ϵ_v	-2.50	%	0.31	%



q'	p'	ϵ_1	ϵ_v
0.00	318.49	0.00	0.00
22.75	327.09	0.02	0.01
84.80	347.14	0.08	0.05
146.99	368.25	0.15	0.11
203.52	386.84	0.24	0.15
309.23	422.08	0.44	0.22
416.39	457.67	0.65	0.24
521.08	492.82	0.87	0.28
613.32	523.44	1.12	0.31
683.83	546.81	1.32	0.31
771.36	576.12	1.65	0.28
833.57	596.73	1.93	0.22
885.64	614.21	2.23	0.11
928.10	628.11	2.54	0.02
853.86	553.09	3.01	-0.28
933.71	604.97	3.27	-0.43
958.02	614.23	3.61	-0.59
988.25	623.15	4.30	-1.01
1071.93	676.57	4.92	-1.35
1085.85	680.82	5.63	-1.79
1092.83	683.28	6.35	-2.25
1094.33	683.91	6.74	-2.50
1087.90	681.63	7.80	-3.17
1079.32	679.03	8.54	-3.63

Job:	Encl. No
Lund No 0	35
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = 0.076$

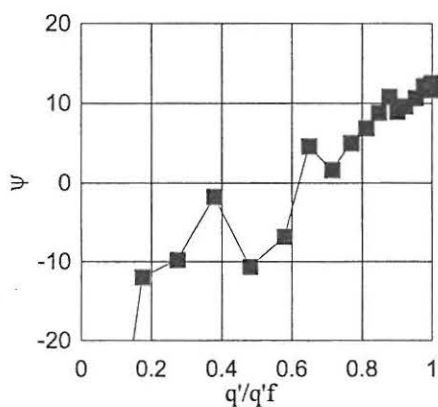
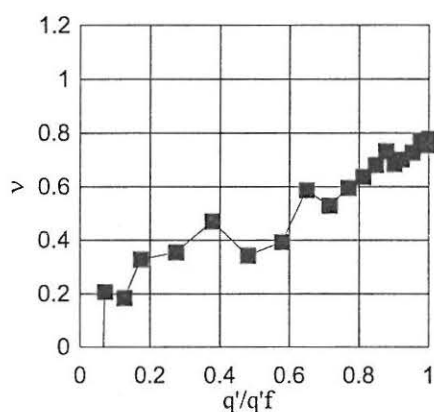


Job:	Encl. No
Lund No 0	36
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	25.6	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.612		
kal_tri3	25.10.89	Saturation	1.11		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-320 kPa
		ϵ_1	0.132 %
		ϵ_v	0.068 %
	2. Drained compression.	Deformation rate: 7.5 % ph	

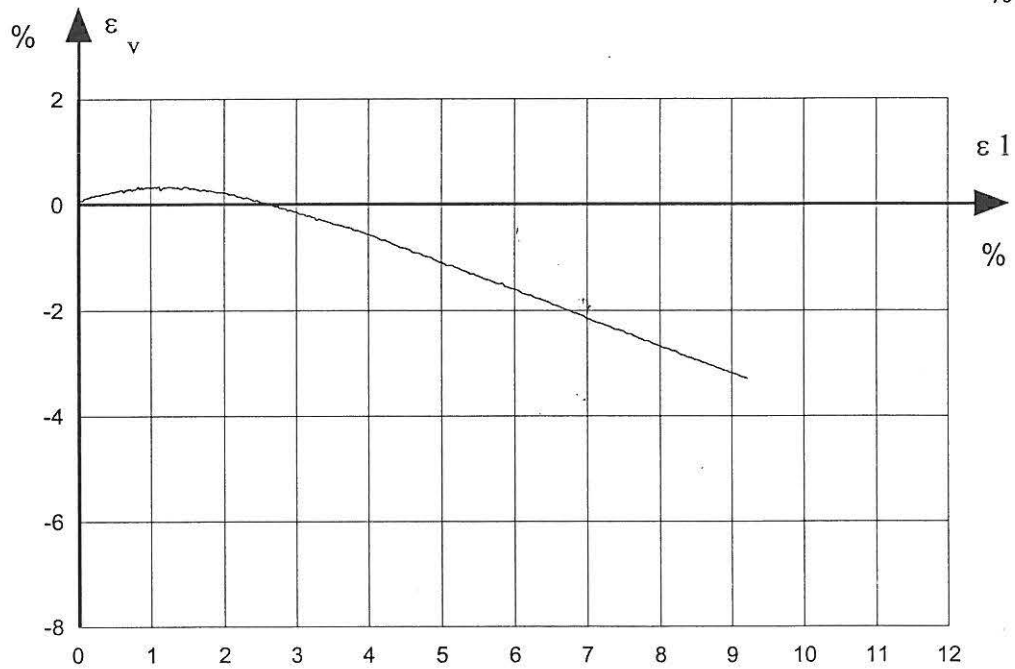
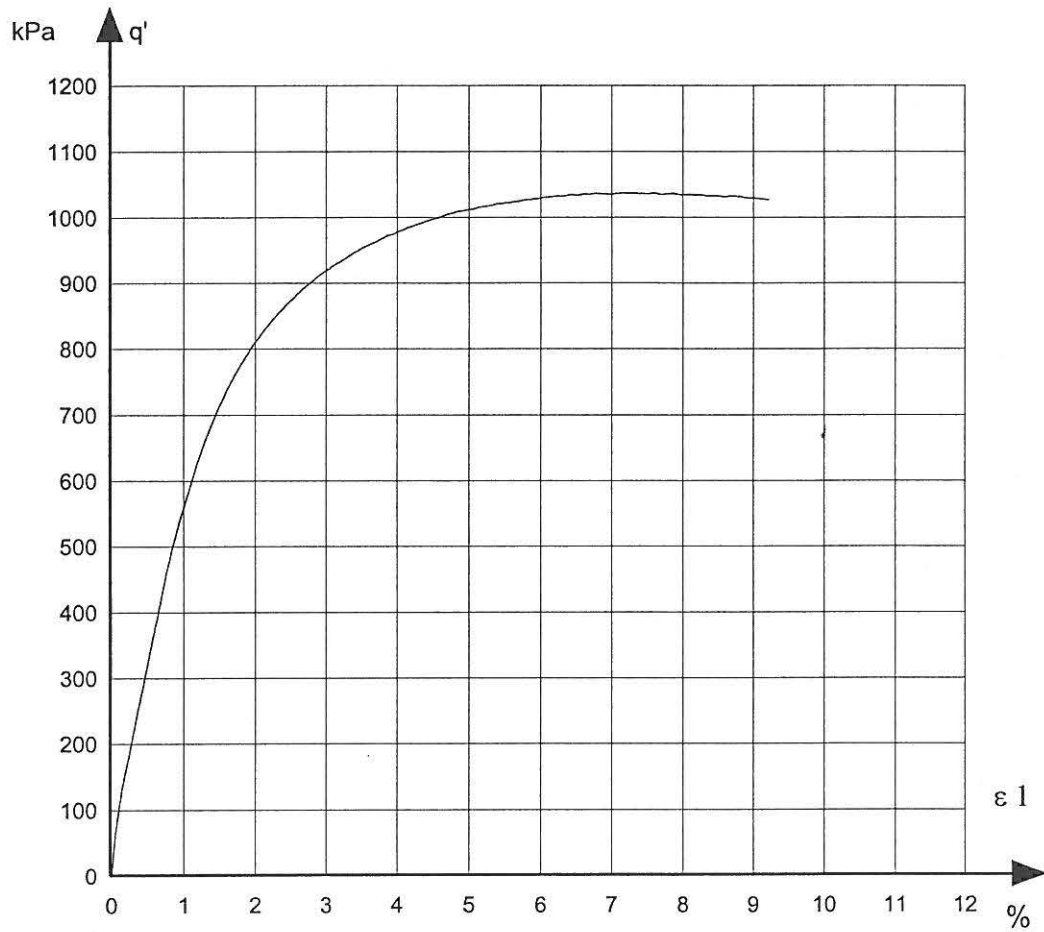
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	1 037.52	kPa	600.56	kPa
Mean normal stress	p'	664.46	kPa	519.06	kPa
Confining pressures	σ_3	318.62	kPa	318.87	kPa
Vertical strain	ϵ_1	7.16	%	1.11	%
Volumetric strain	ϵ_v	-2.23	%	0.35	%



q'	p'	ϵ_1	ϵ_v
0.00	316.96	0.00	0.00
11.00	322.54	0.01	0.05
72.80	342.88	0.06	0.08
129.73	361.86	0.14	0.13
179.79	378.68	0.23	0.16
284.81	413.55	0.42	0.22
393.07	449.64	0.63	0.23
498.58	484.81	0.84	0.30
600.56	519.06	1.11	0.35
674.07	543.18	1.34	0.31
742.54	566.00	1.62	0.30
797.97	584.48	1.91	0.24
842.57	599.35	2.22	0.16
879.21	611.30	2.55	0.04
910.13	621.99	2.87	-0.11
934.34	629.68	3.21	-0.24
956.99	637.74	3.55	-0.37
989.18	648.47	4.25	-0.69
1012.30	656.05	5.00	-1.09
1024.79	659.96	5.68	-1.44
1035.06	663.76	6.40	-1.80
1037.52	664.46	7.16	-2.23
1036.62	664.41	7.86	-2.58
1031.95	662.47	8.59	-2.98

Job:	Encl. No
Lund No 0	37
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.098

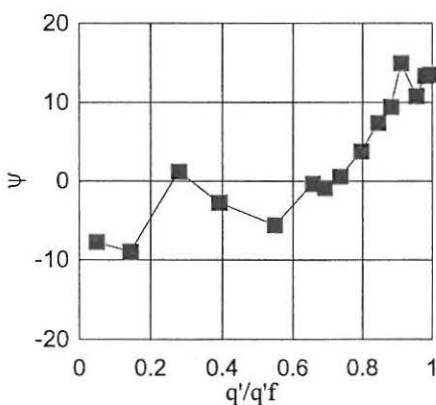
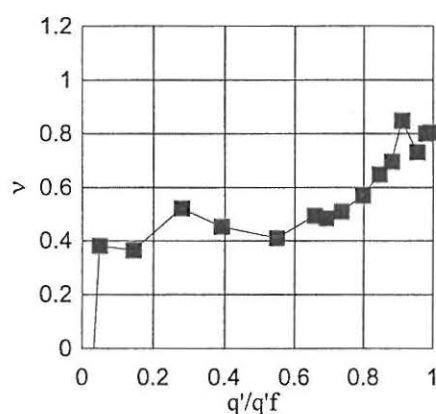


Job:	Encl. No
Lund No 0	38
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	25.3	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.615		
		Saturation	1.09		
		Dimension H mm	71.5		
kal_tri3		D mm	69.7		

TEST-PROGRAM	Drained compression.			
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-46 kPa	
		ϵ_1	-0.056 %	
		ϵ_v	0.132 %	
	2. Drained compression.			
Deformation rate:			7.8 % ph	

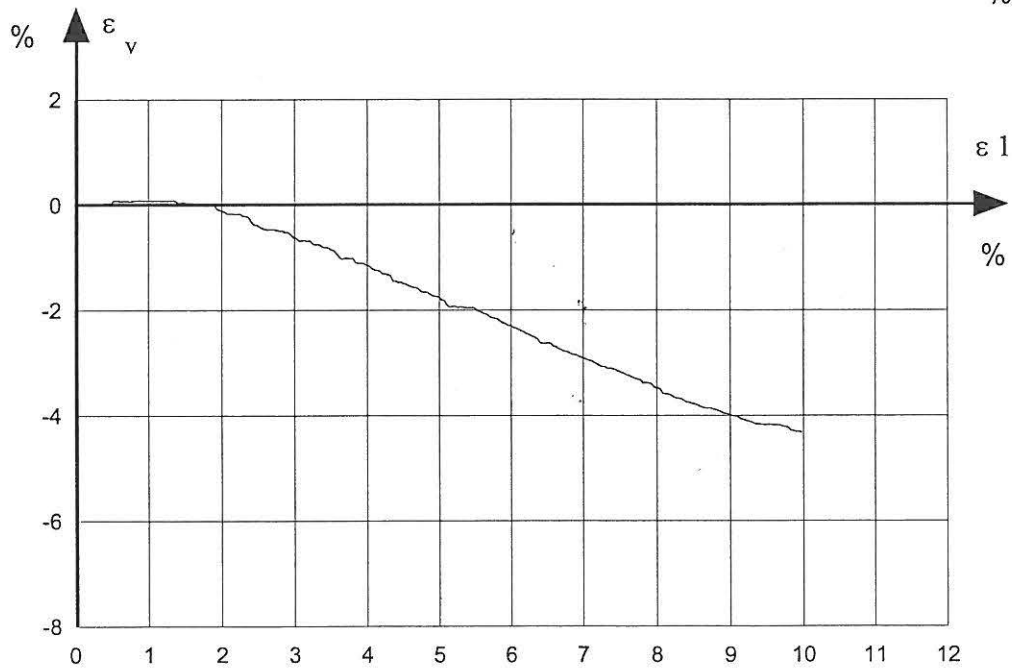
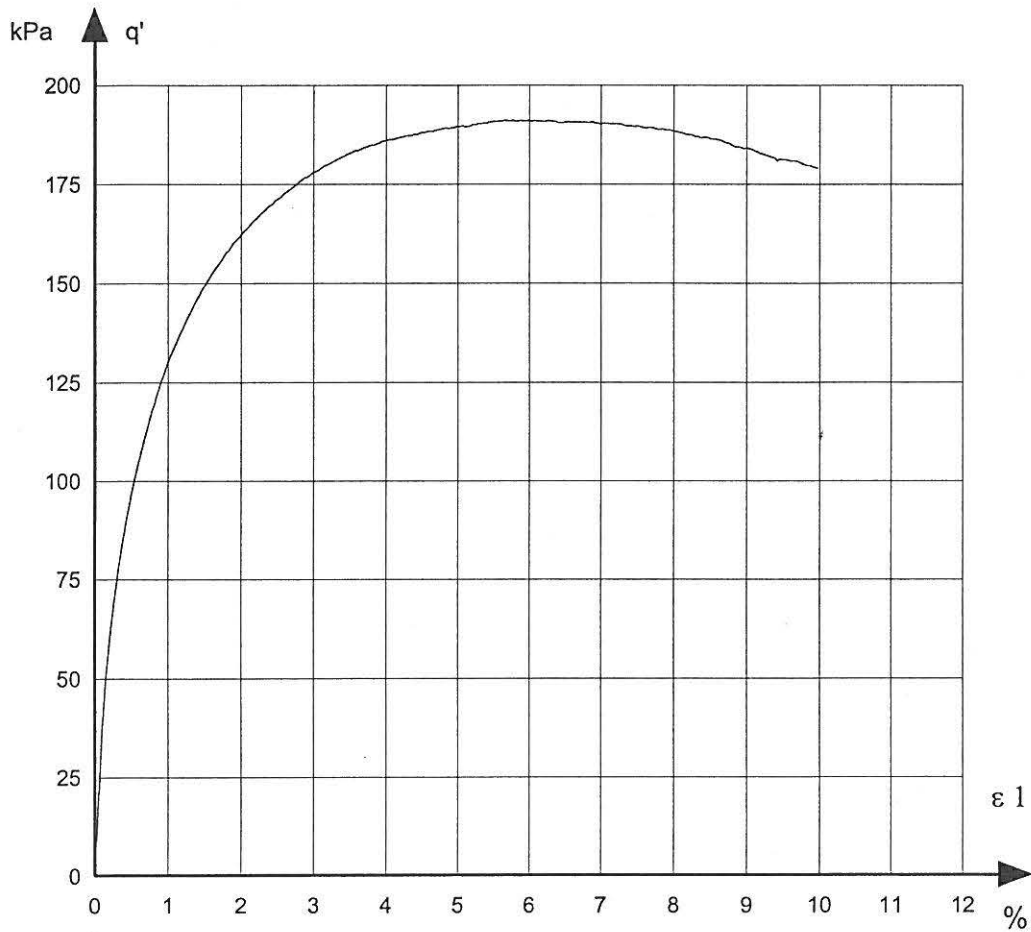
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	190.63	kPa	131.89	kPa
Mean normal stress	p'	110.63	kPa	90.92	kPa
Confining pressures	σ_3	47.08	kPa	46.96	kPa
Vertical strain	ϵ_1	6.40	%	1.03	%
Volumetric strain	ϵ_v	-2.62	%	0.09	%



q'	p'	ϵ_1	ϵ_v
0.00	46.96	0.00	0.00
3.78	48.22	0.00	0.01
9.22	50.03	0.02	0.01
27.34	55.94	0.07	0.02
53.27	64.71	0.16	0.02
74.61	71.83	0.29	0.03
104.50	81.79	0.58	0.08
125.31	88.73	0.90	0.09
131.89	90.92	1.03	0.09
140.42	93.76	1.23	0.09
151.92	97.60	1.58	0.04
160.99	100.75	1.93	-0.06
167.99	102.95	2.29	-0.20
173.53	104.80	2.65	-0.46
181.93	107.60	3.39	-0.79
186.61	109.16	4.13	-1.24
189.14	110.13	4.87	-1.69
191.04	110.76	5.62	-2.06
190.99	110.75	6.36	-2.54
190.48	110.58	7.10	-2.96
189.01	110.09	7.85	-3.37
186.39	109.22	8.59	-3.83
181.93	107.73	9.33	-4.15
179.16	106.81	9.97	-4.33

Job:	Encl. No
Lund No 0	39
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.066
Error during saturation.

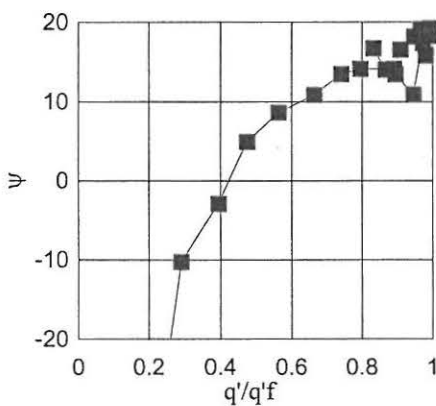
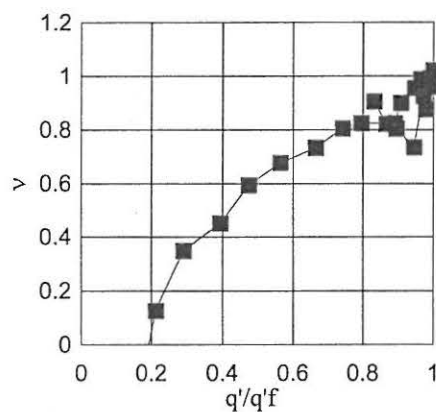


Job:	Encl. No
Lund No 0	40
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.8	0.628
Calibration file	Date	Void ratio	2.65	
kal200_891	08.06.89	Saturation	0.576	
		Dimension H mm	1.09	
		D mm	71.5	
			69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	10-5 kPa
		ϵ_1	0.003 %
		ϵ_v	0.162 %
	2. Drained compression.		
Deformation rate:			8.9 % ph

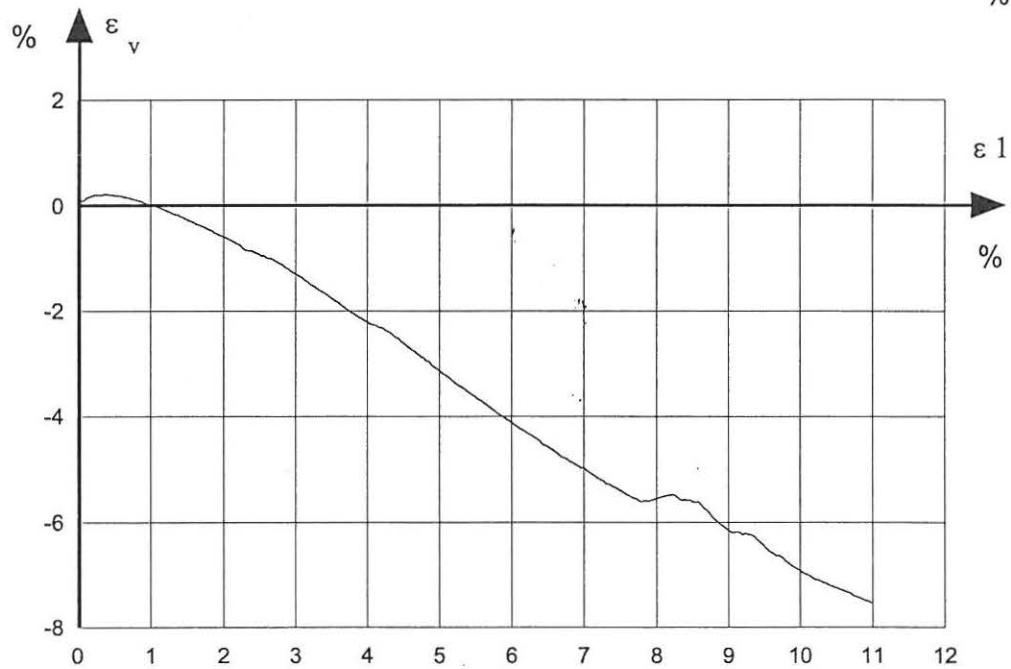
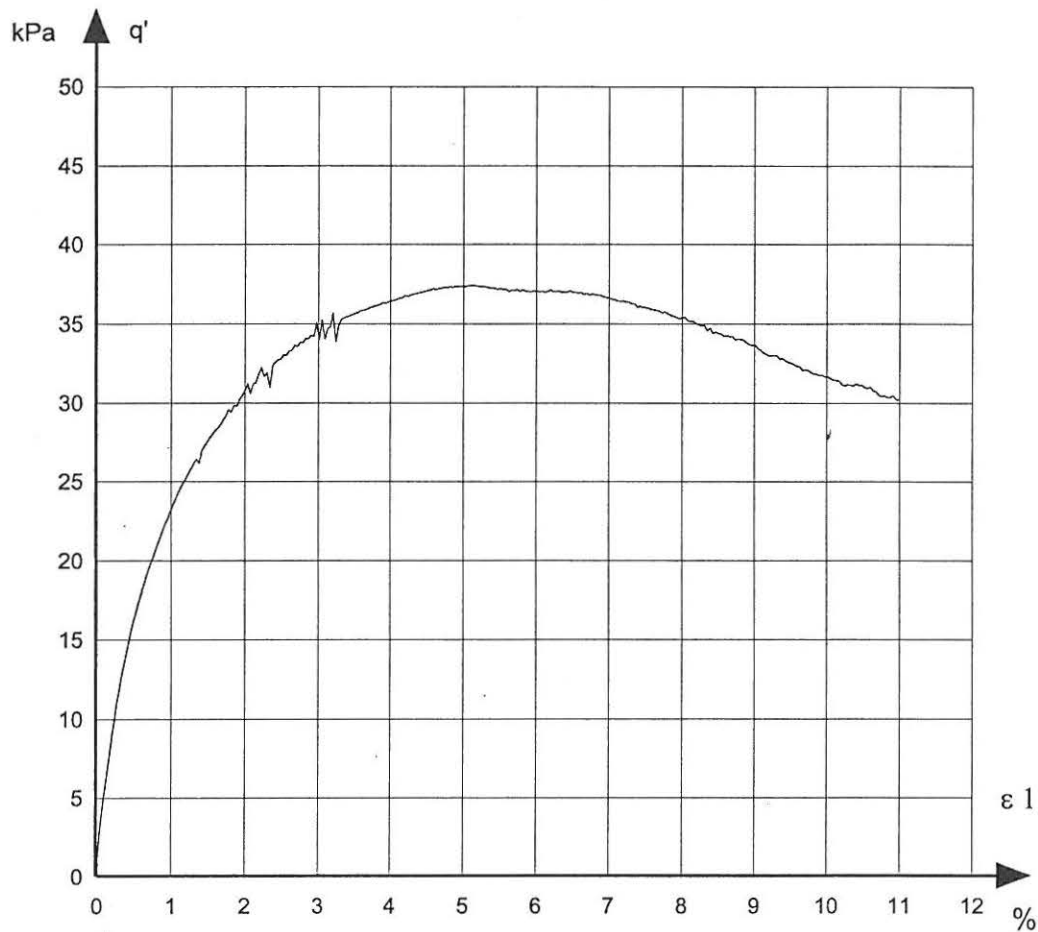
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	37.44	kPa	14.75	kPa
Mean normal stress	p'	17.23	kPa	9.67	kPa
Confining pressures	σ_3	4.75	kPa	4.75	kPa
Vertical strain	ϵ_1	5.12	%	0.41	%
Volumetric strain	ϵ_v	-3.25	%	0.22	%



q'	p'	ϵ_1	ϵ_v
0.00	4.75	0.00	0.00
2.68	5.52	0.03	0.08
7.95	7.40	0.17	0.18
10.88	8.38	0.25	0.21
14.75	9.67	0.41	0.22
17.75	10.67	0.57	0.19
21.11	11.79	0.81	0.11
24.90	13.05	1.16	-0.05
27.79	13.89	1.52	-0.27
29.79	40.58	1.88	-0.51
33.33	15.86	2.63	-0.99
33.97	28.46	3.01	-1.30
35.46	16.57	3.39	-1.64
36.14	16.80	3.78	-2.02
36.66	16.97	4.16	-2.30
37.14	17.13	4.54	-2.65
37.44	17.23	5.12	-3.25
37.11	17.25	5.69	-3.82
37.10	17.12	6.46	-4.54
36.39	16.88	7.23	-5.19
35.39	16.55	7.99	-5.55
33.48	15.91	9.04	-6.18
32.46	15.57	9.55	-6.51
31.15	15.14	10.32	-7.14

Job:	Encl. No
Lund No 0	41
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ -0.315

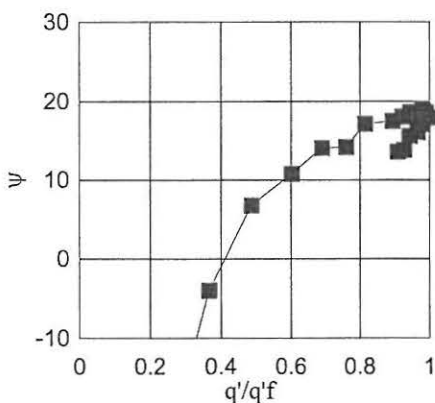
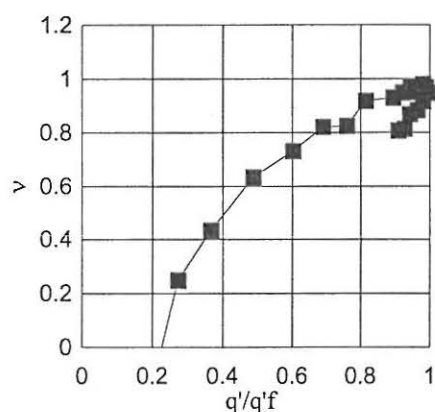


Job:	Lund No 0	Encl. No	42
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	22.7	At failure 0.647
		Grain density	2.65	
Calibration file	Date	Void ratio	0.577	
kal200_891	09.06.89	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-5 kPa
		ϵ_1	-0.136 %
		ϵ_v	-0.691 %
	2. Drained compression.	Deformation rate: 9.0 % ph	

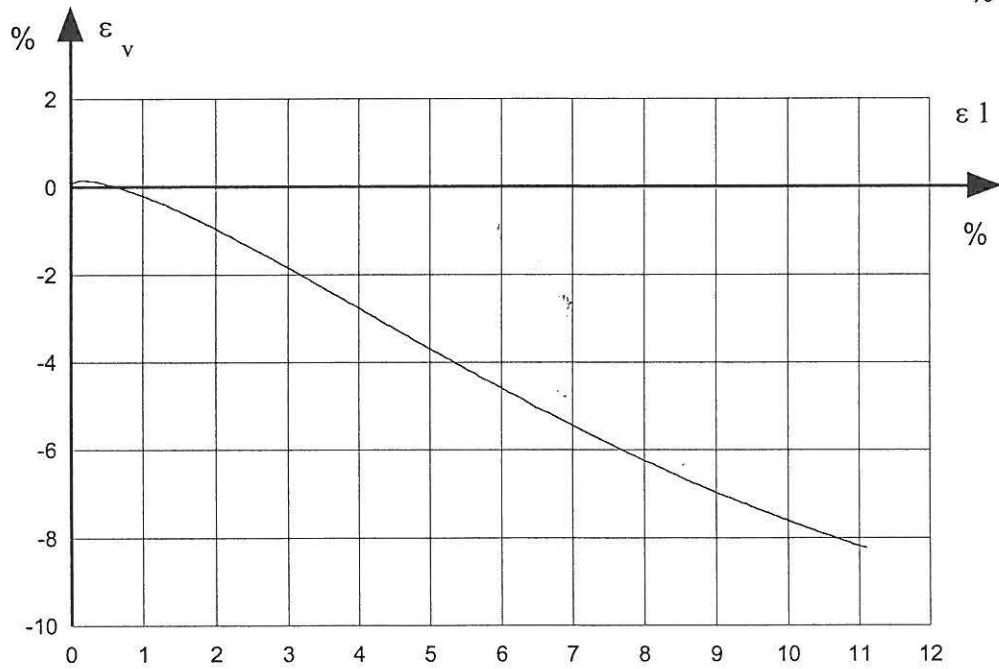
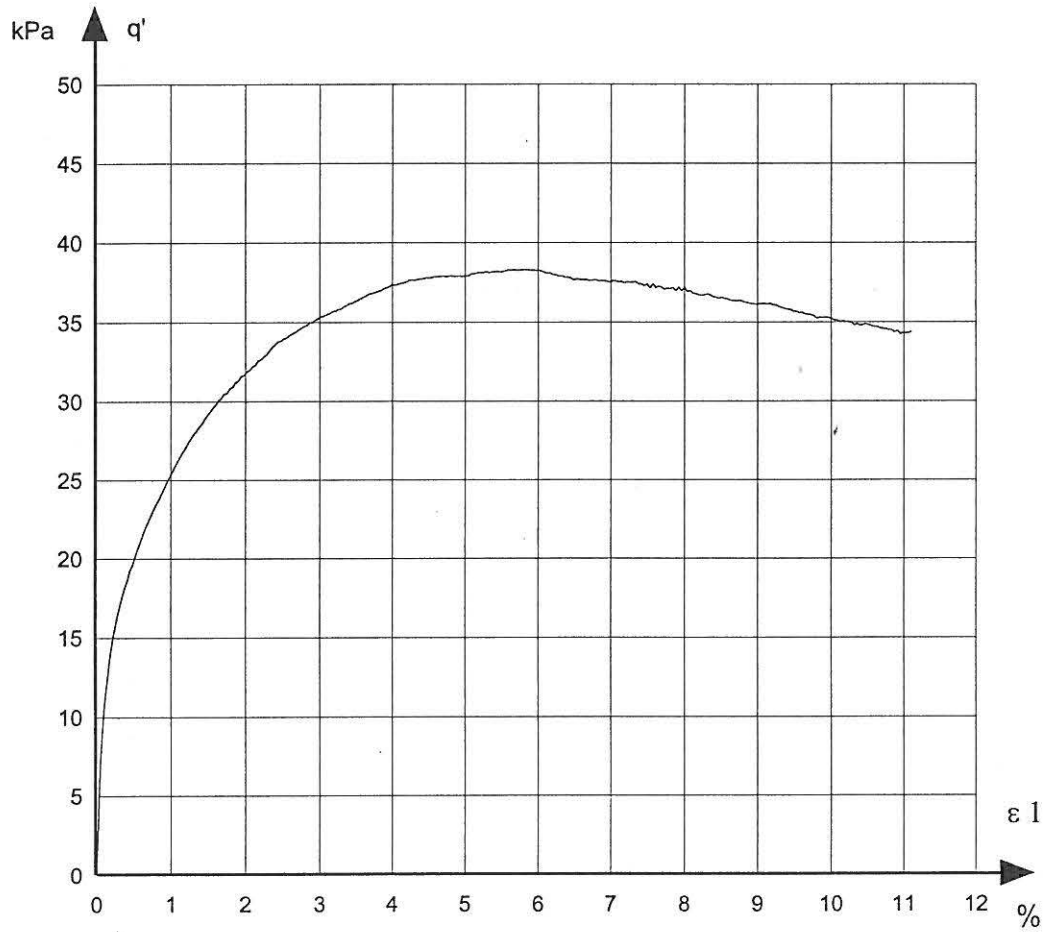
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	38.34 kPa	14.06 kPa
Mean normal stress	p'	17.66 kPa	9.57 kPa
Confining pressures	σ_3	4.88 kPa	4.88 kPa
Vertical strain	ϵ_1	5.82 %	0.19 %
Volumetric strain	ϵ_v	-4.42 %	0.16 %



q'	p'	ϵ_1	ϵ_v
-0.05	4.74	0.00	0.00
1.45	5.36	0.01	0.06
7.01	7.22	0.05	0.12
10.52	8.38	0.10	0.15
14.06	9.57	0.19	0.16
18.66	11.10	0.41	0.10
23.08	12.57	0.76	-0.06
26.45	13.70	1.12	-0.29
29.14	14.59	1.49	-0.53
31.23	15.16	1.86	-0.84
34.29	16.31	2.63	-1.50
35.37	16.67	3.02	-1.85
36.19	16.94	3.41	-2.21
36.96	17.20	3.79	-2.56
37.56	17.40	4.19	-2.94
37.88	17.51	4.58	-3.30
37.91	17.64	4.97	-3.66
38.34	17.66	5.82	-4.42
37.74	17.58	6.52	-5.04
37.56	17.40	7.29	-5.69
36.99	17.21	8.08	-6.29
36.13	16.92	9.13	-7.06
35.52	16.60	9.64	-7.38
34.85	16.50	10.43	-7.86

Job:	Encl. No
Lund No 0	43
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.082

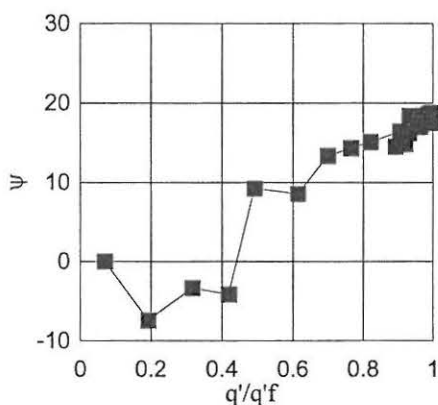
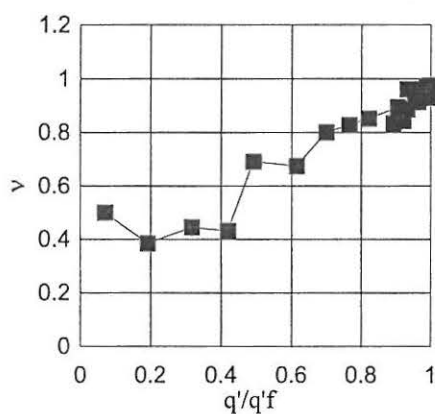


Job:	Encl. No
Lund No 0	44
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.6	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.573		
kal200_891	07.06.89	Saturation	1.05		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-10 kPa
		ϵ_1	-0.110 %
		ϵ_v	-0.279 %
	2.Drained compression.		
	Deformation rate:		8.8 % ph

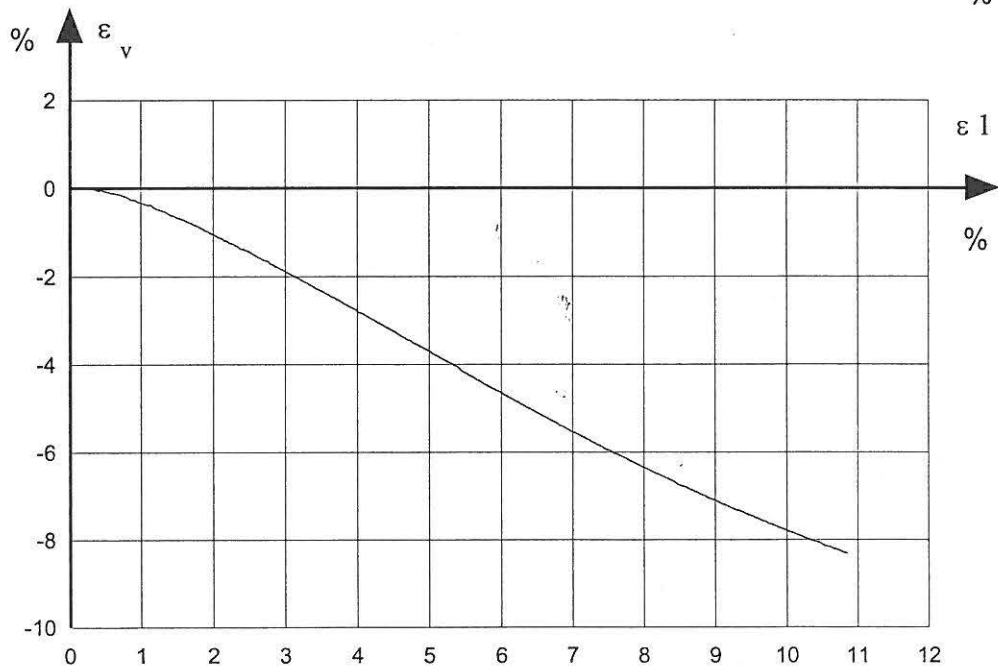
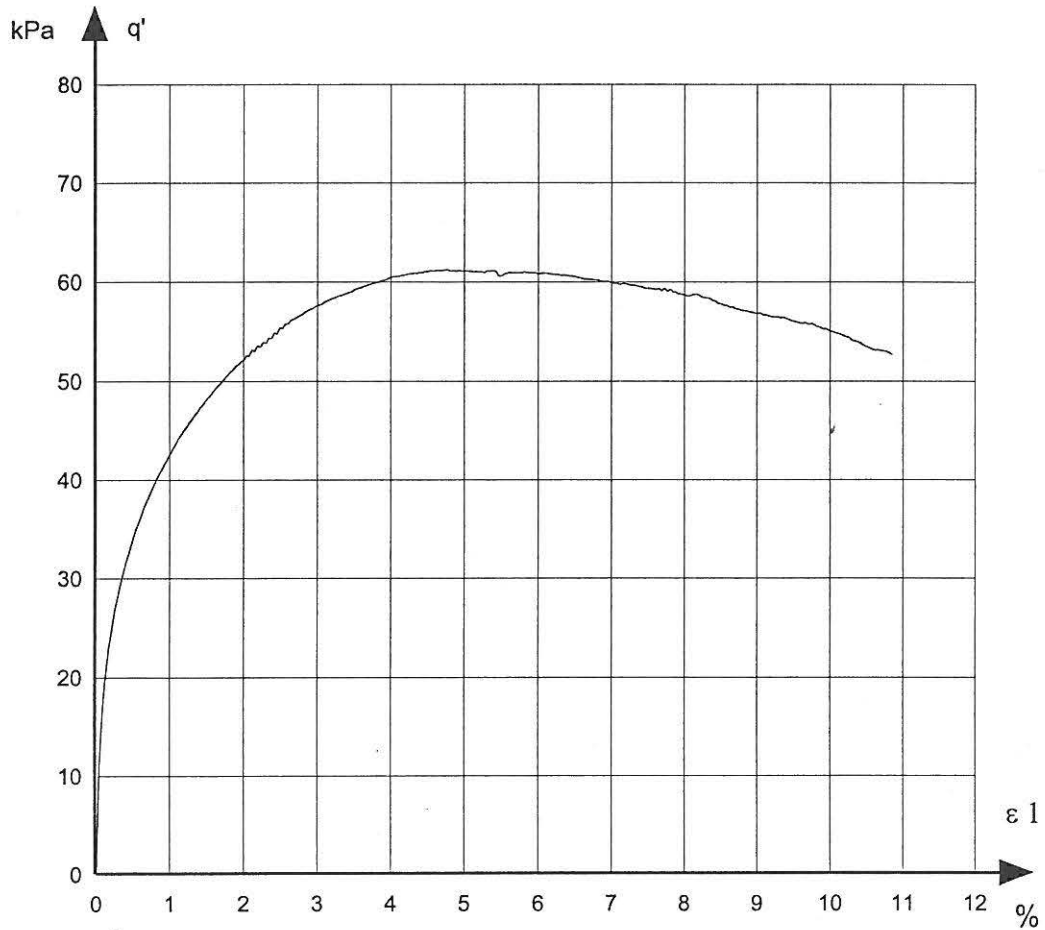
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	61.27	kPa	25.68	kPa
Mean normal stress	p'	30.31	kPa	18.32	kPa
Confining pressures	σ_3	9.88	kPa	9.76	kPa
Vertical strain	ϵ_1	4.77	%	0.23	%
Volumetric strain	ϵ_v	-3.49	%	0.03	%



q'	p'	ϵ_1	ϵ_v
0.00	9.88	0.00	0.00
4.27	11.18	0.01	0.00
11.75	13.68	0.05	0.01
19.43	16.23	0.11	0.02
25.68	18.32	0.23	0.03
30.04	19.90	0.34	-0.02
37.64	22.30	0.67	-0.13
42.88	24.18	1.02	-0.34
46.91	25.52	1.37	-0.57
50.30	26.52	1.74	-0.83
55.42	28.36	2.49	-1.42
57.13	28.80	2.87	-1.76
58.48	29.25	3.24	-2.10
59.56	29.74	3.63	-2.45
60.53	29.93	4.01	-2.81
60.98	30.08	4.39	-3.14
61.27	30.31	4.77	-3.49
60.99	30.21	5.62	-4.29
60.68	30.11	6.30	-4.92
59.89	29.97	7.07	-5.58
59.00	29.42	7.84	-6.22
57.03	28.77	8.89	-7.03
56.37	28.55	9.39	-7.37
54.65	27.97	10.18	-7.90

Job:	Encl. No
Lund No 0	45
Exc:	Check:
LBI, AMS	FRJ

Remark:	
Preparation [%]	$\Delta \epsilon_1$ 0.073

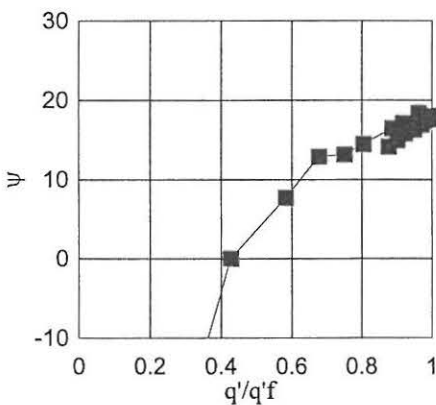
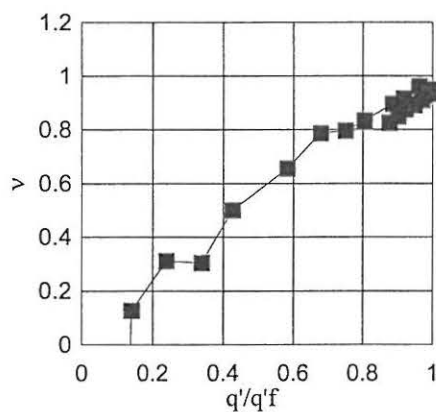


Job:	Encl. No
Lund No 0	46
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.7	At failure 0.636
		Grain density	2.65	
Calibration file	Date	Void ratio	0.575	
		Saturation	1.05	
kal200_891	12.06.89	Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-10 kPa
		ϵ_1	-0.112 %
		ϵ_v	-0.291 %
	2. Drained compression.		
Deformation rate:			8.8 % ph

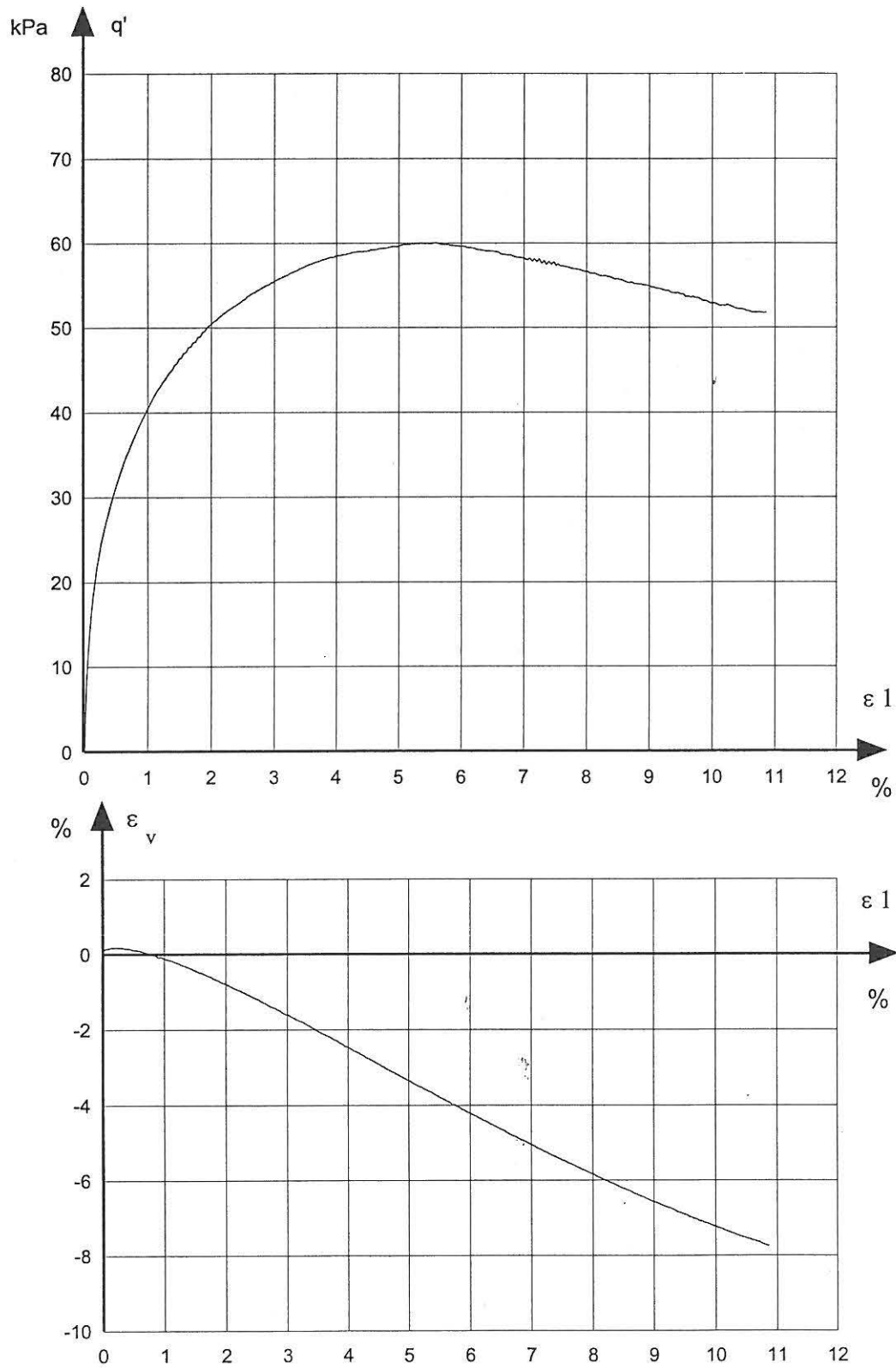
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	60.07 kPa	25.65 kPa
Mean normal stress	p'	29.66 kPa	18.31 kPa
Confining pressures	σ_3	9.63 kPa	9.76 kPa
Vertical strain	ϵ_1	5.59 %	0.30 %
Volumetric strain	ϵ_v	-3.85 %	0.19 %



q'	p'	ϵ_1	ϵ_v
0.00	9.76	0.00	0.00
4.33	11.32	0.01	0.12
8.40	12.56	0.03	0.13
14.38	14.68	0.08	0.15
20.35	16.67	0.17	0.19
25.65	18.31	0.30	0.19
35.01	21.43	0.66	0.07
40.78	23.35	1.01	-0.13
45.09	24.91	1.37	-0.34
48.35	26.00	1.74	-0.59
53.27	27.39	2.49	-1.18
55.06	28.11	2.87	-1.49
56.50	28.59	3.25	-1.79
57.74	29.25	3.63	-2.14
58.60	29.41	4.02	-2.47
59.01	29.43	4.40	-2.82
59.46	29.58	4.78	-3.16
60.07	29.66	5.59	-3.85
59.24	29.38	6.32	-4.49
58.23	28.92	7.09	-5.12
56.93	28.74	7.86	-5.72
55.46	28.24	8.63	-6.30
54.15	27.81	9.42	-6.85
52.63	27.18	10.20	-7.35

Job:	Encl. No
Lund No 0	47
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.078

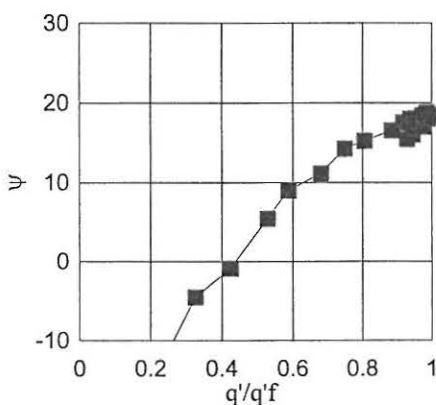
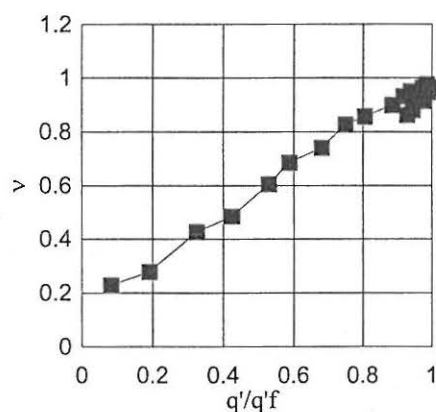


Job:	Encl. No
Lund No 0	48
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.8	At failure 0.633
		Grain density	2.65	
Calibration file	Date	Void ratio	0.568	
kal200_891	01.06.89	Saturation	1.06	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-20 kPa
		ϵ_1	-0.122 %
		ϵ_v	-0.004 %
	2. Drained compression.	Deformation rate: 8.5 % ph	

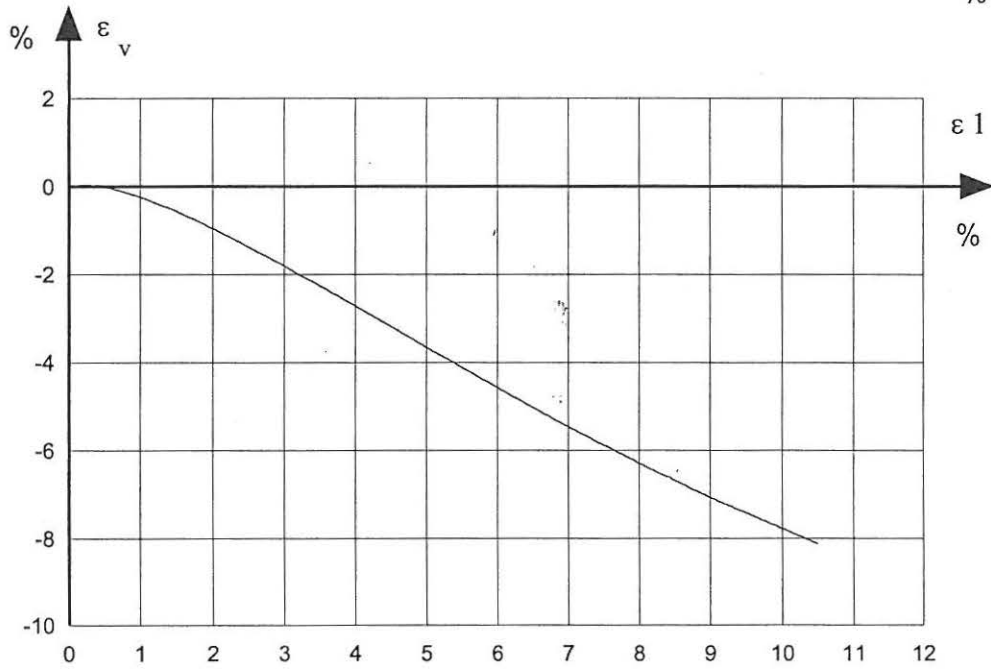
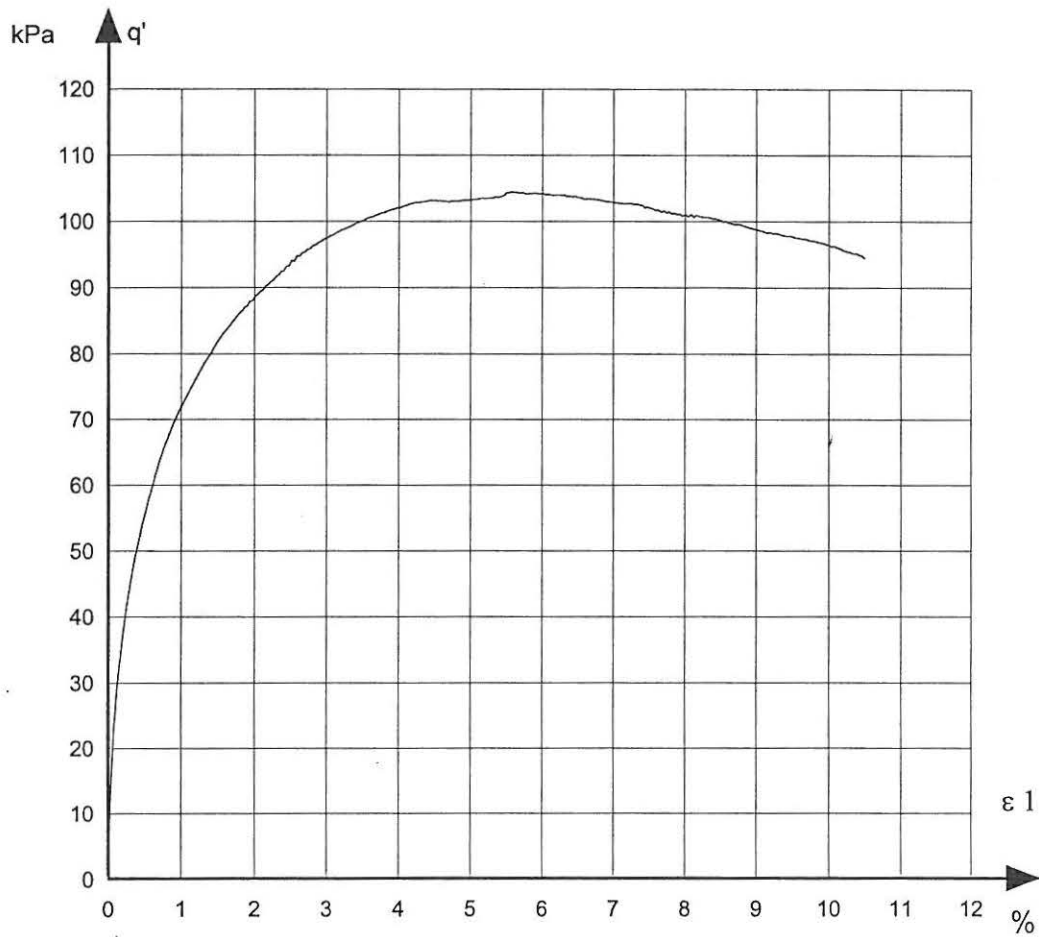
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	104.47	kPa	44.39	kPa
Mean normal stress	p'	55.71	kPa	35.69	kPa
Confining pressures	σ_3	20.89	kPa	20.89	kPa
Vertical strain	ϵ_1	5.57	%	0.28	%
Volumetric strain	ϵ_v	-4.17	%	0.05	%



q'	p'	ϵ_1	ϵ_v
0.05	20.91	0.00	0.00
8.62	23.76	0.01	0.01
19.95	27.54	0.06	0.03
34.02	32.23	0.16	0.04
44.39	35.69	0.28	0.05
55.16	39.28	0.48	0.00
61.35	41.22	0.64	-0.05
71.12	44.72	0.96	-0.21
78.30	46.99	1.30	-0.43
84.18	48.95	1.65	-0.68
92.47	51.72	2.36	-1.25
95.85	52.84	2.73	-1.56
98.06	53.58	3.09	-1.89
99.99	54.22	3.46	-2.21
101.55	54.74	3.83	-2.55
102.83	55.17	4.20	-2.90
103.16	55.40	4.57	-3.25
104.47	55.71	5.57	-4.17
104.08	55.71	6.05	-4.61
103.32	55.33	6.80	-5.28
101.97	54.88	7.55	-5.90
100.63	54.44	8.30	-6.53
98.62	53.76	9.06	-7.11
96.99	53.10	9.83	-7.66

Job:	Encl. No
Lund No 0	49
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.086

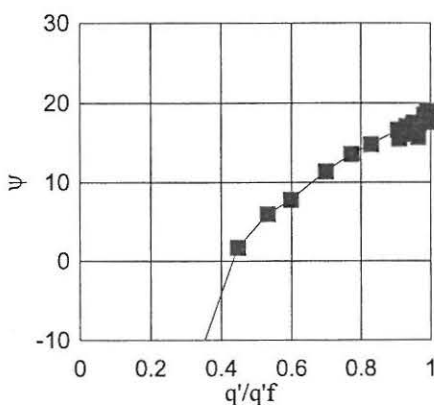
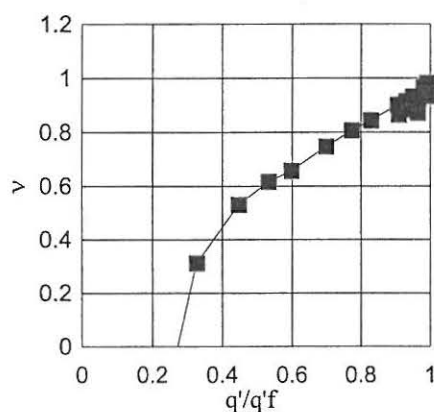


Job:	Lund No 0	Encl. No	50
Exc:	LBI, AMS	Check:	FRJ

Description of soil			Before test	At failure
Lund No 0		Water content %	23.1	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.577	0.642
kal200_891	13.06.89	Saturation	1.06	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-20 kPa
		ϵ_1	-0.082 %
		ϵ_v	-0.238 %
	2. Drained compression.		
Deformation rate:		8.6 % ph	

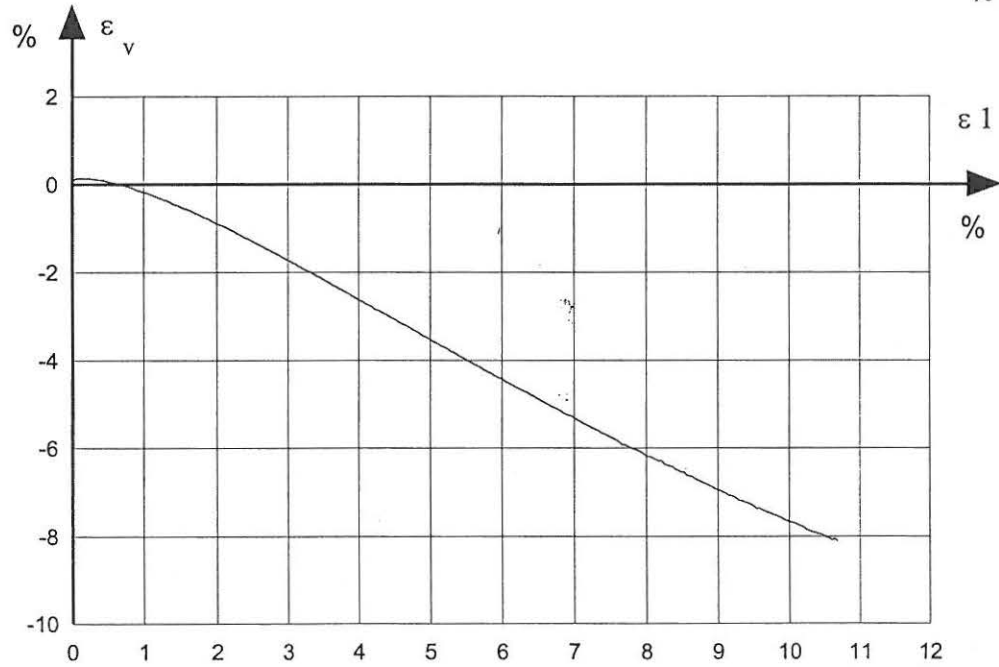
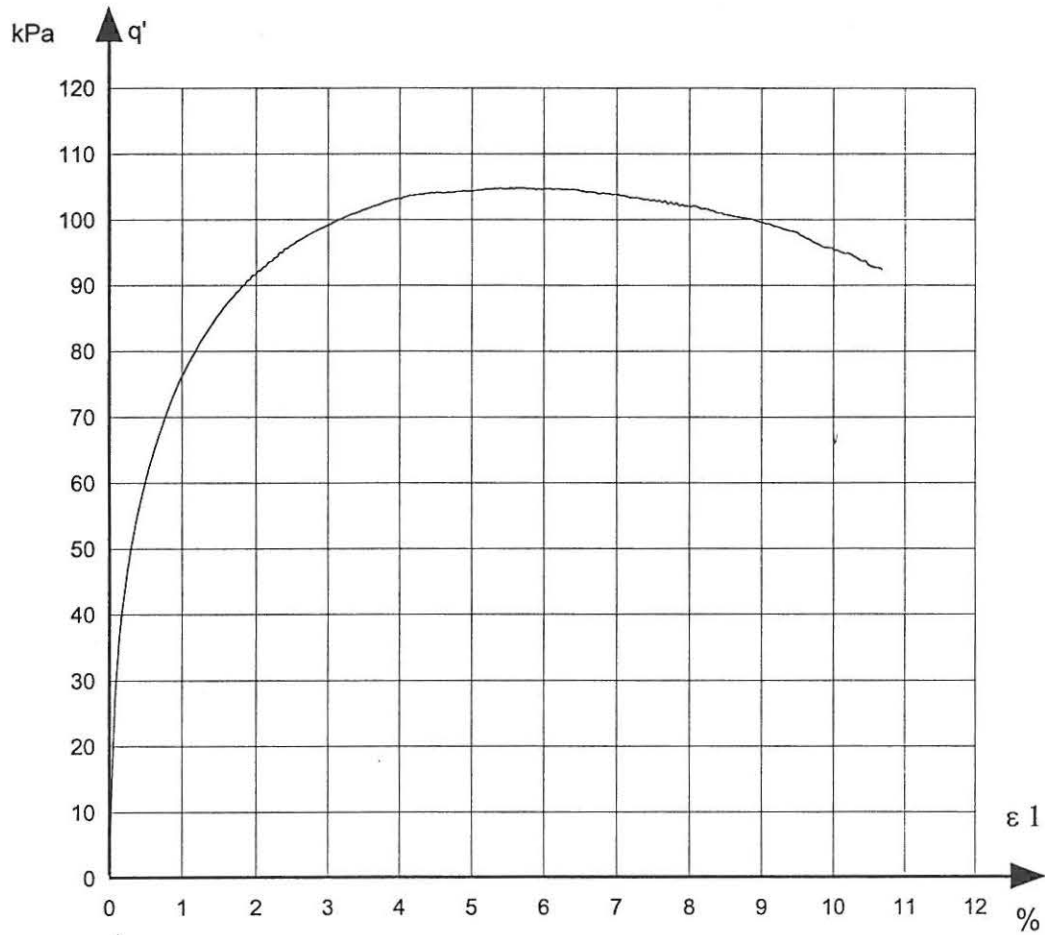
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	104.92 kPa	34.34 kPa
Mean normal stress	p'	55.49 kPa	31.96 kPa
Confining pressures	σ_3	20.52 kPa	20.52 kPa
Vertical strain	ϵ_1	5.62 %	0.12 %
Volumetric strain	ϵ_v	-4.09 %	0.15 %



q'	p'	ϵ_1	ϵ_v
0.05	20.41	0.00	0.00
0.40	20.65	0.00	0.05
12.83	24.67	0.03	0.12
34.34	31.96	0.12	0.15
46.90	36.15	0.24	0.14
55.78	39.11	0.39	0.11
62.77	41.31	0.55	0.06
73.36	44.97	0.88	-0.10
81.11	47.55	1.22	-0.31
87.02	49.40	1.58	-0.56
95.04	52.07	2.32	-1.15
97.57	53.04	2.69	-1.46
99.66	53.86	3.07	-1.78
101.29	54.15	3.45	-2.10
102.81	54.91	3.82	-2.45
103.82	55.00	4.21	-2.82
104.14	55.10	4.59	-3.15
104.92	55.49	5.62	-4.09
104.69	55.54	6.12	-4.53
104.04	55.20	6.89	-5.23
102.94	54.96	7.66	-5.92
101.09	54.34	8.44	-6.49
98.99	53.39	9.21	-7.09
95.42	52.20	10.01	-7.67

Job:	Encl. No
Lund No 0	51
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.071

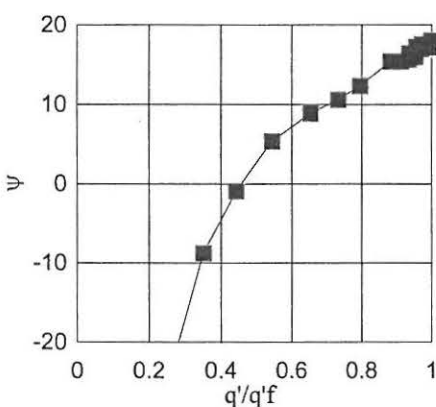
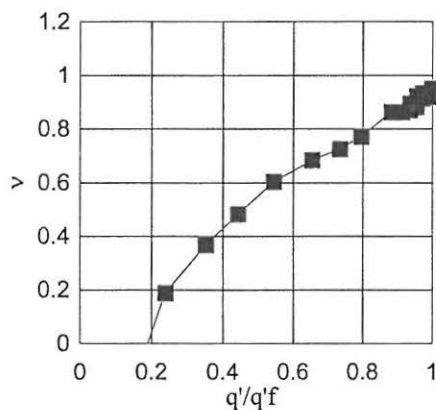


Job:	Encl. No
Lund No 0	52
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.4	At failure 0.631
		Grain density	2.65	
Calibration file	Date	Void ratio	0.576	
kal200_891	06.06.89	Saturation	1.03	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-40 kPa
		ϵ_1	-0.074 %
		ϵ_v	-0.159 %
	2. Drained compression.		
Deformation rate:			8.2 % ph

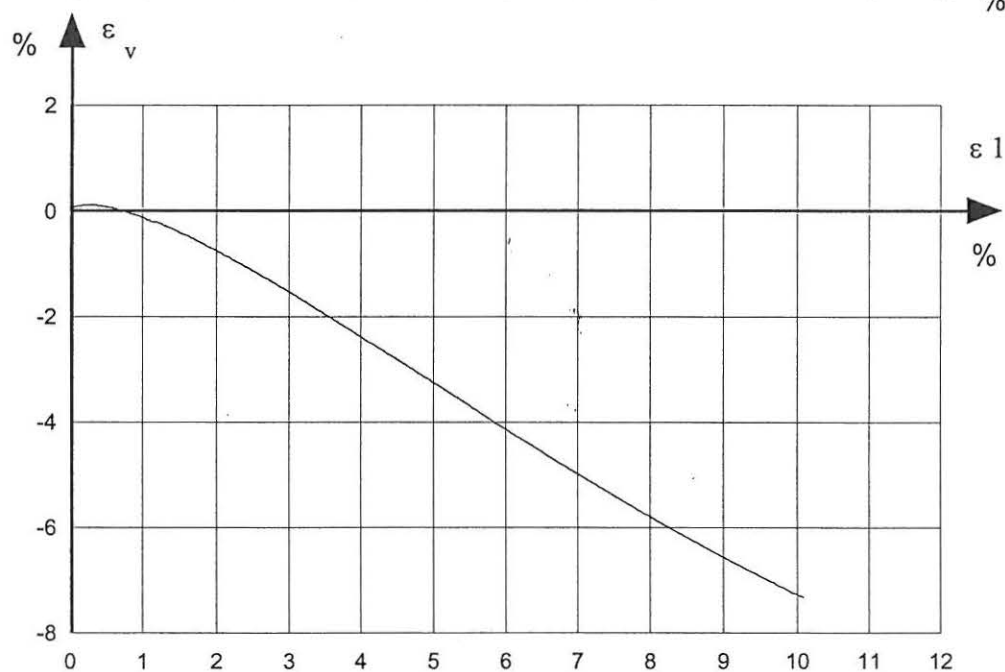
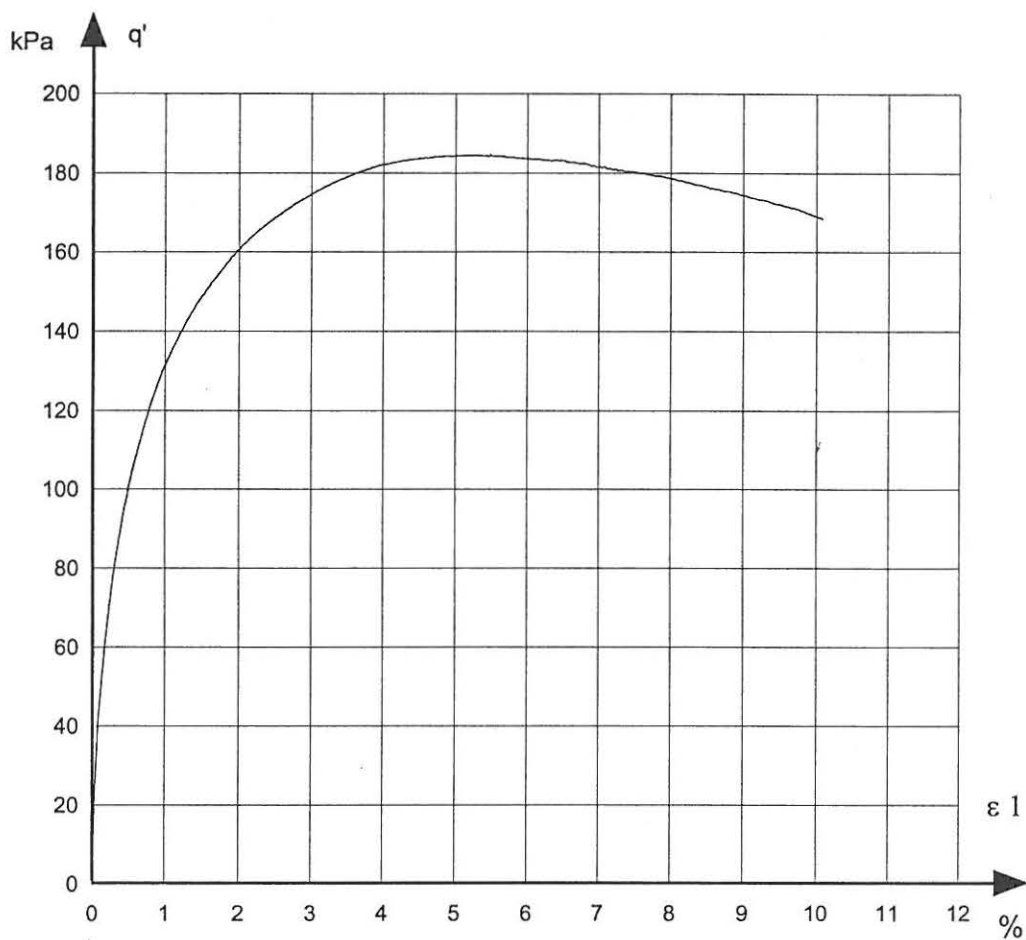
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	184.55	kPa	81.73	kPa
Mean normal stress	p'	101.67	kPa	67.40	kPa
Confining pressures	σ_3	40.16	kPa	40.16	kPa
Vertical strain	ϵ_1	5.27	%	0.31	%
Volumetric strain	ϵ_v	-3.48	%	0.13	%



q'	p'	ϵ_1	ϵ_v
0.00	40.16	0.00	0.00
8.20	42.89	0.01	0.04
15.40	45.29	0.02	0.05
44.21	54.90	0.09	0.10
64.94	61.80	0.19	0.12
81.73	67.40	0.31	0.13
100.54	73.67	0.49	0.09
120.65	80.37	0.78	-0.02
135.45	85.31	1.09	-0.15
146.82	89.22	1.42	-0.33
162.72	94.27	2.10	-0.83
168.19	96.34	2.45	-1.08
172.61	97.94	2.81	-1.36
176.32	98.80	3.16	-1.66
179.27	99.91	3.52	-1.97
181.62	100.70	3.88	-2.27
183.06	101.30	4.24	-2.58
184.55	101.67	5.27	-3.48
184.13	101.53	5.71	-3.88
183.18	101.34	6.45	-4.50
180.86	100.57	7.19	-5.13
178.93	99.80	7.94	-5.75
175.83	98.77	8.69	-6.32
172.27	97.71	9.44	-6.88

Job:	Encl. No
Lund No 0	53
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.071

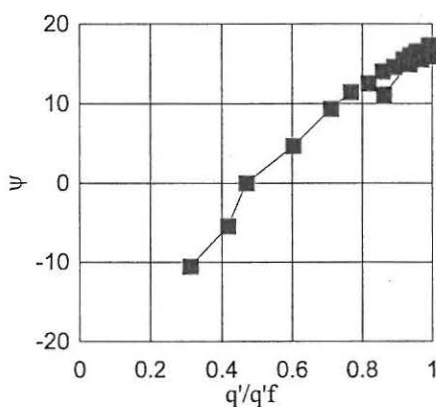
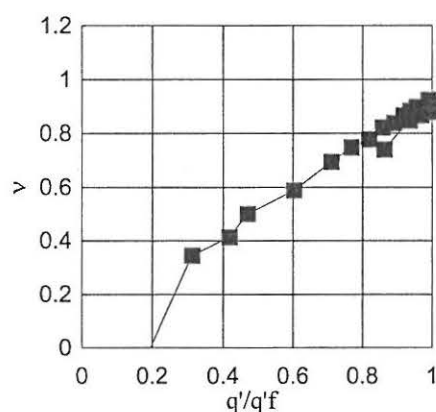


Job:	Encl. No
Lund No 0	54
Exc:	Check:
LBI, AMS	FRJ

Description of soil			Before test	At failure
Lund No 0		Water content %	22.3	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.565	
kal200_891	10.08.89	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.	σ_3	100-40 kPa	
		ϵ_1	-0.062 %	
		ϵ_v	-0.136 %	
2. Drained compression.				
Deformation rate:		7.8 % ph		

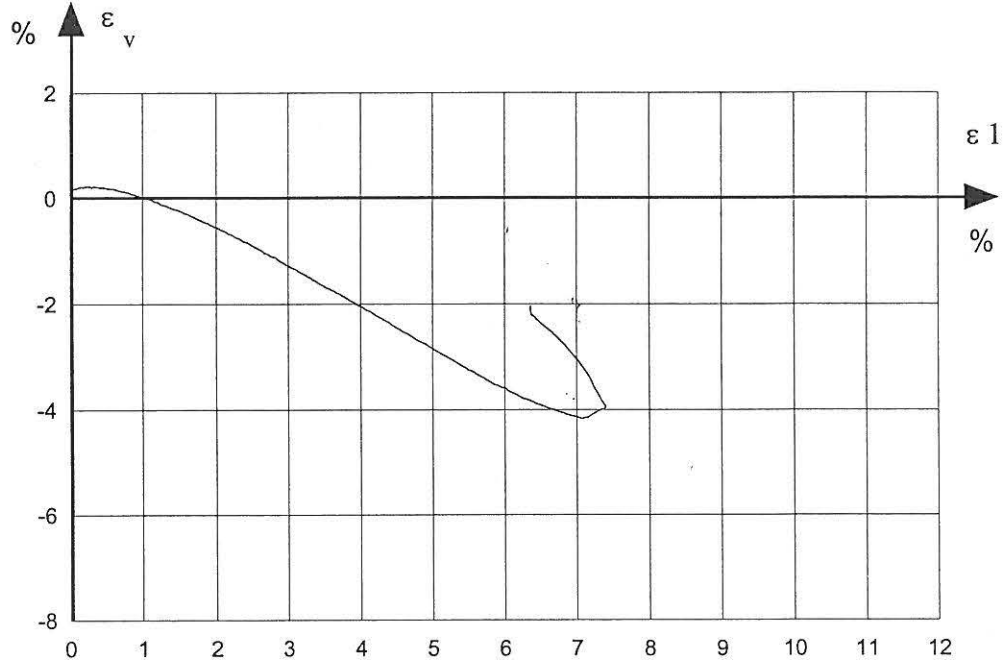
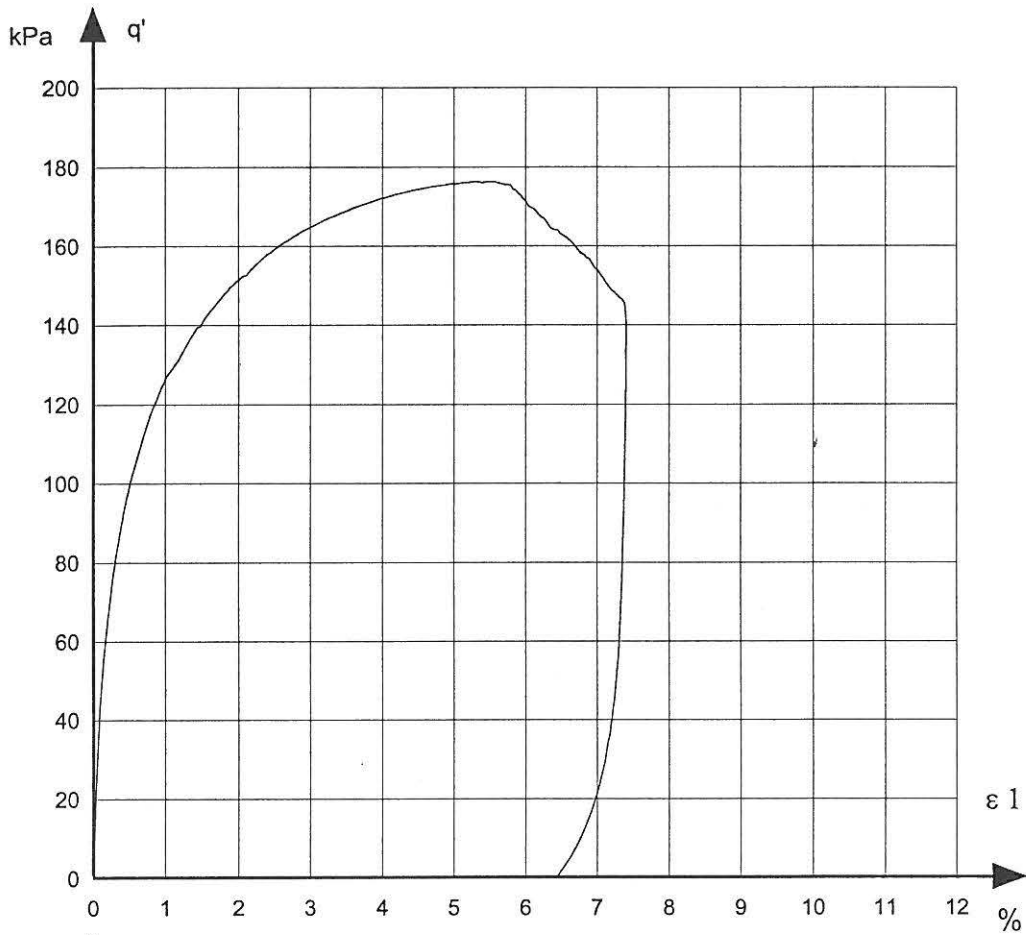
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'		82.84 kPa
Mean normal stress	p'		68.14 kPa
Confining pressures	σ_3		40.53 kPa
Vertical strain	ϵ_1		0.32 %
Volumetric strain	ϵ_v		0.23 %



q'	p'	ϵ_1	ϵ_v
0.05	40.55	0.00	0.00
2.21	41.15	0.00	0.10
8.21	43.27	0.01	0.13
19.68	46.97	0.03	0.15
31.21	50.81	0.05	0.18
55.15	58.79	0.14	0.21
73.75	65.11	0.25	0.23
82.84	68.14	0.32	0.23
106.50	75.91	0.60	0.18
125.47	82.48	0.97	0.03
135.51	85.58	1.30	-0.13
144.30	88.51	1.64	-0.32
151.41	91.00	1.99	-0.54
157.19	92.93	2.34	-0.78
161.83	94.47	2.70	-1.04
165.46	95.56	3.06	-1.31
168.44	96.68	3.42	-1.60
170.93	97.51	3.78	-1.87
173.08	98.22	4.15	-2.14
174.52	98.58	4.51	-2.45
175.58	98.81	4.87	-2.73
176.43	99.22	5.53	-3.25
164.92	95.63	6.33	-3.81
152.11	90.86	7.07	-4.16

Job:	Encl. No
Lund No 0	55
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.108
The specimen slipped out.

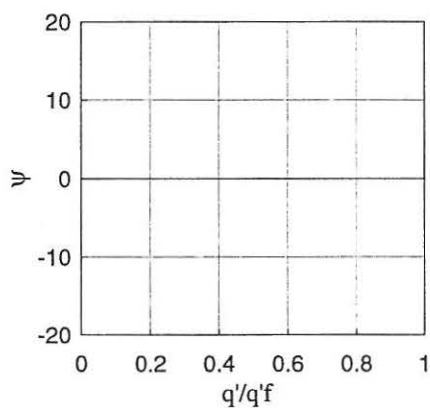
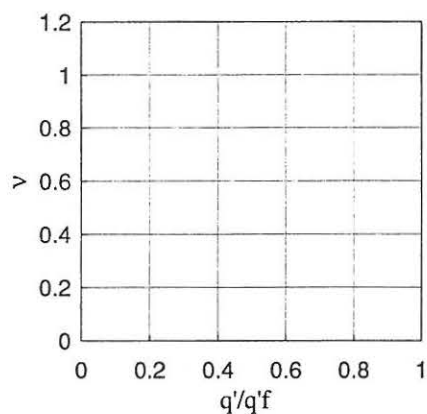


Job:	Lund No 0	Encl. No	56
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	22.0	At failure 0.630
		Grain density	2.65	
Calibration file	Date	Void ratio	0.570	
		Saturation	1.01	
kal200_891	08.06.89	Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-80 kPa
		ε_1	-0.015 %
		ε_v	-0.166 %
	2.Drained compression.		
	Deformation rate:		8.0 % ph

		Values at failure		Values for $\Delta\varepsilon_v = 0$	
Deviator stress	q'	336.70	kPa	171.90	kPa
Mean normal stress	p'	190.90	kPa	136.00	kPa
Confining pressures	σ_3	78.70	kPa	78.70	kPa
Vertical strain	ε_1	6.55	%	0.49	%
Volumetric strain	ε_v	-3.62	%	0.18	%

[illegible]

Job: Lund No 0	Encl. No 69
Exc:	Check:
LBL AMS	FRJ

Remark:	
Preparation [%]	$\Delta \varepsilon_1 = 0.071$

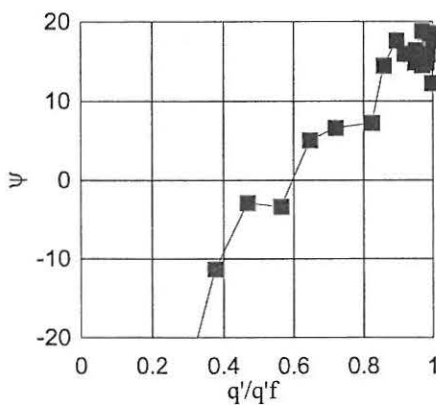
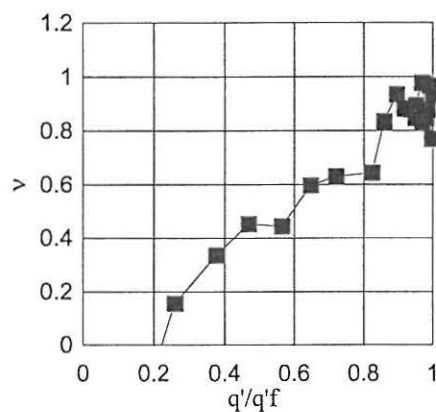
Data-file has been lost.

Job:	Encl. No
Lund No 0	70
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	21.4	At failure 0.613
		Grain density	2.65	
Calibration file	Date	Void ratio	0.565	
kal200_891	15.08.89	Saturation	1.00	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-80 kPa
		ϵ_1	-0.022 %
		ϵ_v	-0.034 %
	2. Drained compression.	Deformation rate: 8.2 % ph	

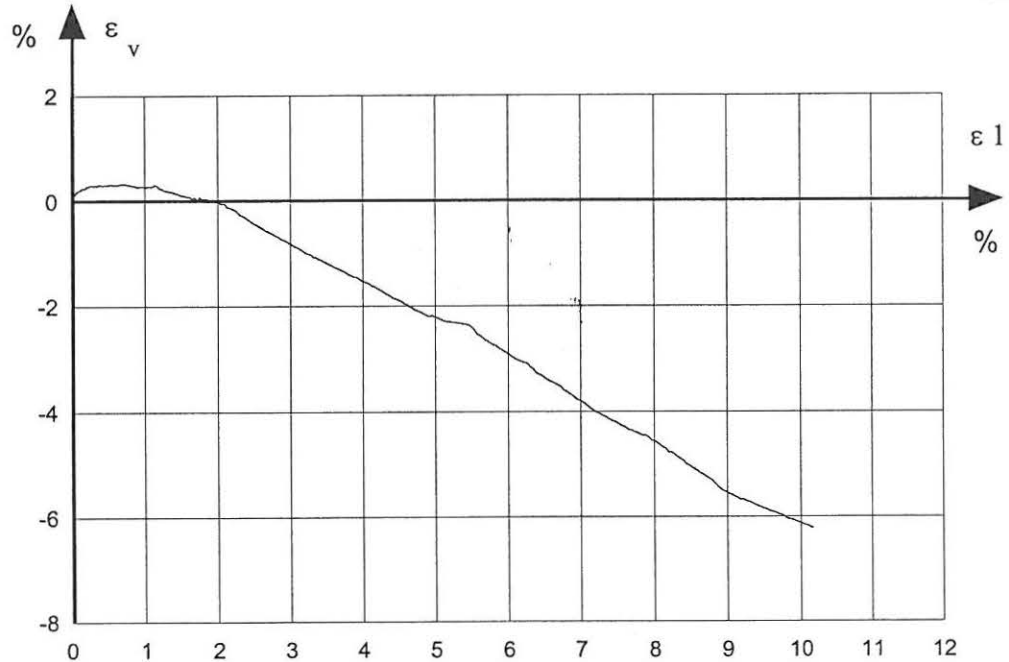
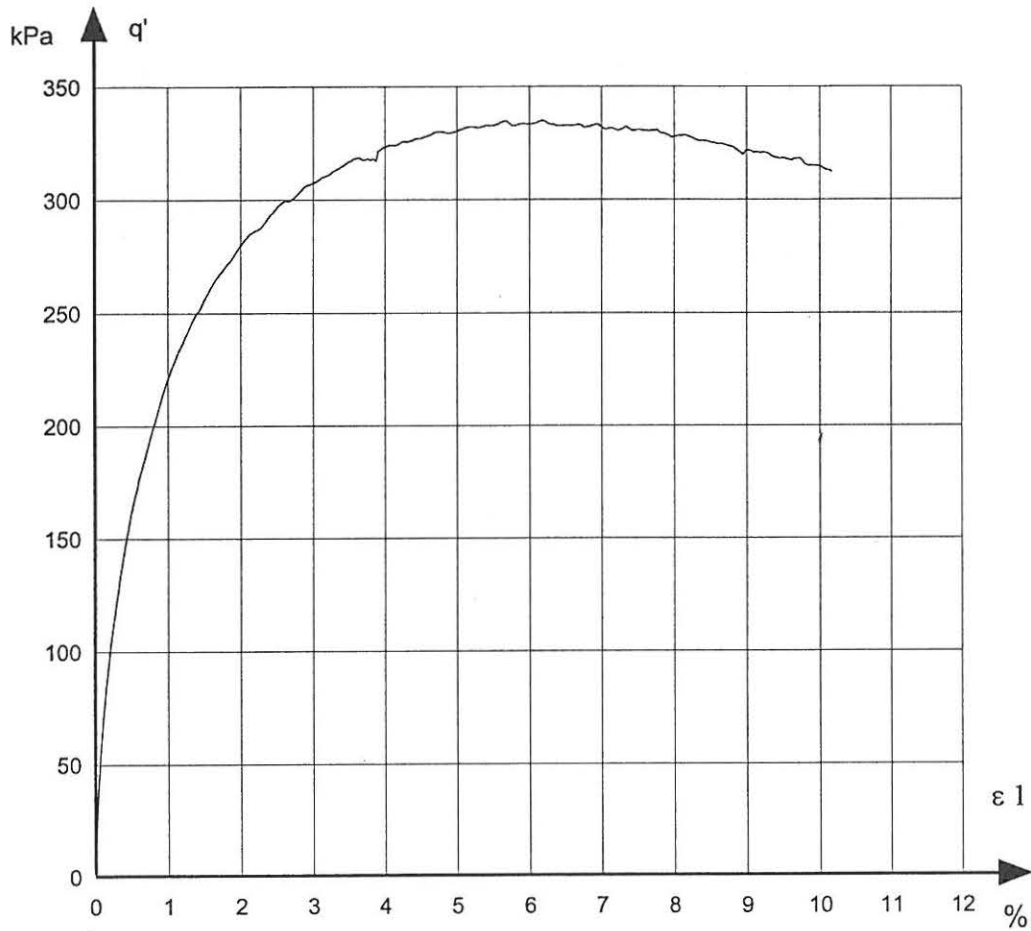
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	334.98	kPa	189.15	kPa
Mean normal stress	p'	190.72	kPa	141.74	kPa
Confining pressures	σ_3	79.06	kPa	78.69	kPa
Vertical strain	ϵ_1	6.17	%	0.70	%
Volumetric strain	ϵ_v	-3.04	%	0.34	%



q'	p'	ϵ_1	ϵ_v
-0.05	78.80	0.00	0.00
13.34	83.39	0.01	0.08
34.40	90.28	0.03	0.13
58.78	98.53	0.08	0.19
87.17	107.87	0.15	0.25
126.33	120.92	0.30	0.29
156.89	131.24	0.46	0.31
189.15	141.74	0.70	0.34
216.95	151.26	0.95	0.29
241.45	159.30	1.26	0.21
276.30	170.91	1.92	0.02
287.57	174.05	2.27	-0.22
299.87	179.02	2.62	-0.52
307.24	181.22	2.98	-0.79
313.90	183.32	3.34	-1.07
317.73	184.35	3.72	-1.33
324.21	187.13	4.08	-1.57
329.67	188.45	4.84	-2.14
334.08	190.42	5.60	-2.54
334.98	190.72	6.17	-3.04
331.62	189.10	7.14	-3.94
328.37	188.02	7.92	-4.48
324.28	186.91	8.70	-5.23
318.44	185.21	9.49	-5.84

Job: Lund No 0	Encl. No 59
Exc: LBI, AMS	Check: FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.106

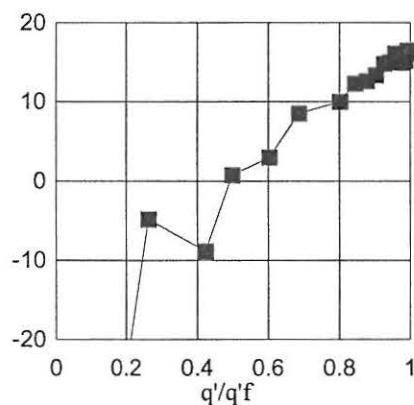
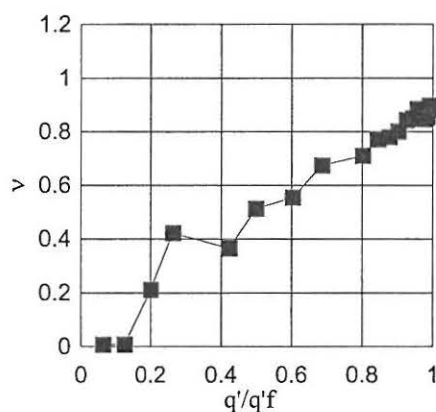


Job:	Encl. No
Lund No 0	60
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	21.3	
Calibration file	Date	Void ratio	0.559	0.611
ka1500_891	16.08.89	Saturation	1.01	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.034 %
		ϵ_v	0.109 %
	2. Drained compression.		
	Deformation rate:		8.2 % ph

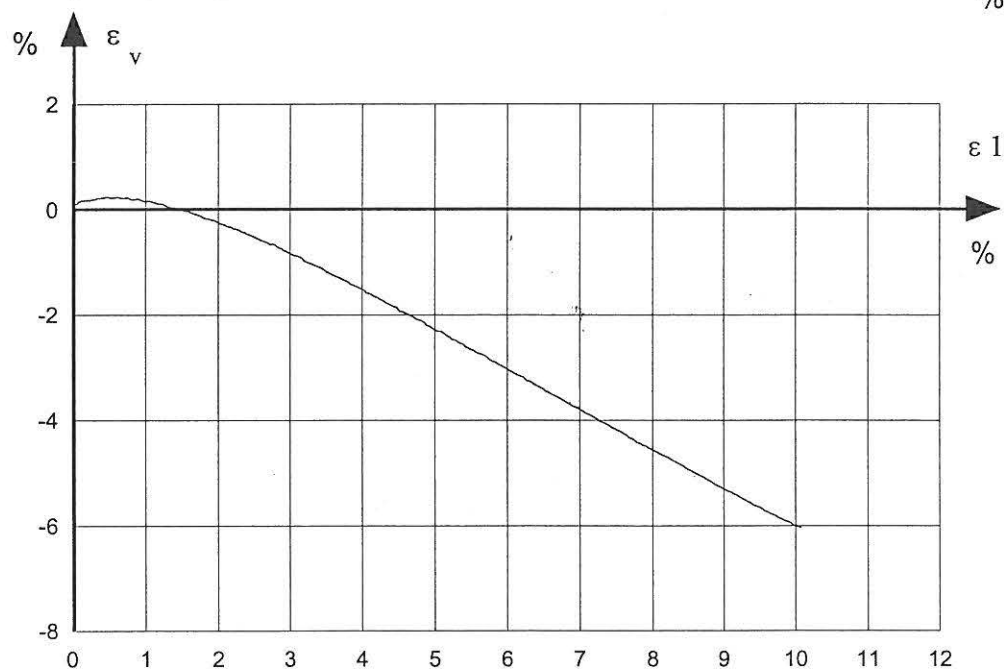
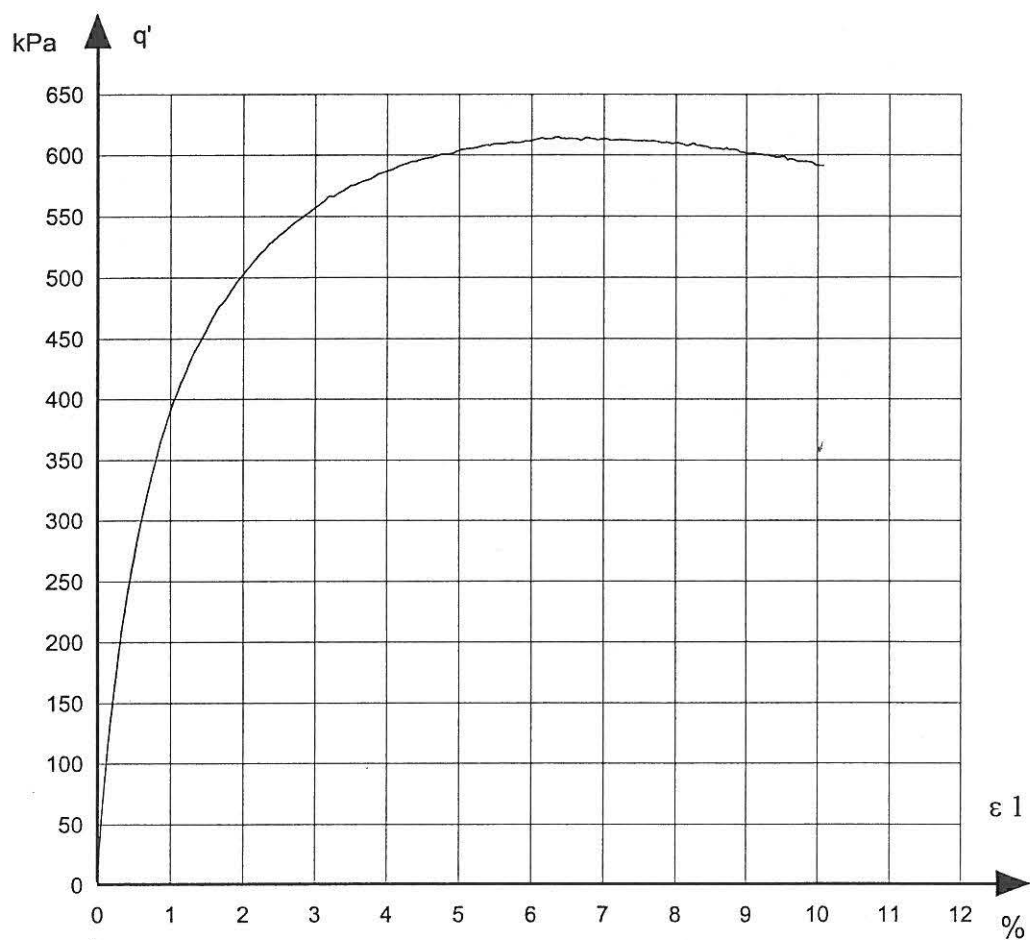
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	614.96 kPa	260.71 kPa
Mean normal stress	p'	362.99 kPa	244.03 kPa
Confining pressures	σ_3	158.00 kPa	157.13 kPa
Vertical strain	ϵ_1	6.37 %	0.46 %
Volumetric strain	ϵ_v	-3.31 %	0.25 %



q'	p'	ϵ_1	ϵ_v
0.15	157.18	0.00	0.00
10.27	160.80	0.01	0.06
38.51	169.96	0.03	0.09
76.41	182.85	0.08	0.13
123.49	198.17	0.15	0.17
162.65	211.22	0.22	0.19
260.71	244.03	0.46	0.25
306.61	259.83	0.61	0.25
371.47	281.45	0.89	0.22
422.78	298.80	1.20	0.11
493.12	322.00	1.86	-0.17
519.11	331.04	2.21	-0.36
538.39	336.97	2.57	-0.56
554.83	342.44	2.94	-0.78
568.95	347.28	3.31	-1.03
579.50	350.42	3.68	-1.29
588.16	353.68	4.06	-1.58
601.24	357.91	4.81	-2.10
609.47	360.66	5.57	-2.70
614.96	362.99	6.37	-3.31
612.14	361.30	7.10	-3.87
610.66	361.18	7.86	-4.47
605.51	359.46	8.63	-5.02
598.48	356.62	9.41	-5.60

Job:	Encl. No
Lund No 0	61
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.061

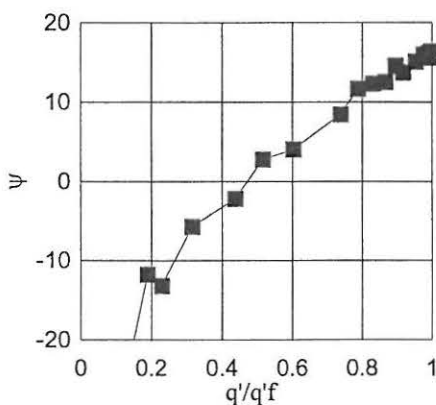
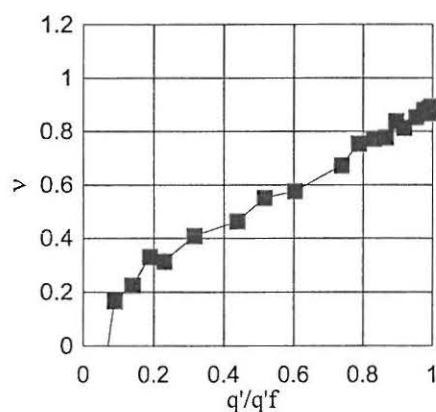


Job:	Encl. No
Lund No 0	62
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.2	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.559		
kal500_891	17.08.89	Saturation	1.05		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.		σ_3	100-160 kPa
			ϵ_1	0.034 %
			ϵ_v	0.109 %
	2. Drained compression.		Deformation rate: 7.9 % ph	

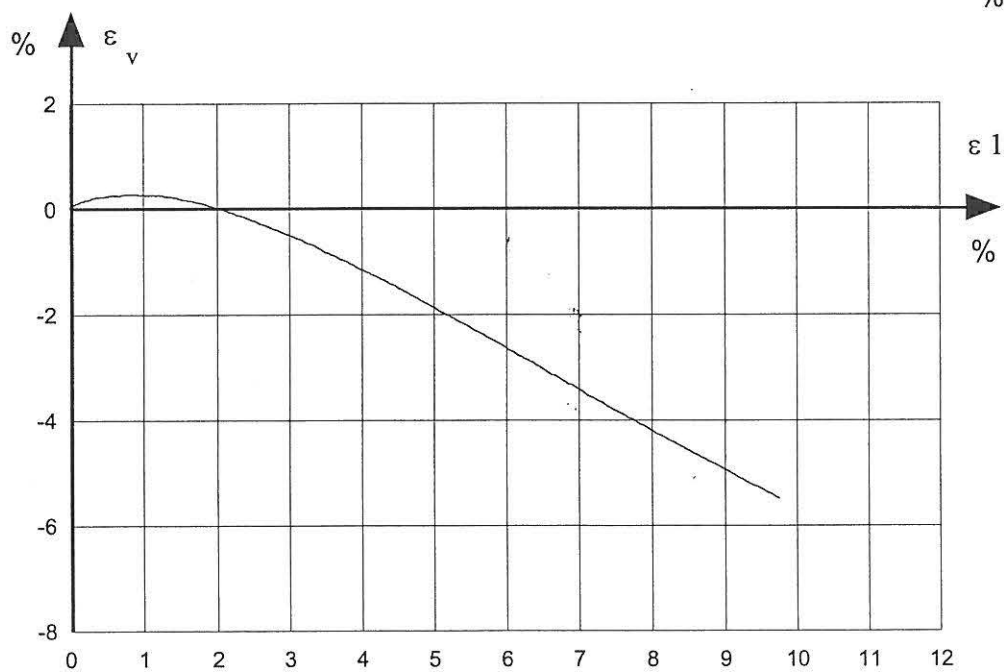
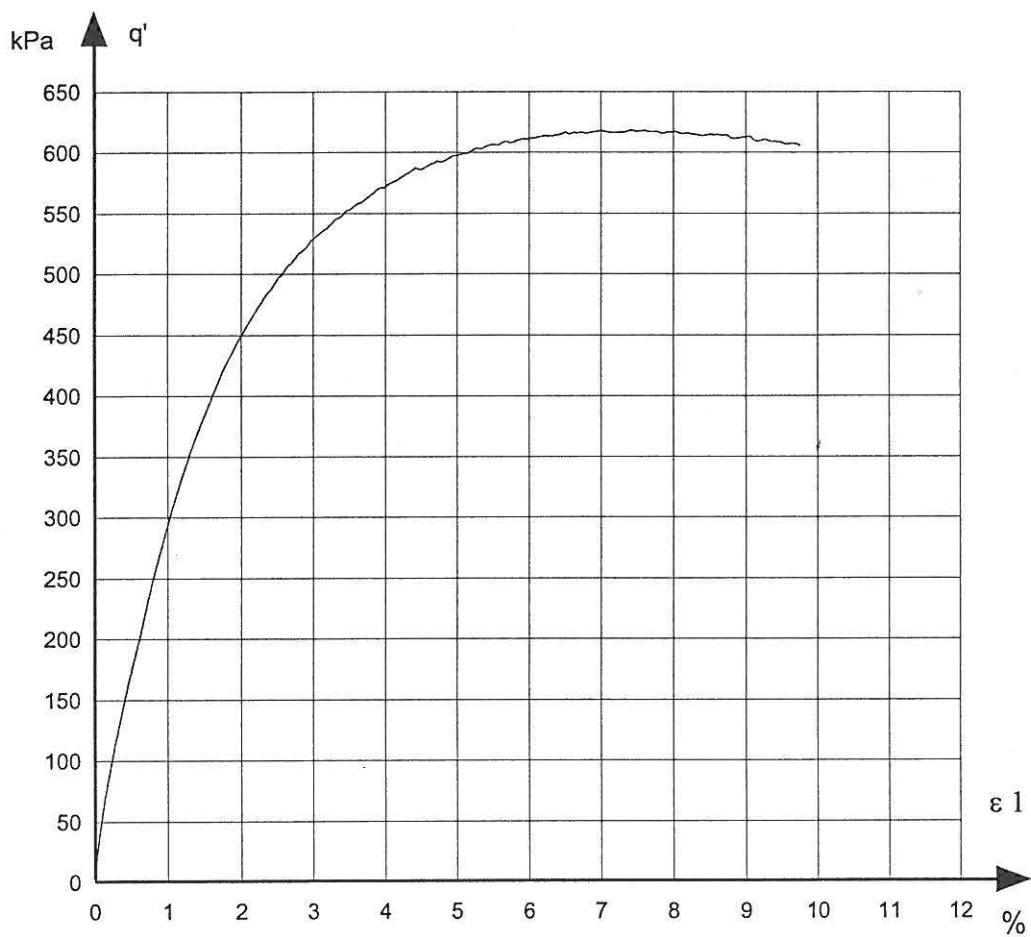
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	618.43	kPa	270.42	kPa
Mean normal stress	p'	363.89	kPa	247.77	kPa
Confining pressures	σ_3	157.75	kPa	157.63	kPa
Vertical strain	ϵ_1	7.41	%	0.89	%
Volumetric strain	ϵ_v	-3.74	%	0.29	%



q'	p'	ϵ_1	ϵ_v
0.15	157.43	0.00	0.00
14.64	161.88	0.01	0.04
38.19	170.23	0.05	0.09
54.63	176.21	0.09	0.11
86.14	186.46	0.18	0.16
116.83	196.82	0.28	0.20
142.08	204.74	0.38	0.23
195.53	222.55	0.58	0.27
270.42	247.77	0.89	0.29
318.51	263.92	1.11	0.27
372.55	281.69	1.42	0.22
456.90	309.67	2.06	0.00
487.43	319.98	2.41	-0.17
514.16	329.14	2.76	-0.36
535.12	336.12	3.11	-0.56
553.09	342.49	3.47	-0.80
567.14	347.05	3.84	-1.03
588.84	354.16	4.57	-1.54
602.86	358.33	5.31	-2.11
612.16	361.81	6.06	-2.68
616.18	363.15	6.81	-3.27
618.43	363.89	7.41	-3.74
614.72	362.41	8.33	-4.44
611.14	361.21	9.09	-5.00

Job:	Encl. No
Lund No 0	63
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.090

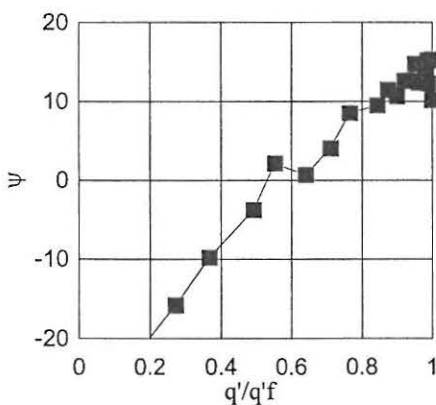
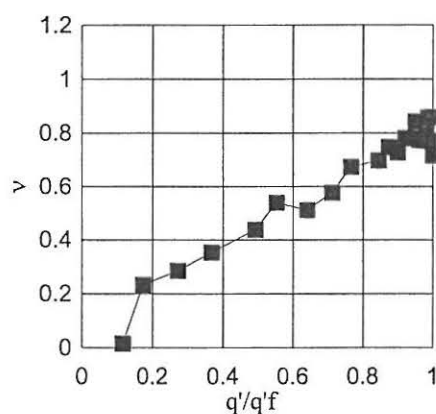


Job:	Lund No 0	Encl. No	64
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	21.3	At failure 0.601
		Grain density	2.65	
Calibration file	Date	Void ratio	0.558	
k2000_891	22.08.89	Saturation	1.01	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-320 kPa
		ϵ_1	0.119 %
		ϵ_v	0.238 %
	2. Drained compression.	Deformation rate: 8.7 % ph	

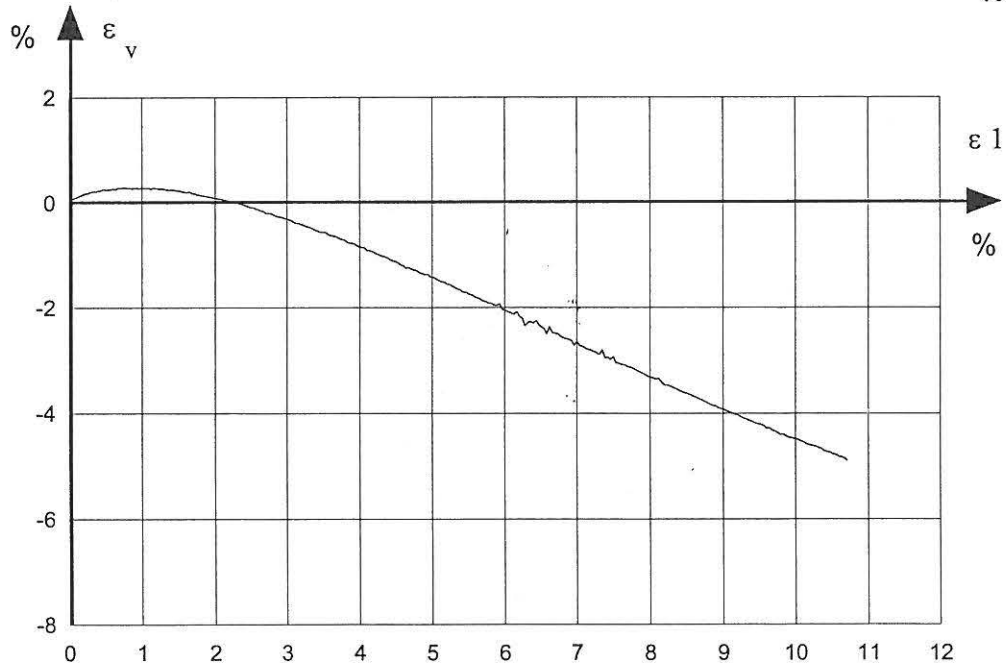
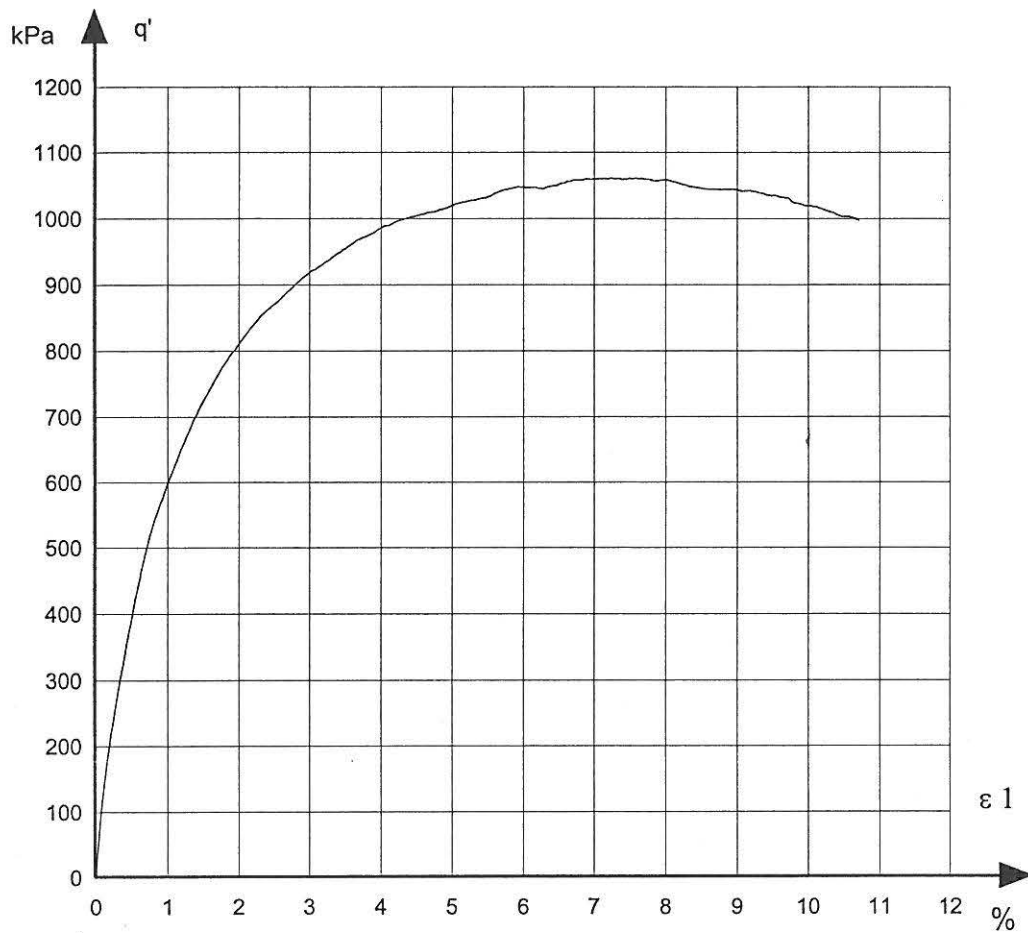
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 061.47	kPa	521.70	kPa
Mean normal stress	p'	665.07	kPa	486.90	kPa
Confining pressures	σ_3	311.25	kPa	313.00	kPa
Vertical strain	ϵ_1	7.34	%	0.76	%
Volumetric strain	ϵ_v	-2.80	%	0.29	%



q'	p'	ϵ_1	ϵ_v
0.00	308.50	0.00	0.00
41.27	321.50	0.02	0.04
122.05	349.05	0.09	0.10
183.01	369.00	0.16	0.14
289.07	404.73	0.32	0.21
390.19	440.69	0.48	0.26
521.70	486.90	0.76	0.29
586.95	506.15	0.95	0.28
679.78	537.84	1.29	0.27
754.90	563.26	1.64	0.22
812.75	582.67	2.00	0.09
895.64	608.54	2.74	-0.20
927.55	620.56	3.12	-0.39
954.73	629.99	3.49	-0.56
977.81	637.69	3.87	-0.77
997.81	644.23	4.25	-0.98
1009.32	646.06	4.63	-1.24
1030.58	652.52	5.39	-1.66
1048.03	658.84	6.16	-2.07
1059.77	664.76	6.92	-2.62
1061.47	665.07	7.34	-2.80
1047.10	657.65	8.47	-3.60
1041.21	658.19	9.24	-4.06
1019.86	649.70	10.03	-4.49

Job:	Encl. No
Lund No 0	65
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.050

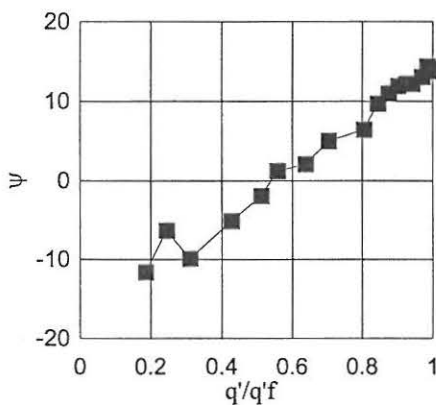
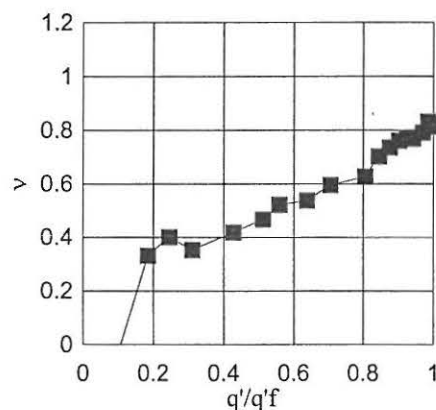


Job:	Encl. No
Lund No 0	66
Exc:	Check:
LBI, AMS	FRJ

Description of soil			Before test	At failure
Lund No 0		Water content %	22.1	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.562	0.610
k2000_891	23.08.89	Saturation	1.04	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.			
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-320 kPa	
		ϵ_1	0.111 %	
		ϵ_v	0.272 %	
	2. Drained compression.			
Deformation rate:			8.8 % ph	

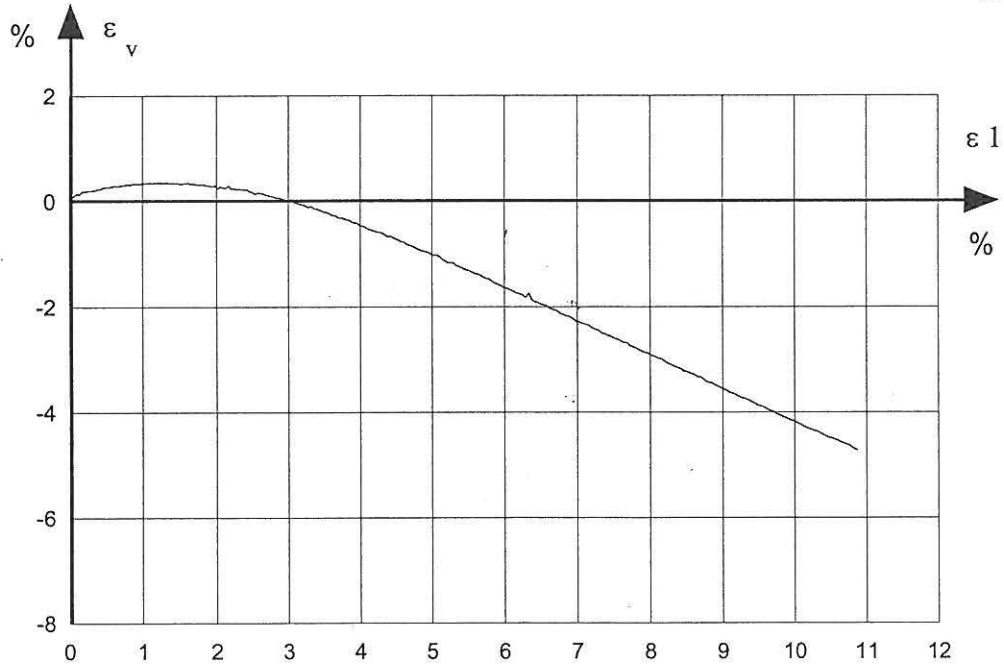
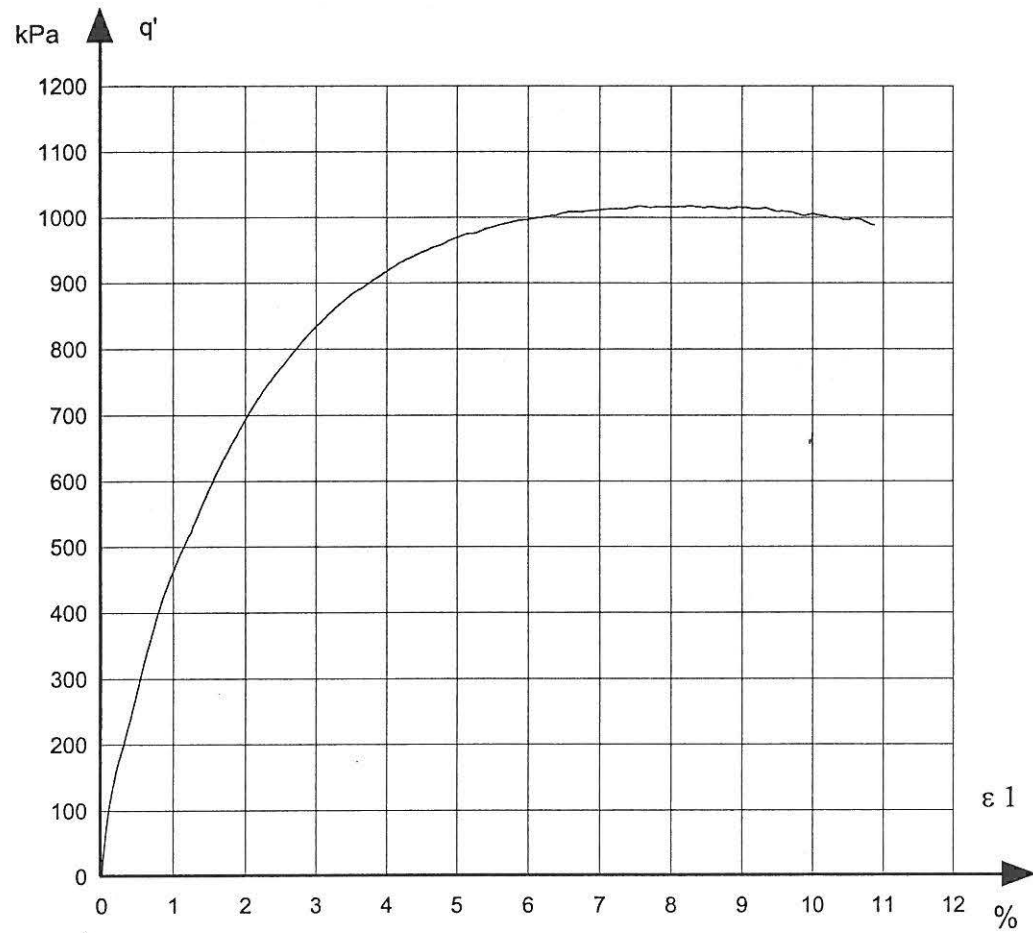
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	1 018.26 kPa	521.35 kPa
Mean normal stress	p'	651.54 kPa	485.03 kPa
Confining pressures	σ_3	312.12 kPa	311.25 kPa
Vertical strain	ϵ_1	8.23 %	1.24 %
Volumetric strain	ϵ_v	-3.05 %	0.35 %



q'	p'	ϵ_1	ϵ_v
0.58	306.69	0.00	0.00
44.00	325.79	0.03	0.08
101.91	344.22	0.10	0.15
187.96	373.03	0.27	0.20
249.54	394.05	0.42	0.23
316.78	417.47	0.57	0.28
435.02	457.01	0.90	0.33
521.35	485.03	1.24	0.35
568.47	499.86	1.41	0.35
649.54	528.14	1.77	0.32
717.68	551.35	2.12	0.25
820.03	584.97	2.86	0.07
859.81	598.35	3.24	-0.08
891.06	608.52	3.62	-0.26
918.43	617.64	3.99	-0.46
941.50	625.96	4.37	-0.66
958.22	630.78	4.75	-0.86
986.73	640.91	5.52	-1.31
1002.01	645.88	6.29	-1.82
1012.53	648.76	7.06	-2.31
1018.26	651.54	8.23	-3.05
1016.07	650.69	8.62	-3.30
1012.44	649.73	9.40	-3.81
1002.17	645.81	10.20	-4.30

Job:	Encl. No
Lund No 0	67
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.500

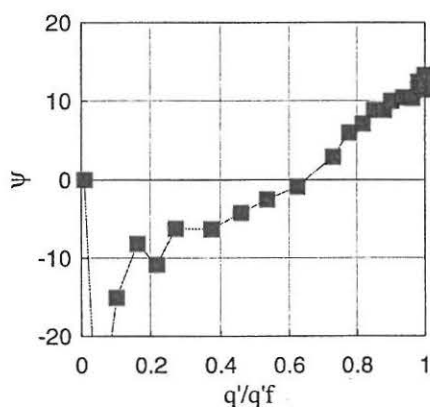
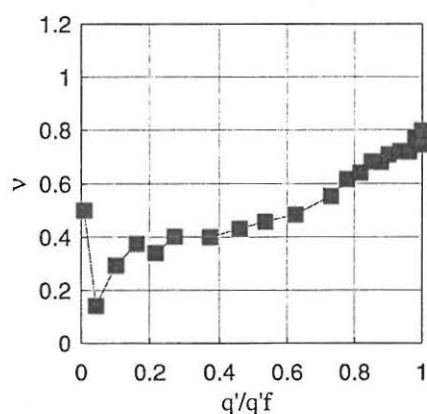


Job:	Encl. No
Lund No 0	68
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.0	Before test	At failure	
		Grain density	2.65			
Calibration file	Date	Void ratio	0.563			0.606
k2000_892	29.08.89	Saturation	1.04			
		Dimension H mm	71.5			
		D mm	69.7			

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-620 kPa
		ϵ_1	0.194 %
		ϵ_v	0.502 %
	2. Drained compression.		
		Deformation rate:	8.1 % ph

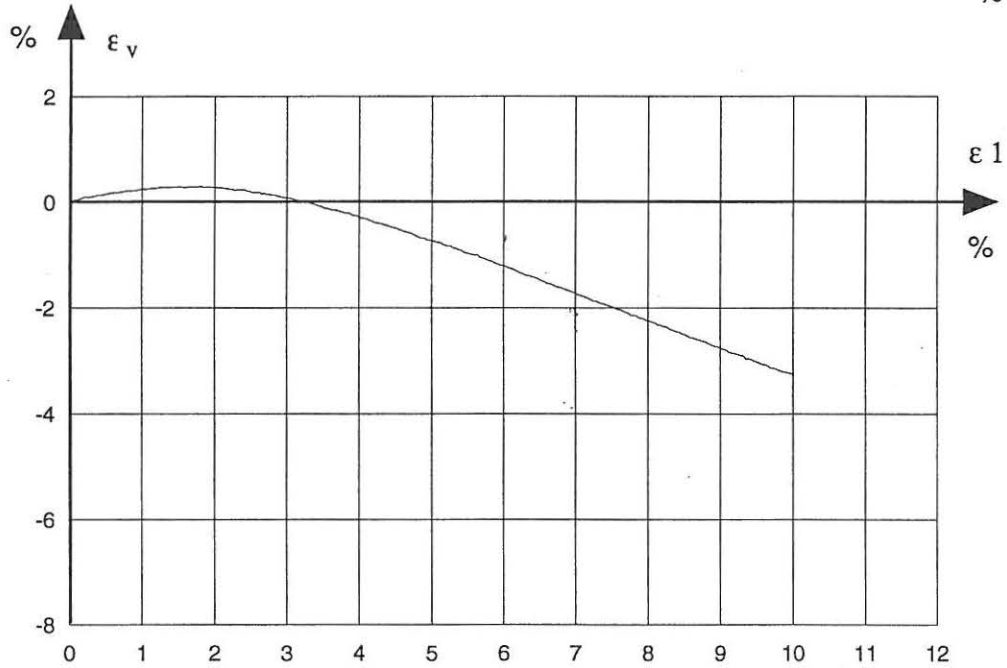
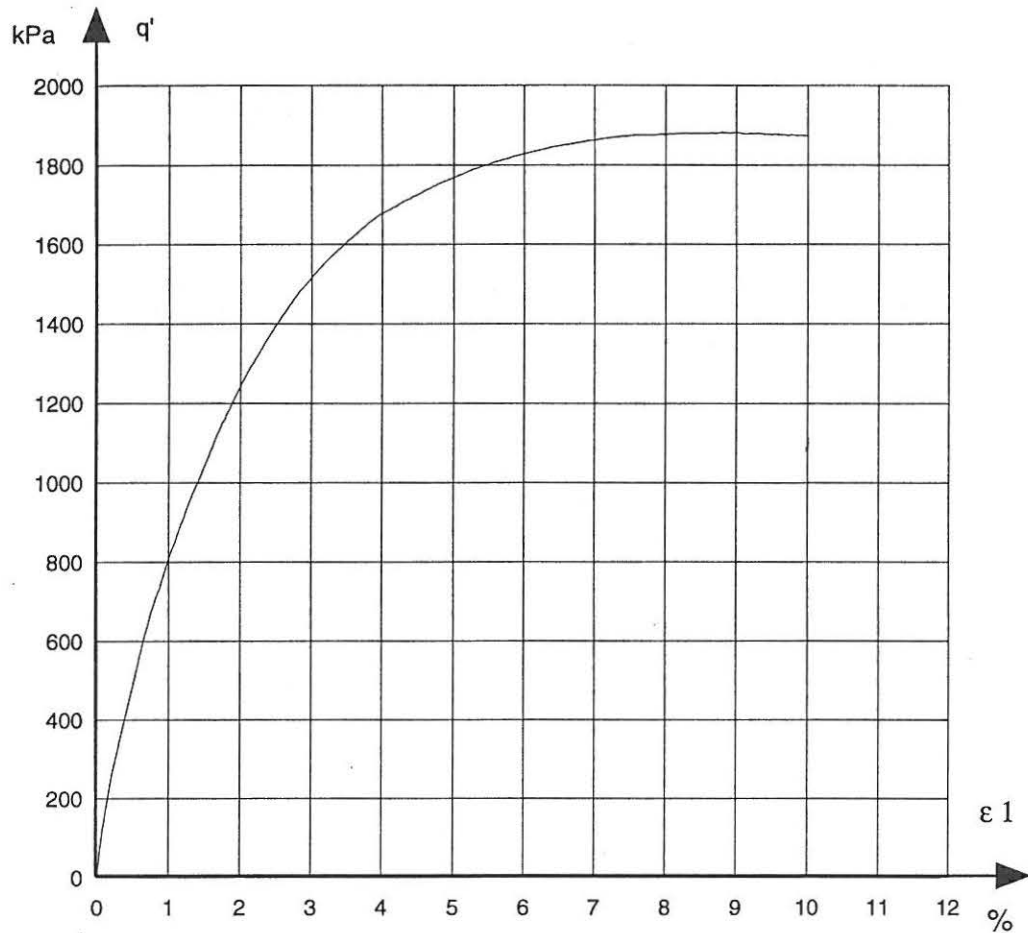
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 881.37	kPa	1 173.23	kPa
Mean normal stress	p'	1 251.12	kPa	1 013.83	kPa
Confining pressures	σ_3	624.00	kPa	622.75	kPa
Vertical strain	ϵ_1	9.01	%	1.81	%
Volumetric strain	ϵ_v	-2.76	%	0.30	%



q'	p'	ϵ_1	ϵ_v
0.00	622.50	0.00	0.00
16.88	628.88	0.01	0.00
82.04	648.35	0.05	0.03
190.75	686.08	0.14	0.06
304.46	722.74	0.26	0.09
409.84	759.36	0.39	0.14
511.45	792.48	0.53	0.16
701.76	856.67	0.81	0.22
864.36	911.87	1.11	0.26
1006.83	957.61	1.43	0.29
1173.23	1013.83	1.81	0.30
1371.36	1079.37	2.41	0.23
1462.39	1111.21	2.75	0.15
1536.94	1136.56	3.10	0.06
1597.84	1155.86	3.45	-0.07
1653.60	1176.20	3.81	-0.20
1693.78	1187.84	4.17	-0.35
1760.18	1210.23	4.89	-0.67
1809.54	1225.18	5.63	-0.99
1842.58	1237.69	6.36	-1.39
1865.15	1245.22	7.10	-1.78
1876.06	1247.86	7.84	-2.14
1881.37	1251.12	9.01	-2.76
1878.75	1250.25	9.35	-2.96

Job:	Encl. No
Lund No 0	69
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 =$ 0.085

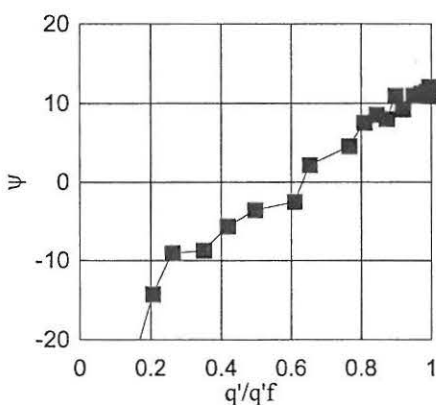
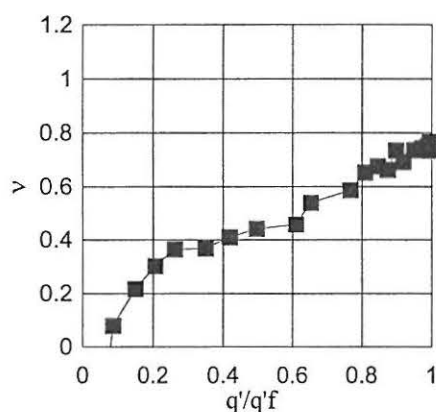


Job:	Encl. No
Lund No 0	70
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	21.3	At failure 0.590
		Grain density	2.65	
Calibration file	Date	Void ratio	0.557	
k2000_892	31.08.89	Saturation	1.02	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-620 kPa
		ϵ_1	0.287 %
		ϵ_v	0.642 %
	2. Drained compression.	Deformation rate: 8.4 % ph	

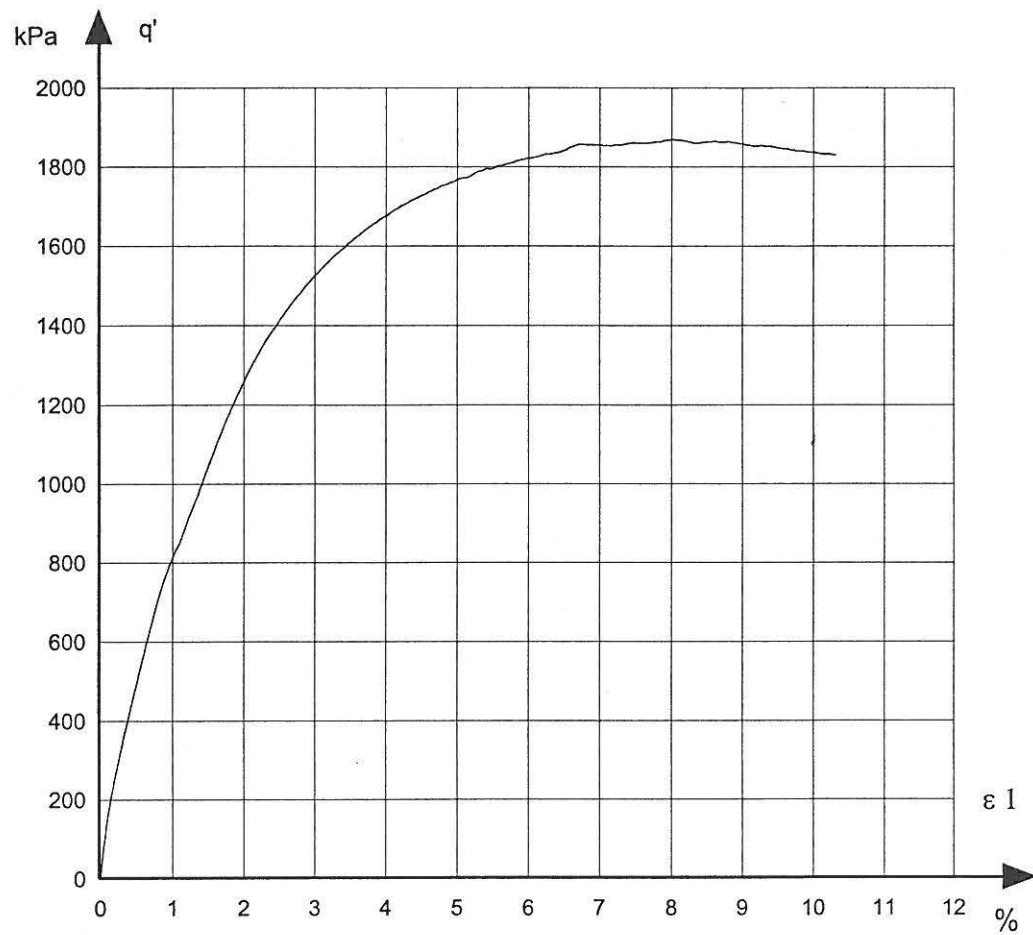
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 869.48	kPa	1 140.48	kPa
Mean normal stress	p'	1 249.91	kPa	1 003.91	kPa
Confining pressures	σ_3	626.75	kPa	623.75	kPa
Vertical strain	ϵ_1	8.04	%	1.70	%
Volumetric strain	ϵ_v	-2.15	%	0.43	%



q'	p'	ϵ_1	ϵ_v
0.58	624.19	0.00	0.00
27.37	632.12	0.01	0.03
158.38	676.04	0.10	0.10
276.44	716.65	0.22	0.17
384.96	751.57	0.35	0.22
490.29	786.18	0.49	0.26
655.32	841.44	0.72	0.32
781.53	883.26	0.93	0.36
928.22	933.66	1.25	0.40
1140.48	1003.91	1.70	0.43
1221.19	1031.06	1.89	0.42
1433.69	1100.40	2.58	0.30
1513.05	1127.60	2.93	0.20
1578.83	1149.78	3.28	0.07
1632.74	1166.25	3.64	-0.05
1677.88	1181.04	4.01	-0.22
1717.38	1195.96	4.37	-0.35
1772.73	1214.41	5.11	-0.70
1816.52	1228.26	5.85	-1.06
1851.64	1243.96	6.60	-1.41
1857.93	1242.31	7.35	-1.81
1869.48	1249.91	8.04	-2.15
1861.56	1245.02	8.88	-2.57
1844.85	1237.95	9.65	-2.96

Job:	Encl. No
Lund No 0	71
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.088

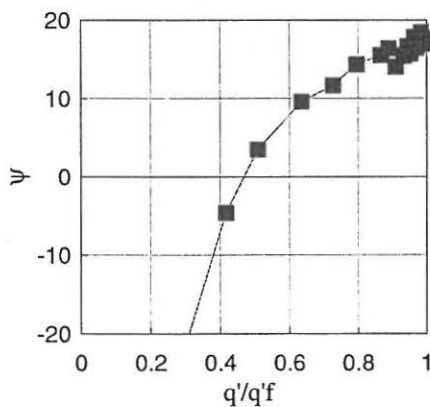
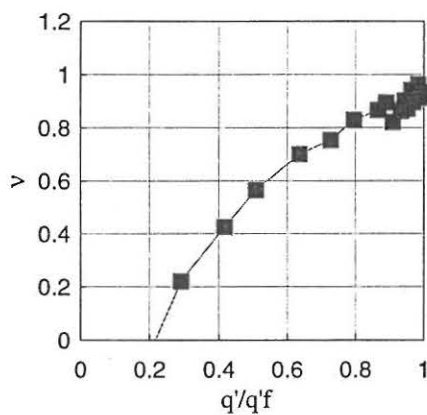


Job:	Lund No 0	Encl. No	72
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	20.8	
Calibration file	Date	Void ratio	0.558	0.617
kal200_891	14.08.89	Saturation	0.99	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-20 kPa
		ϵ_1	-0.093 %
		ϵ_v	-0.076 %
	2. Drained compression.		
	Deformation rate:		8.9 % ph

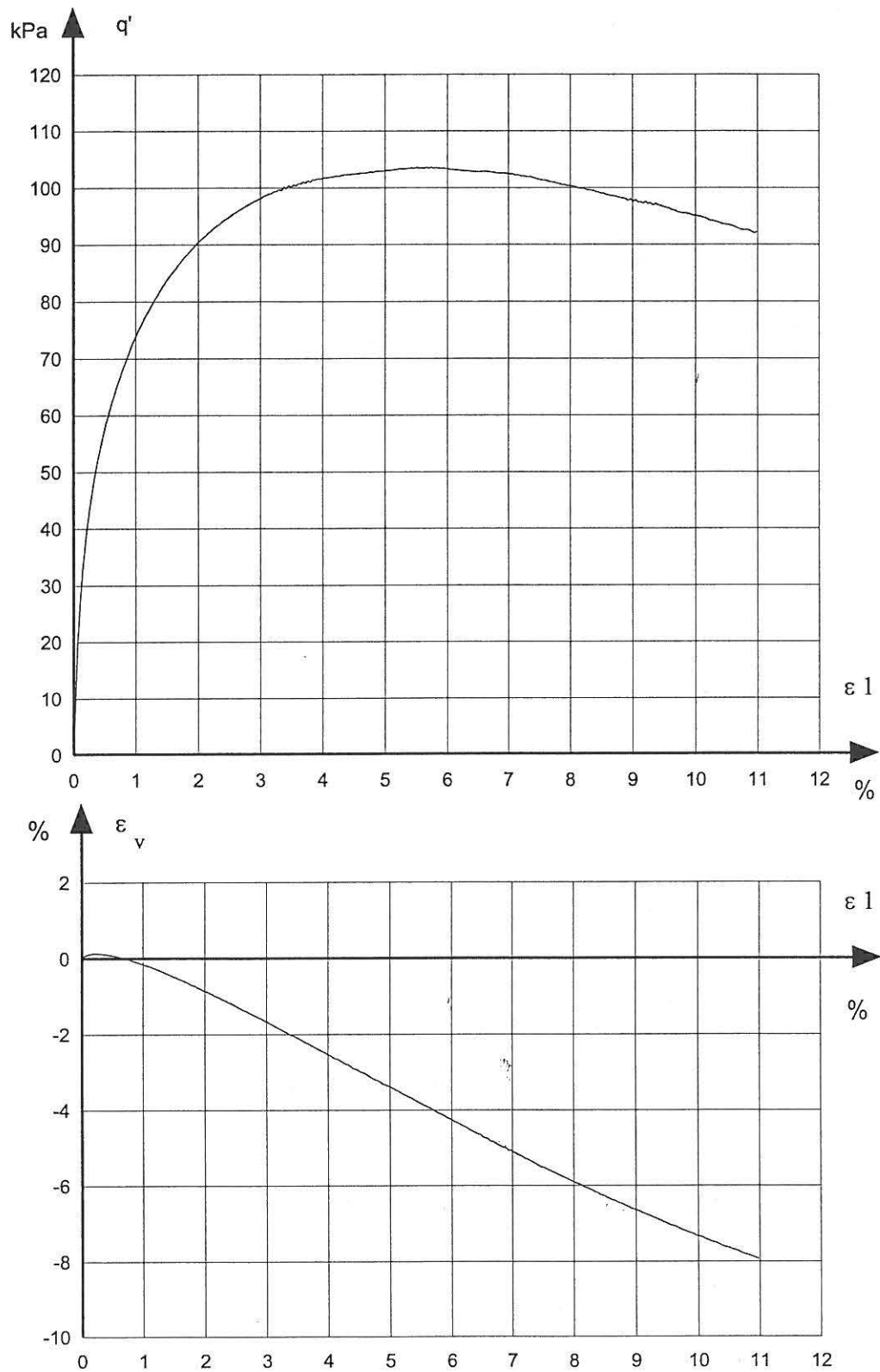
		Values at failure	Values for $\Delta\epsilon_v = 0$
Deviator stress	q'	103.64 kPa	43.14 kPa
Mean normal stress	p'	55.19 kPa	34.90 kPa
Confining pressures	σ_3	20.64 kPa	20.52 kPa
Vertical strain	ϵ_1	5.50 %	0.25 %
Volumetric strain	ϵ_v	-3.82 %	0.14 %



q'	p'	ϵ_1	ϵ_v
0.05	20.53	0.00	0.00
7.10	22.63	0.02	0.04
19.59	27.17	0.06	0.09
30.10	30.68	0.12	0.12
43.14	34.90	0.25	0.14
52.61	38.18	0.39	0.12
65.87	42.47	0.71	-0.00
75.37	45.76	1.05	-0.18
82.44	48.00	1.41	-0.41
89.63	50.52	1.92	-0.79
92.00	51.31	2.15	-0.97
97.71	53.21	2.91	-1.58
99.73	54.01	3.29	-1.92
101.05	54.45	3.68	-2.24
101.85	54.59	4.07	-2.60
102.47	54.92	4.46	-2.93
102.93	55.08	4.84	-3.26
103.64	55.19	5.50	-3.82
102.94	55.08	6.39	-4.59
102.32	54.87	7.16	-5.23
100.47	54.38	7.94	-5.85
98.64	53.52	8.65	-6.38
96.62	52.85	9.52	-7.01
94.11	52.01	10.31	-7.52

Job:	Encl. No
Lund No 0	73
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1 = 0.093$

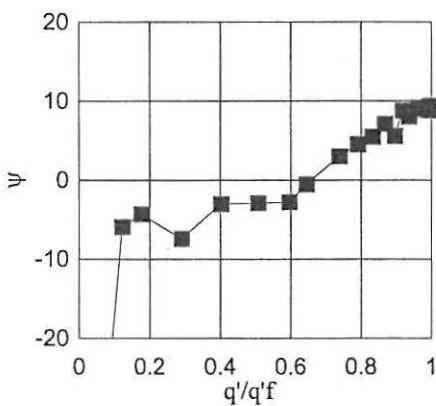
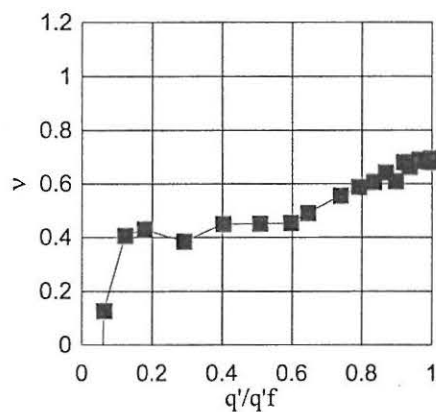


Job:	Encl. No
Lund No 0	74
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	23.3	Before test	At failure	
		Grain density	2.65			
Calibration file	Date	Void ratio	0.611			0.641
k2000_892	04.09.89	Saturation	1.01			
		Dimension H mm	71.5			
		D mm	69.7			

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-620 kPa
		ϵ_1	0.240 %
		ϵ_v	0.559 %
	2. Drained compression.		
Deformation rate:			8.4 % ph

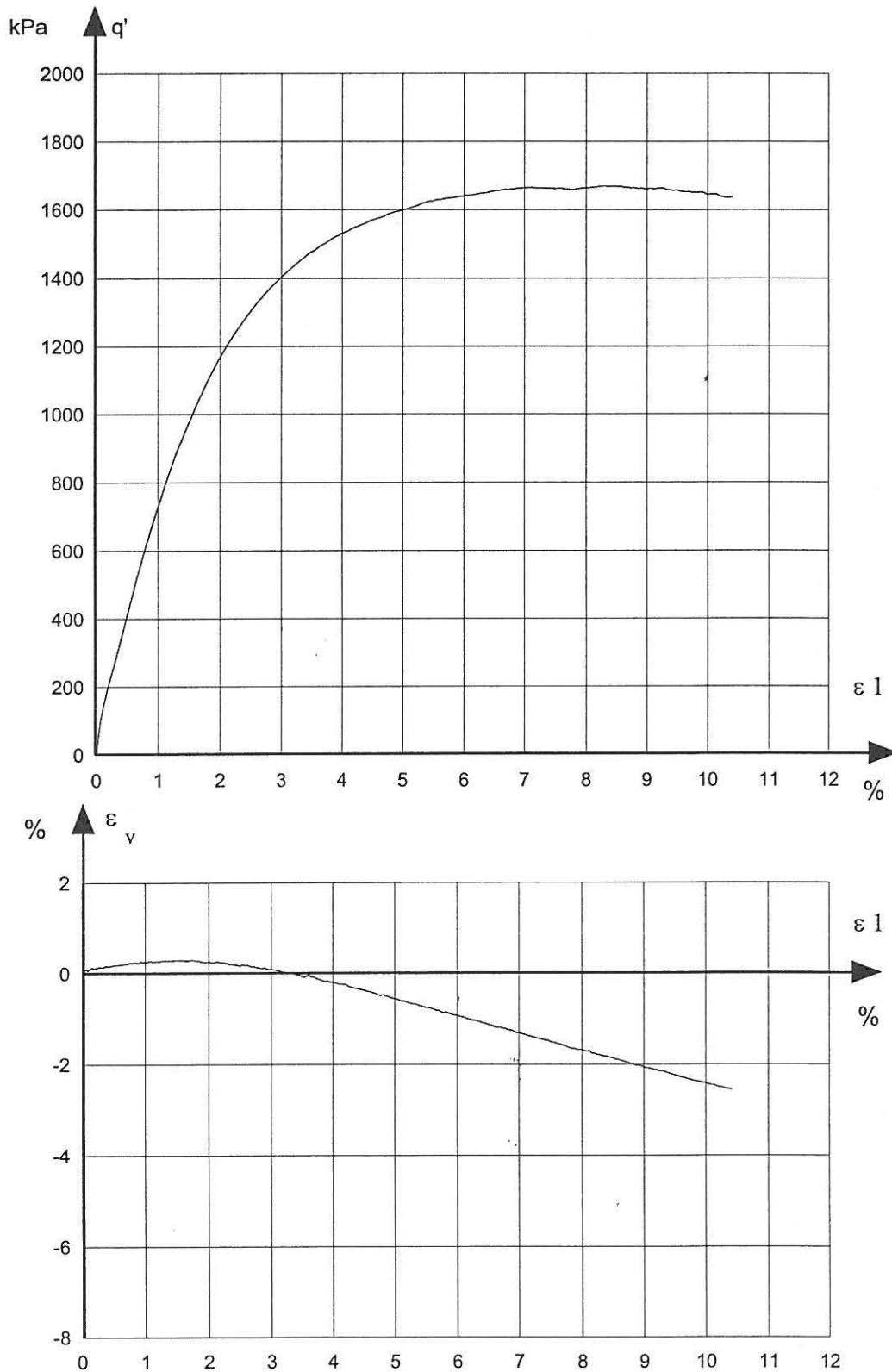
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 670.01	kPa	1077.74	kPa
Mean normal stress	p'	1 184.67	kPa	986.99	kPa
Confining pressures	σ_3	628.00	kPa	627.75	kPa
Vertical strain	ϵ_1	8.46	%	1.73	%
Volumetric strain	ϵ_v	-1.85	%	0.30	%



q'	p'	ϵ_1	ϵ_v
-0.58	622.81	0.00	0.00
13.39	626.47	0.00	0.05
103.04	657.35	0.07	0.10
202.98	692.41	0.19	0.12
294.52	723.92	0.32	0.14
486.33	788.86	0.60	0.20
672.31	846.35	0.90	0.23
847.37	907.46	1.21	0.26
995.98	956.99	1.53	0.29
1077.74	986.99	1.73	0.30
1235.06	1037.44	2.21	0.25
1321.89	1066.13	2.56	0.19
1391.80	1090.68	2.92	0.11
1449.32	1110.61	3.28	0.01
1496.44	1126.81	3.65	-0.07
1534.42	1139.47	4.02	-0.20
1563.18	1147.56	4.39	-0.32
1608.49	1163.41	5.14	-0.61
1638.43	1173.14	5.90	-0.89
1659.13	1180.04	6.65	-1.18
1663.96	1179.65	7.42	-1.48
1670.01	1184.67	8.46	-1.85
1662.39	1180.88	8.96	-2.04
1652.22	1177.74	9.74	-2.32

Job:	Encl. No
Lund No 0	75
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.099

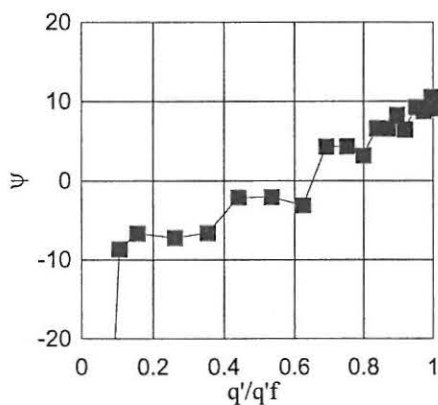
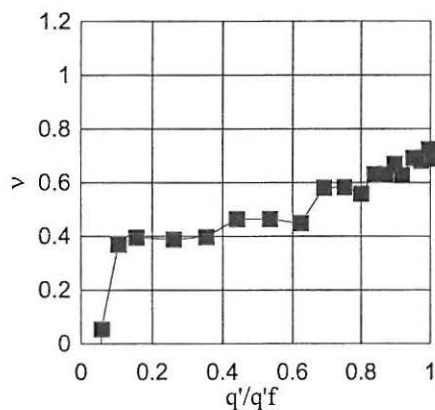


Job:	Encl. No
Lund No 0	76
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.6	
Calibration file	Date	Void ratio	0.610	0.640
k2000_892	05.09.89	Saturation	1.02	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-620 kPa
		ϵ_1	0.257 %
		ϵ_v	0.521 %
	2. Drained compression.		
Deformation rate:			8.4 % ph

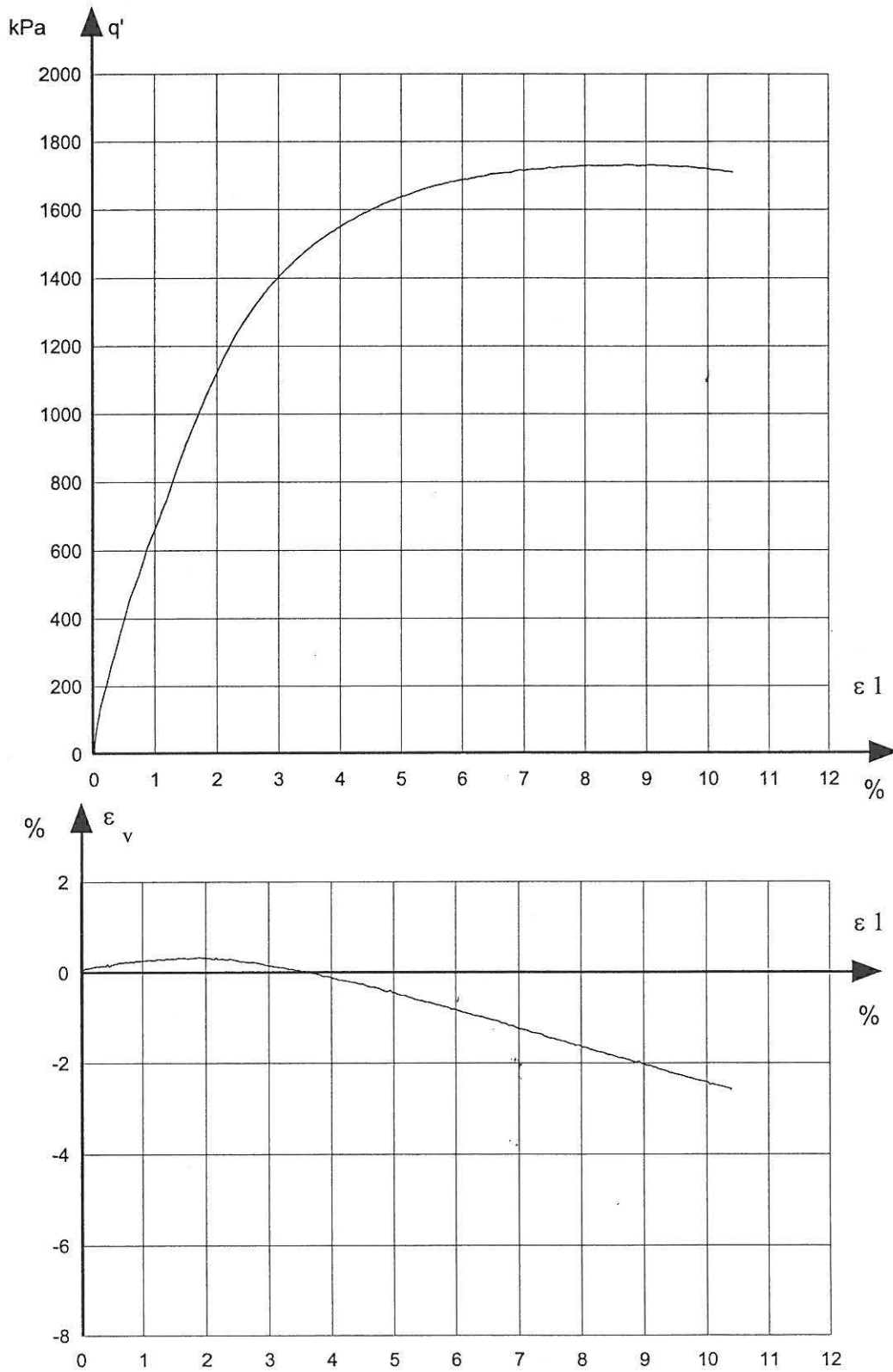
		Values at failure	Values for $\Delta \epsilon_v = 0$
Deviator stress	q'	1 732.90 kPa	1083.19 kPa
Mean normal stress	p'	1 203.13 kPa	984.56 kPa
Confining pressures	σ_3	625.50 kPa	623.50 kPa
Vertical strain	ϵ_1	8.68 %	1.89 %
Volumetric strain	ϵ_v	-1.89 %	0.35 %



q'	p'	ϵ_1	ϵ_v
0.00	623.00	0.00	0.00
12.80	627.52	0.00	0.03
100.05	655.60	0.06	0.08
181.35	682.45	0.16	0.11
268.26	711.17	0.29	0.14
453.05	774.02	0.58	0.20
613.74	828.33	0.89	0.26
763.42	880.72	1.21	0.29
925.97	934.16	1.53	0.31
1083.19	984.56	1.89	0.35
1199.08	1025.69	2.20	0.30
1302.61	1059.20	2.55	0.24
1385.52	1087.84	2.90	0.20
1452.77	1110.01	3.27	0.11
1508.07	1128.69	3.63	0.01
1552.78	1144.59	4.00	-0.11
1591.30	1157.43	4.38	-0.21
1646.67	1175.39	5.13	-0.49
1685.00	1188.16	5.89	-0.77
1708.41	1194.72	6.65	-1.06
1724.28	1200.01	7.41	-1.40
1729.46	1201.49	8.18	-1.70
1732.90	1203.13	8.68	-1.89
1726.16	1200.89	9.73	-2.31

Job:	Encl. No
Lund No 0	77
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.115

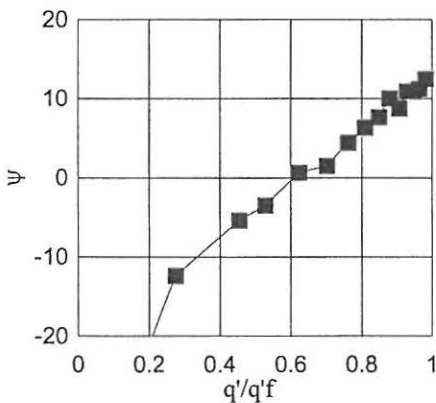
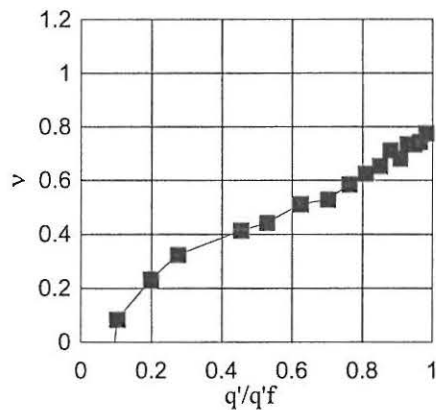


Job:	Encl. No
Lund No 0	78
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	23.3	At failure 0.635
		Grain density	2.65	
Calibration file	Date	Void ratio	0.600	
k2000_892	06.09.89	Saturation	1.03	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-310 kPa
		ϵ_1	0.096 %
		ϵ_v	0.378 %
	2. Drained compression.	Deformation rate: 8.7 % ph	

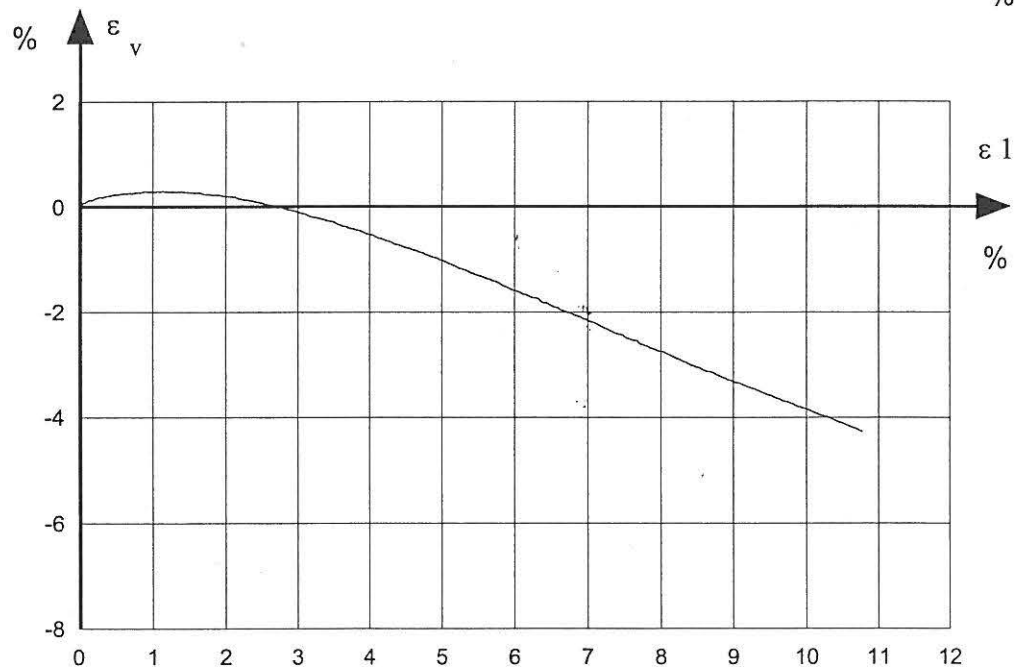
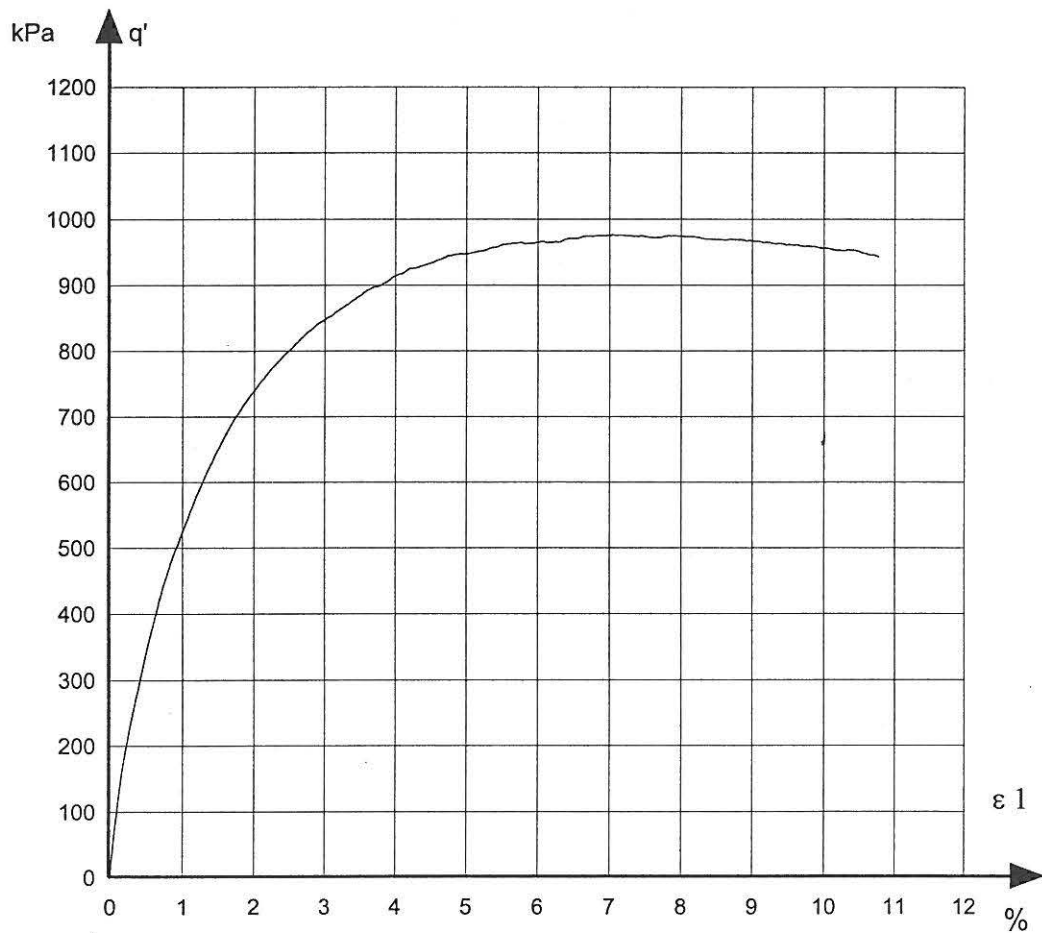
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	976.73	kPa	564.49	kPa
Mean normal stress	p'	634.20	kPa	495.79	kPa
Confining pressures	σ_3	308.63	kPa	307.63	kPa
Vertical strain	ϵ_1	7.05	%	1.14	%
Volumetric strain	ϵ_v	-2.17	%	0.31	%



q'	p'	ϵ_1	ϵ_v
0.00	306.13	0.00	0.00
19.79	313.47	0.01	0.03
101.28	340.39	0.08	0.09
193.14	371.76	0.20	0.15
269.11	397.83	0.34	0.20
443.76	455.05	0.74	0.27
515.69	480.52	0.97	0.30
608.93	510.60	1.31	0.29
685.85	536.49	1.66	0.27
744.06	555.15	2.02	0.21
790.07	572.23	2.39	0.12
829.75	586.46	2.76	0.01
858.43	593.52	3.14	-0.15
885.83	603.40	3.51	-0.29
906.94	609.44	3.89	-0.46
926.65	618.01	4.27	-0.64
941.05	622.56	4.66	-0.83
958.89	627.01	5.43	-1.25
965.78	628.55	6.16	-1.67
976.42	633.35	6.97	-2.13
974.16	631.85	7.75	-2.61
970.06	630.48	8.53	-3.05
963.36	627.25	9.31	-3.47
955.53	625.14	10.10	-3.90

Job:	Encl. No
Lund No 0	79
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.051

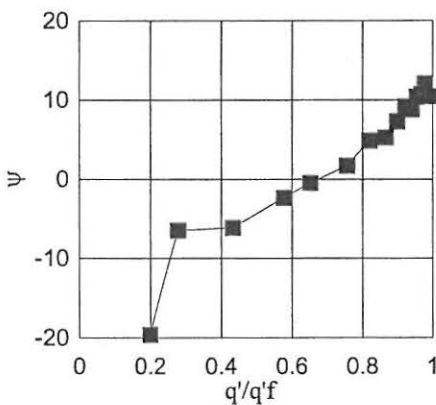
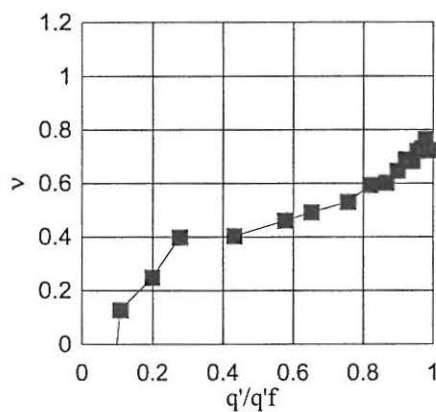


Job:	Encl. No
Lund No 0	80
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	22.2	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.603		
k2000_892	18.09.89	Saturation	0.98		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.		σ_3	100-310 kPa
			ϵ_1	0.115 %
			ϵ_v	0.257 %
	2. Drained compression.		Deformation rate: 8.5 % ph	

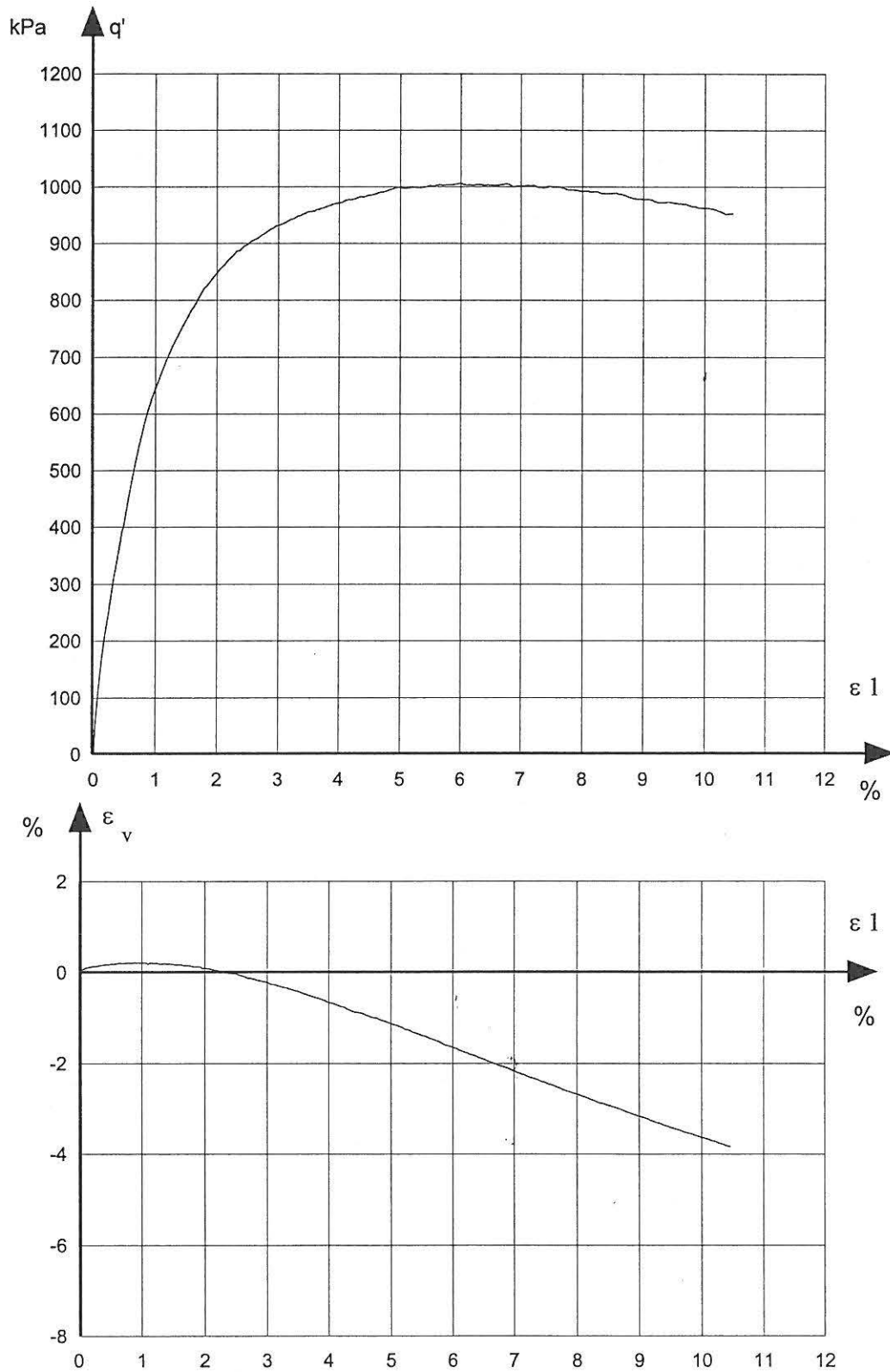
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	1 006.93	kPa	656.76	kPa
Mean normal stress	p'	648.02	kPa	528.29	kPa
Confining pressures	σ_3	312.38	kPa	309.38	kPa
Vertical strain	ϵ_1	5.98	%	1.04	%
Volumetric strain	ϵ_v	-1.62	%	0.22	%



q'	p'	ϵ_1	ϵ_v
-0.58	308.93	0.00	0.00
10.46	313.61	0.01	0.02
108.63	347.08	0.07	0.07
200.31	379.65	0.16	0.12
279.61	403.58	0.28	0.14
435.34	455.99	0.53	0.19
580.83	504.24	0.82	0.21
656.76	528.29	1.04	0.22
762.06	563.65	1.47	0.19
825.10	587.41	1.82	0.12
870.97	602.45	2.18	0.05
903.58	612.32	2.54	-0.05
927.58	620.32	2.91	-0.19
946.24	625.79	3.29	-0.33
960.46	630.28	3.67	-0.49
973.28	635.55	4.05	-0.67
983.46	638.69	4.42	-0.87
999.60	644.57	5.18	-1.20
1006.93	648.02	5.98	-1.62
1004.97	646.87	6.71	-2.00
1000.78	645.47	7.47	-2.39
992.23	641.87	8.24	-2.79
978.23	636.70	9.02	-3.16
965.87	632.33	9.80	-3.53

Job:	Encl. No
Lund No 0	81
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.108

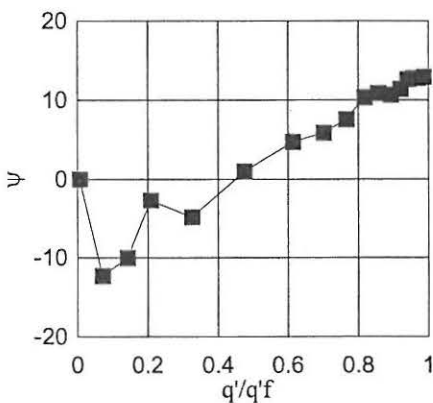
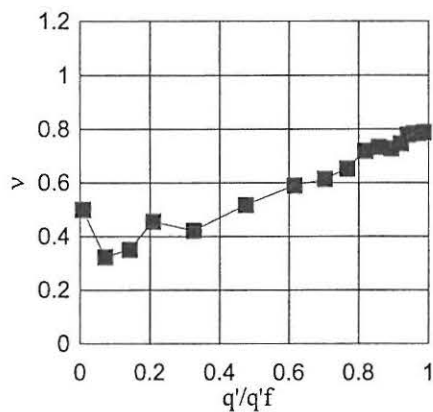


Job:	Encl. No
Lund No 0	82
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	23.1	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.617		
k2000_892	11.09.89	Saturation	0.99		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM	Drained compression.		
CD - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.052 %
		ϵ_v	0.076 %
	2. Drained compression.		
	Deformation rate:		8.8 % ph

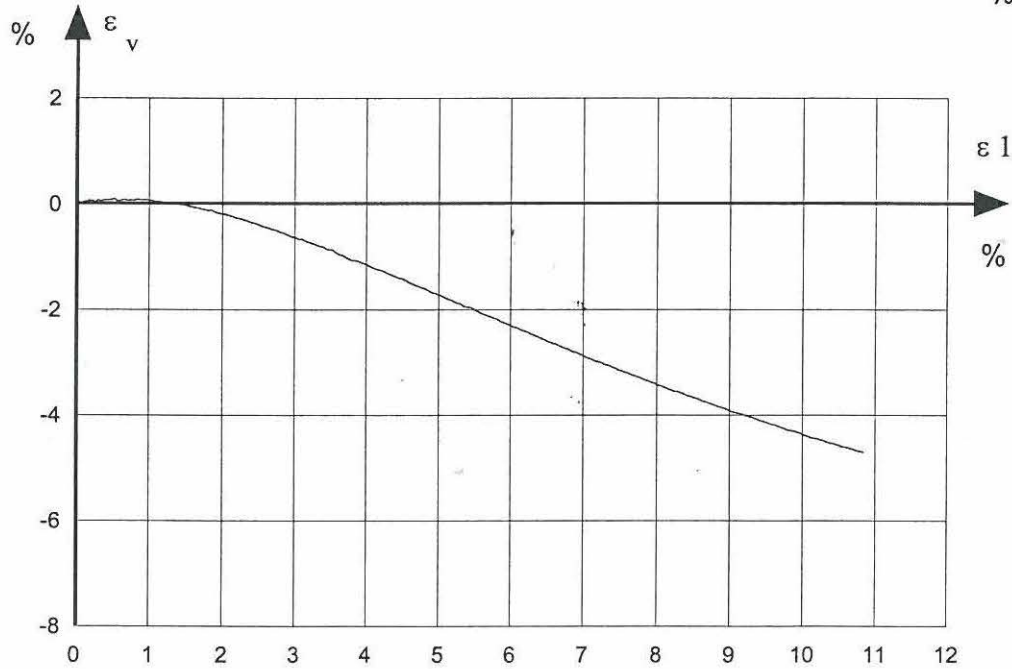
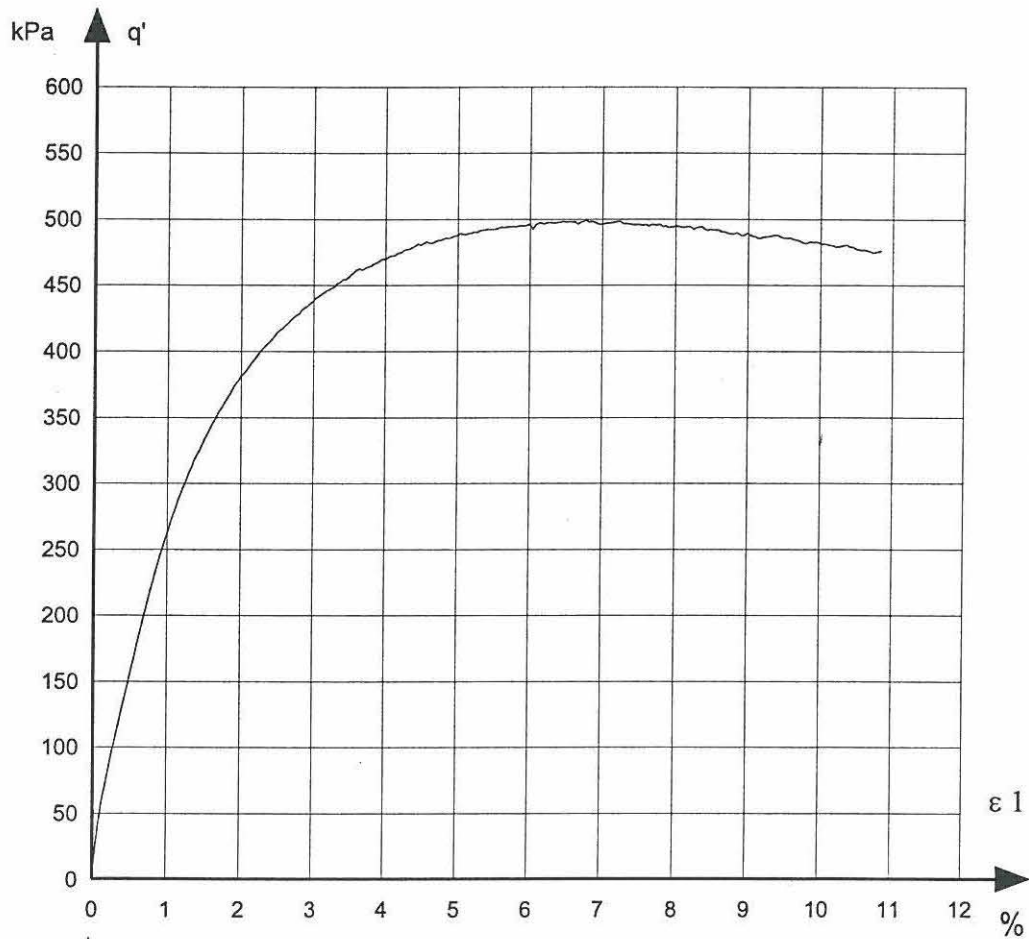
		Values at failure		Values for $\Delta \epsilon_v = 0$	
Deviator stress	q'	499.98	kPa	162.85	kPa
Mean normal stress	p'	325.10	kPa	211.47	kPa
Confining pressures	σ_3	158.44	kPa	157.19	kPa
Vertical strain	ϵ_1	6.77	%	0.53	%
Volumetric strain	ϵ_v	-2.73	%	0.10	%



q'	p'	ϵ_1	ϵ_v
0.00	156.94	0.00	0.00
3.48	158.60	0.00	0.00
35.37	169.23	0.06	0.02
70.68	180.75	0.16	0.05
104.16	191.41	0.28	0.06
162.85	211.47	0.53	0.10
237.68	236.16	0.86	0.09
307.48	258.18	1.28	0.01
351.10	274.22	1.65	-0.07
383.39	284.73	2.03	-0.19
408.90	292.74	2.41	-0.35
428.45	299.75	2.79	-0.53
446.09	305.64	3.17	-0.70
460.12	310.56	3.56	-0.89
469.97	313.84	3.94	-1.11
478.01	316.52	4.33	-1.33
484.30	318.87	4.72	-1.54
492.77	321.69	5.49	-1.99
499.98	325.10	6.77	-2.73
497.17	322.16	7.04	-2.90
494.67	321.33	7.82	-3.31
492.21	320.51	8.59	-3.70
488.26	320.69	9.37	-4.07
480.32	317.04	10.16	-4.44

Job:	Encl. No
Lund No 0	83
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1$ 0.147

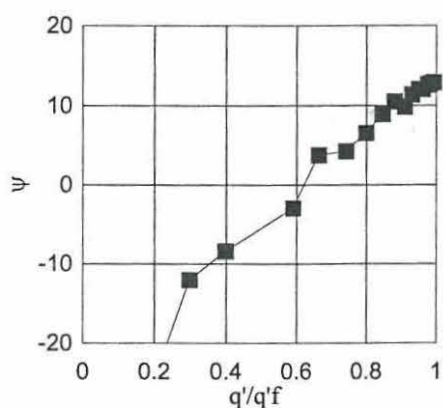
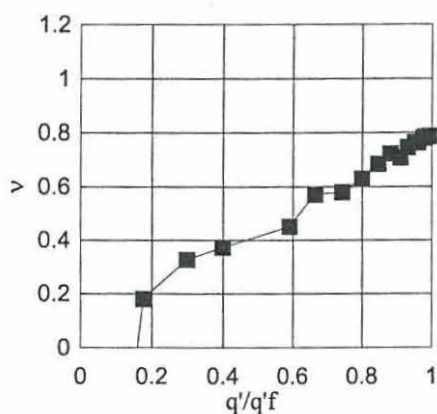


Job:	Lund No 0	Encl. No	84
Exc:	LBI, AMS	Check:	FRJ

Description of soil			Before test	At failure
Lund No 0		Water content %	22.2	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.605	0.642
k2000_892	19.09.89	Saturation	0.97	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.040 %
		ϵ_v	0.034 %
	2. Drained compression.	Deformation rate: 8.8 % ph	

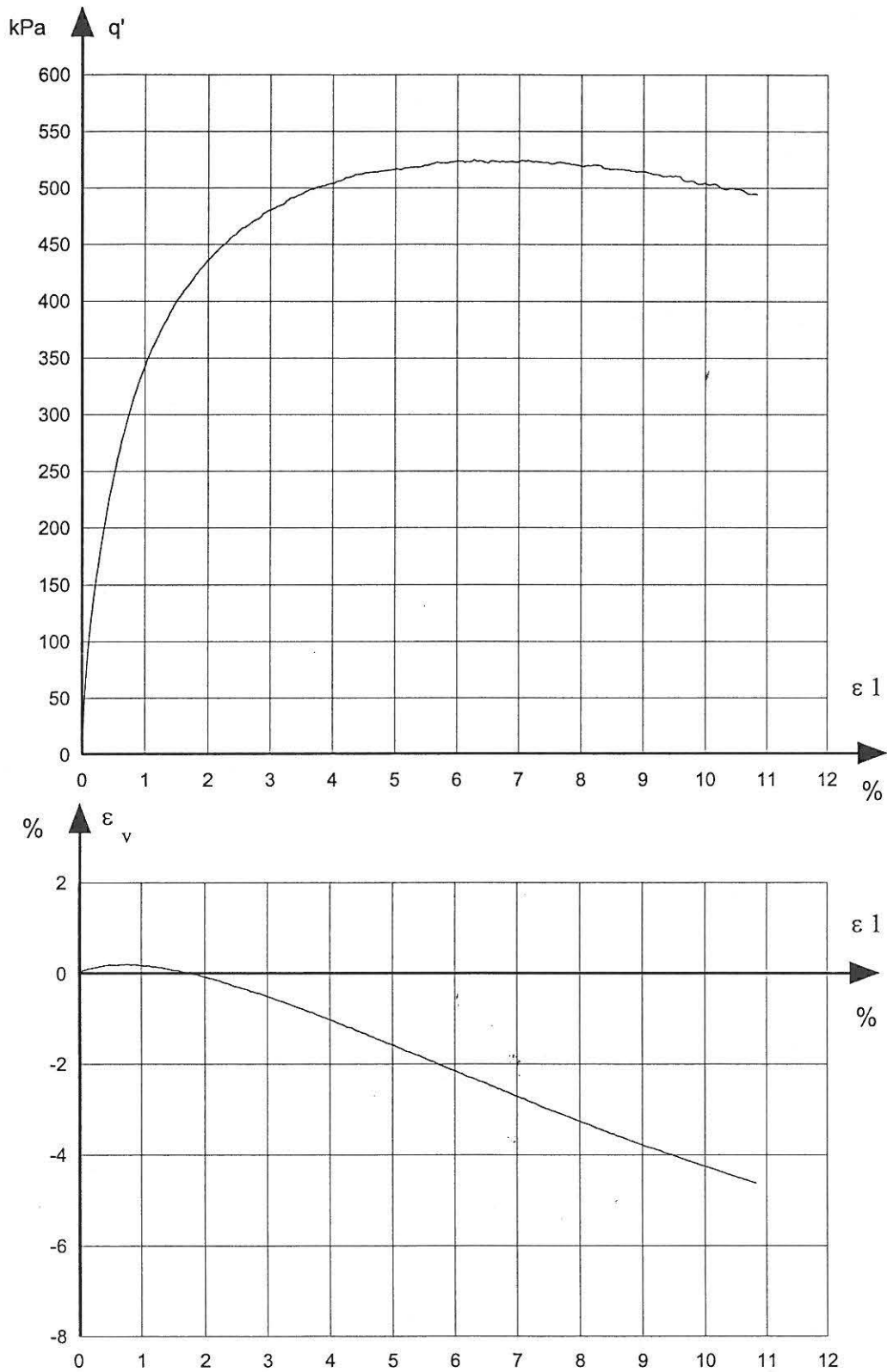
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	525.28	kPa	310.04	kPa
Mean normal stress	p'	332.03	kPa	259.78	kPa
Confining pressures	σ_3	156.94	kPa	156.44	kPa
Vertical strain	ϵ_1	6.27	%	0.79	%
Volumetric strain	ϵ_v	-2.29	%	0.20	%



q'	p'	ϵ_1	ϵ_v
0.00	156.69	0.00	0.00
15.66	161.91	0.01	0.03
92.16	187.66	0.09	0.08
156.37	209.31	0.22	0.12
208.84	225.55	0.37	0.16
310.04	259.78	0.79	0.20
348.83	272.71	1.03	0.17
389.72	287.09	1.39	0.11
419.80	296.37	1.76	0.02
444.28	305.28	2.13	-0.12
462.70	312.17	2.50	-0.28
477.58	316.63	2.88	-0.44
488.77	319.86	3.26	-0.62
499.17	323.58	3.64	-0.82
504.47	324.85	4.02	-1.02
512.37	328.48	4.40	-1.23
515.23	327.68	4.78	-1.45
520.70	330.26	5.54	-1.89
525.28	332.03	6.27	-2.29
524.20	332.42	7.07	-2.75
520.91	330.82	7.84	-3.16
516.49	328.85	8.61	-3.58
510.74	327.43	9.39	-3.96
503.07	325.13	10.17	-4.32

Job:	Encl. No
Lund No 0	85
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.165

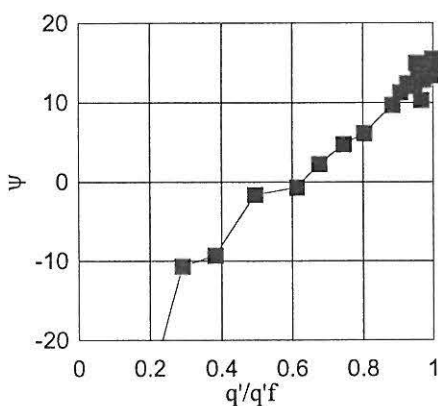
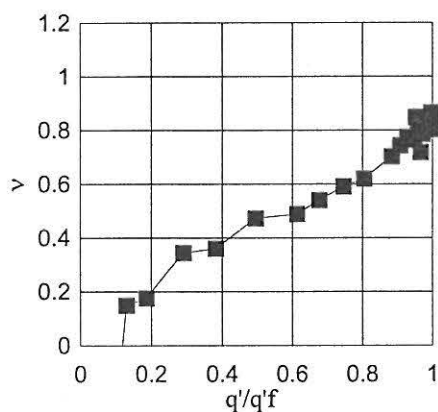


Job:	Encl. No
Lund No 0	86
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	25.6	At failure 0.617
		Grain density	2.65	
Calibration file	Date	Void ratio	0.567	
k2000_892	14.09.89	Saturation	1.19	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.036 %
		ϵ_v	0.079 %
	2. Drained compression.	Deformation rate: 7.9 % ph	

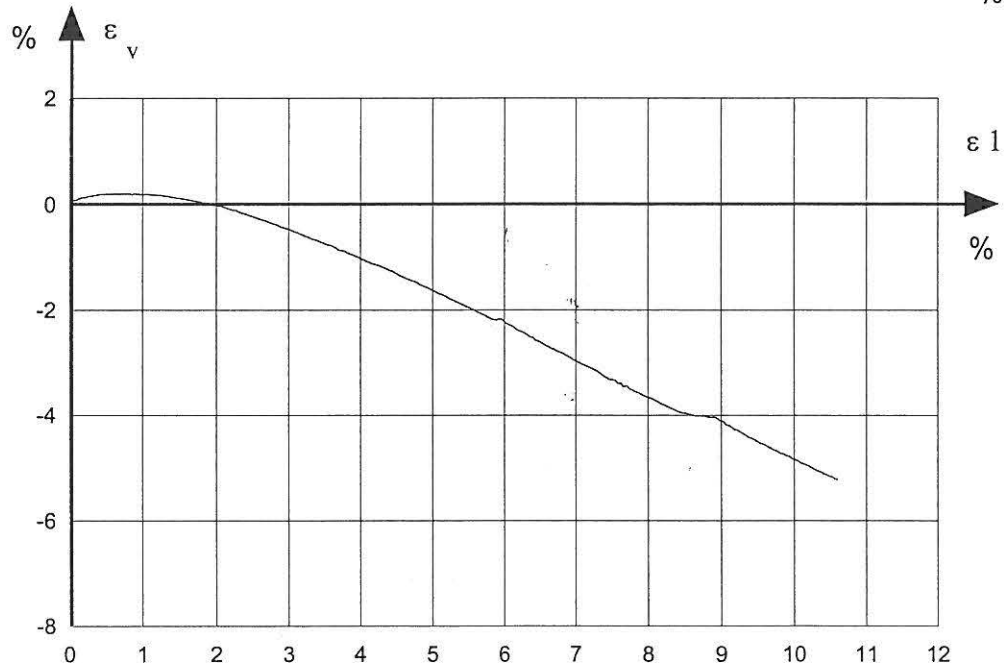
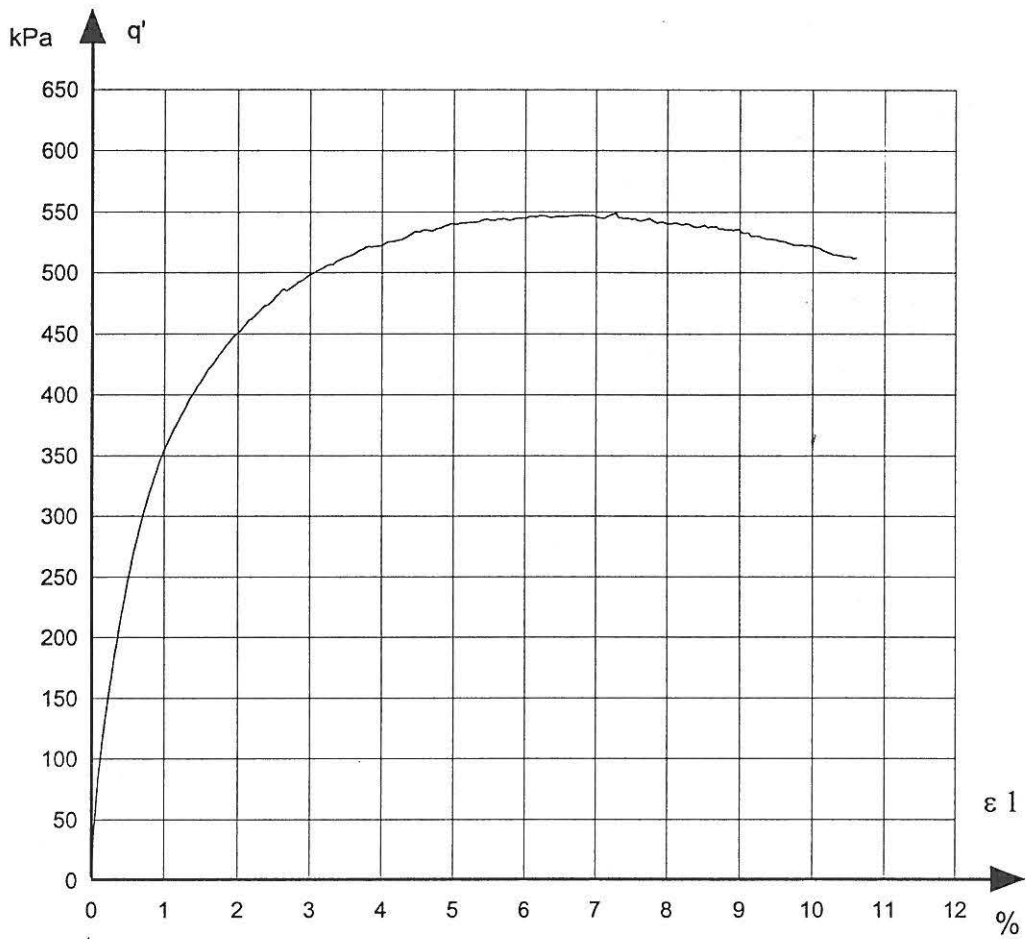
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	549.03	kPa	336.99	kPa
Mean normal stress	p'	341.20	kPa	269.77	kPa
Confining pressures	σ_3	158.19	kPa	157.44	kPa
Vertical strain	ϵ_1	7.27	%	0.88	%
Volumetric strain	ϵ_v	-3.13	%	0.21	%



q'	p'	ϵ_1	ϵ_v
0.00	157.19	0.00	0.00
23.21	163.92	0.01	0.04
71.35	180.97	0.07	0.08
102.07	191.46	0.12	0.11
159.35	209.05	0.24	0.15
210.14	227.48	0.38	0.19
272.14	247.65	0.58	0.20
336.99	269.77	0.88	0.21
371.74	281.35	1.12	0.19
409.64	293.23	1.48	0.12
441.36	304.31	1.84	0.04
485.12	319.64	2.59	-0.26
498.01	322.44	2.97	-0.45
508.95	326.59	3.35	-0.66
519.77	330.69	3.73	-0.86
525.88	331.73	4.12	-1.08
533.57	335.29	4.50	-1.30
541.85	337.55	5.27	-1.79
546.19	339.00	6.04	-2.25
547.19	340.33	6.81	-2.81
549.03	341.20	7.27	-3.13
538.10	336.55	8.36	-3.89
530.24	333.43	9.14	-4.23
522.88	331.98	9.93	-4.78

Job:	Encl. No
Lund No 0	87
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.165

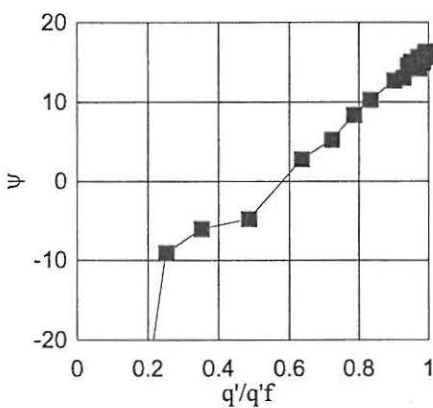
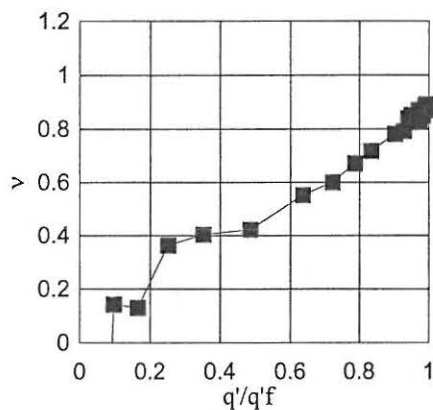


Job:	Lund No 0	Encl. No	88
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	20.9	At failure 0.617
		Grain density	2.65	
Calibration file	Date	Void ratio	0.566	
k2000_892	20.09.89	Saturation	0.98	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CD - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-160 kPa
		ϵ_1	0.031 %
		ϵ_v	0.068 %
	2. Drained compression.	Deformation rate: 8.6 % ph	

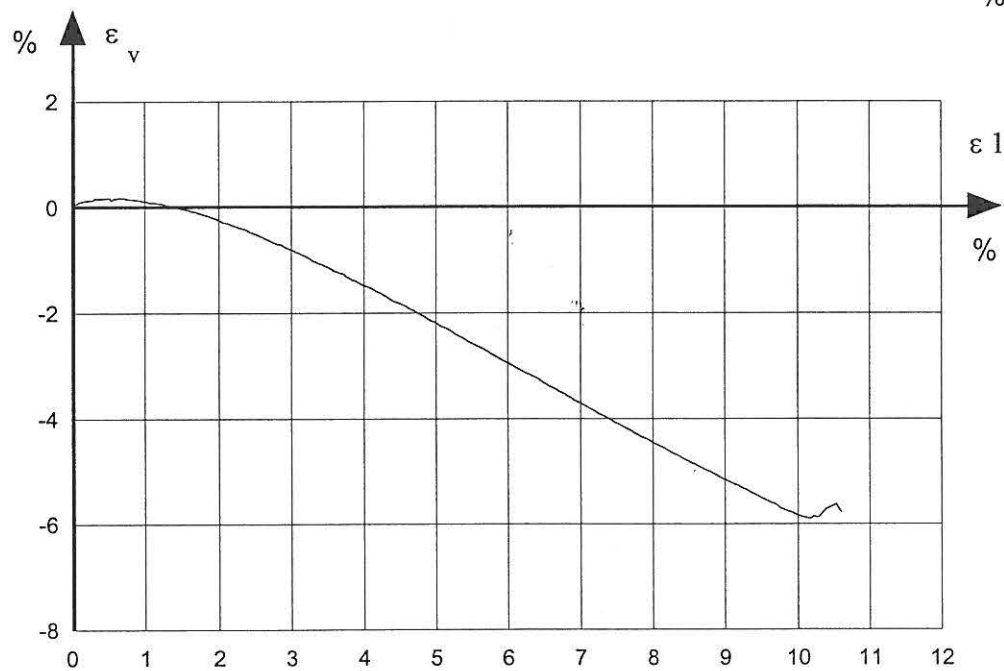
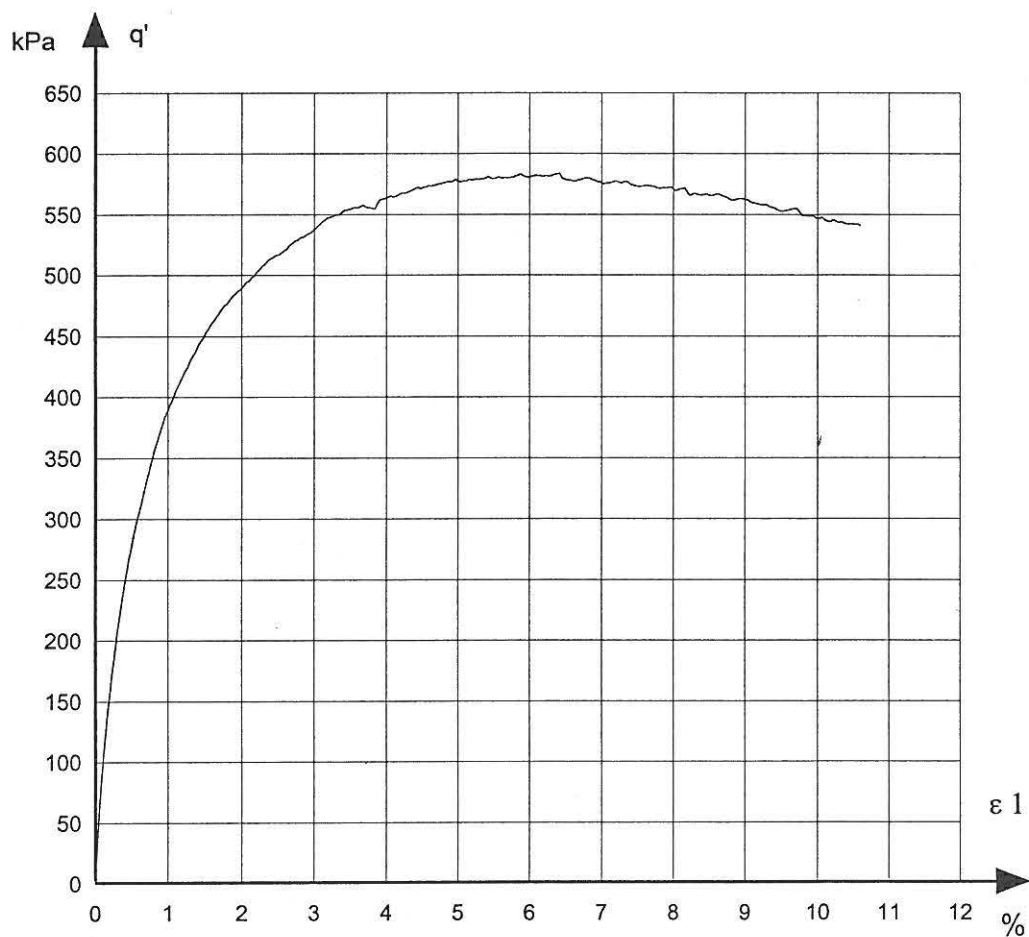
		Values at failure		Values for $\Delta\epsilon_v = 0$	
Deviator stress	q'	584.02	kPa	284.01	kPa
Mean normal stress	p'	353.11	kPa	251.36	kPa
Confining pressures	σ_3	158.44	kPa	156.69	kPa
Vertical strain	ϵ_1	6.40	%	0.51	%
Volumetric strain	ϵ_v	-3.23	%	0.18	%



q'	p'	ϵ_1	ϵ_v
0.00	156.69	0.00	0.00
12.77	160.94	0.00	0.04
55.73	175.51	0.04	0.06
95.77	188.86	0.09	0.10
147.34	206.55	0.18	0.12
205.73	225.76	0.29	0.15
284.01	251.36	0.51	0.18
370.99	280.85	0.88	0.14
421.67	297.74	1.22	0.08
459.61	310.39	1.58	-0.05
486.71	318.92	1.94	-0.20
526.44	332.67	2.69	-0.62
541.68	338.00	3.07	-0.84
553.73	342.26	3.44	-1.09
554.48	338.77	3.83	-1.36
567.93	345.75	4.20	-1.61
574.00	349.02	4.58	-1.87
579.64	350.40	5.34	-2.45
584.02	353.11	6.40	-3.23
579.13	349.73	6.86	-3.59
577.45	349.92	7.32	-3.94
566.48	345.76	8.39	-4.73
558.83	343.21	9.16	-5.27
549.32	341.04	9.94	-5.79

Job:	Encl. No
Lund No 0	89
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta\epsilon_1$ 0.092

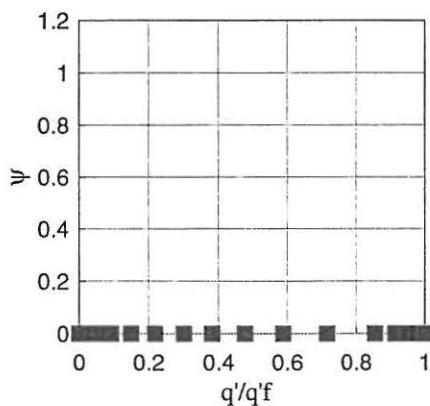
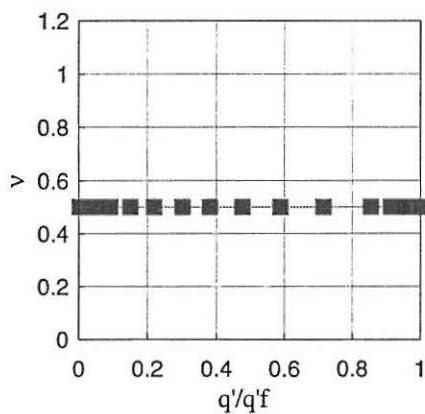


Job:	Lund No 0	Encl. No	90
Exc:	LBI, AMS	Check:	FRJ

Description of soil Lund No 0		Water content %	Before test 23	At failure 0.555
		Grain density	2.65	
Calibration file	Date	Void ratio	0.555	
k2000_891	25.09.89	Saturation	1.01	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.	σ_3	5 kPa	
		ϵ_1	0.000 %	
		ϵ_v	0.000 %	
	2. Undrained compression.			
Deformation rate:		9.1 % ph		

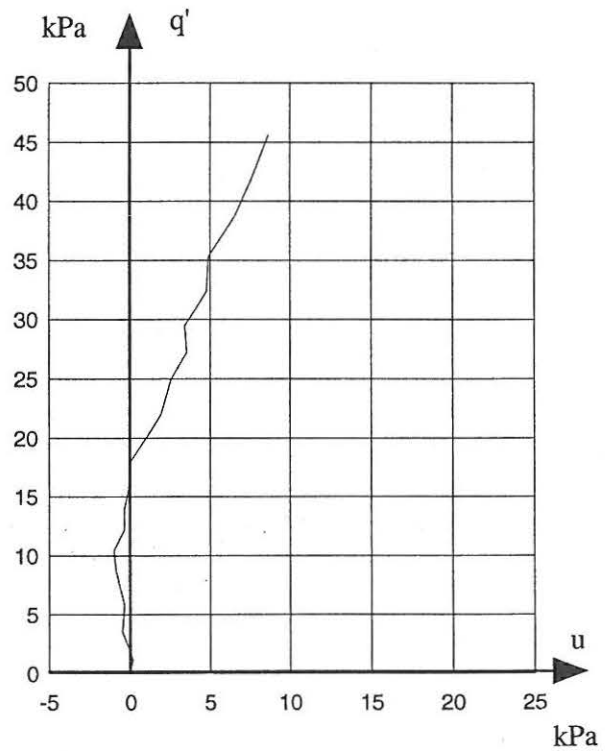
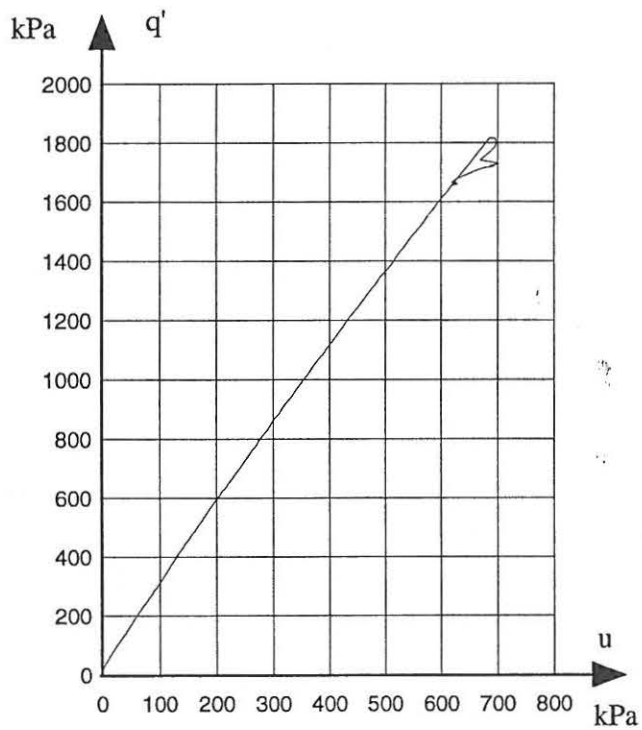
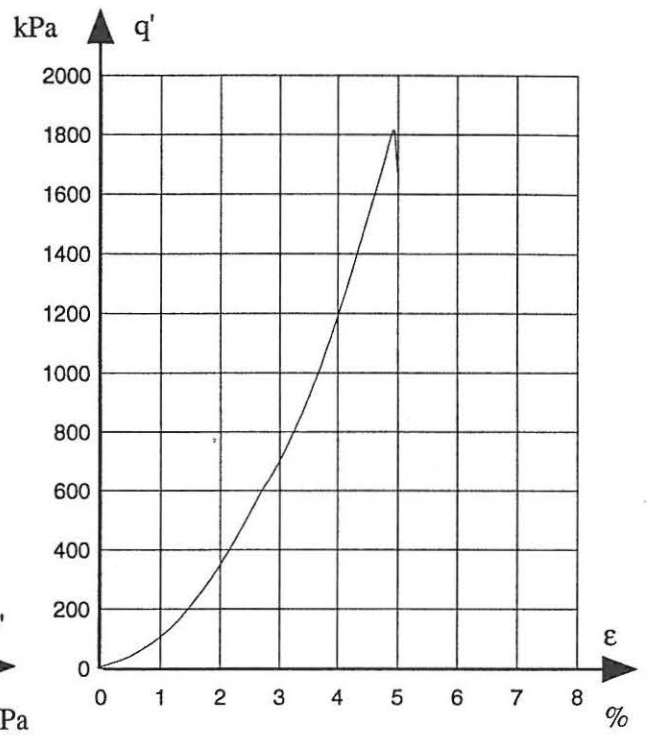
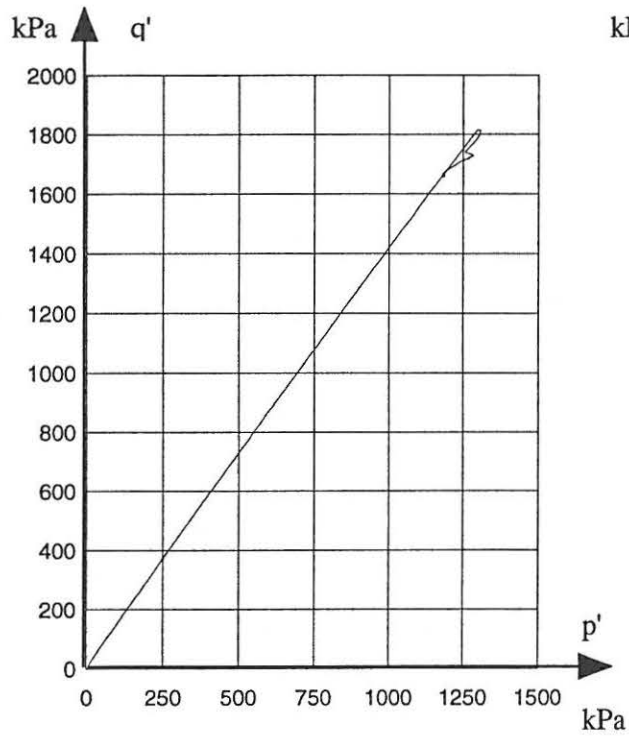
		Maximum values		Minimum values for σ	
Deviator stress	q'	1 816.04	kPa	10.44	kPa
Mean normal stress	p'	1 296.90	kPa	8.11	kPa
Confining pressures	σ_3	691.55	kPa	4.63	kPa
Vertical strain	ϵ_1	4.91	%	0.08	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.00	5.63	0.00	0.00
5.81	7.19	0.02	0.00
10.44	8.11	0.08	0.00
15.65	10.72	0.17	0.00
41.60	27.00	0.51	0.00
89.78	58.07	0.89	0.00
165.62	109.00	1.31	0.00
271.62	181.24	1.73	0.00
398.32	267.51	2.15	0.00
547.16	370.91	2.56	0.00
692.43	473.88	2.98	0.00
863.98	593.74	3.39	0.00
1066.61	740.72	3.78	0.00
1297.18	910.15	4.17	0.00
1552.08	1098.57	4.54	0.00
1816.04	1296.90	4.91	0.00
1751.64	1265.68	4.96	0.00
1719.46	1270.46	4.97	0.00
1710.54	1245.97	4.98	0.00
1700.52	1228.12	4.98	0.00
1689.41	1211.78	4.99	0.00
1678.84	1195.25	4.99	0.00
1667.73	1182.04	5.00	0.00
1658.31	1185.02	5.00	0.00

Job:	Encl. No
Lund No 0	1
Exc:	Check:
LBI	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = 0.000$
No isotropic consolidation.

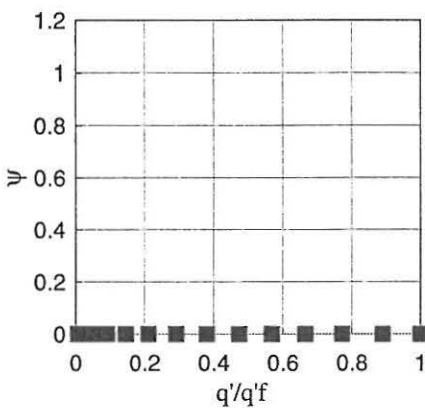
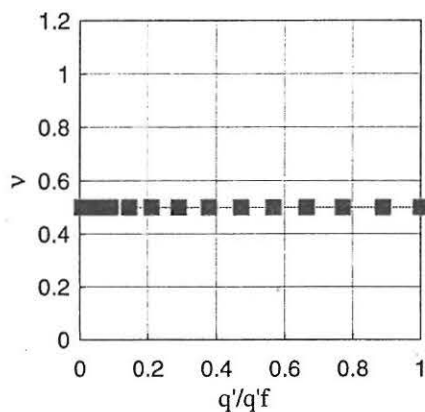


Job:	Encl. No
Lund No 0	2
Exc:	Check:
LBI	FRJ

Description of soil Lund No 0		Water content %	22.3	Before test	At failure	
		Grain density	2.65			
Calibration file	Date	Void ratio	0.551			0.551
kal9	26.09.89	Saturation	1.07			
		Dimension H mm	71.5			
		D mm	69.7			

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-5 kPa
		ϵ_1	-0.071 %
		ϵ_v	-0.461 %
	2.Undrained compression.		
	Deformation rate:		7.5 % ph

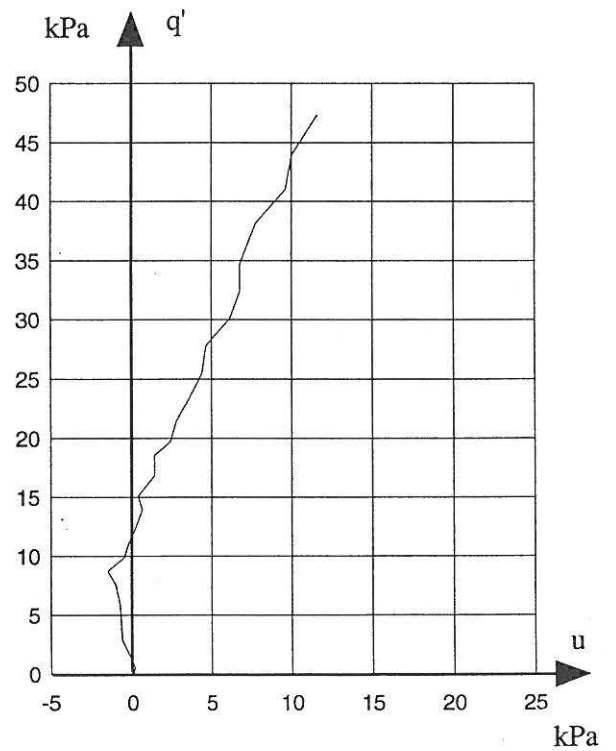
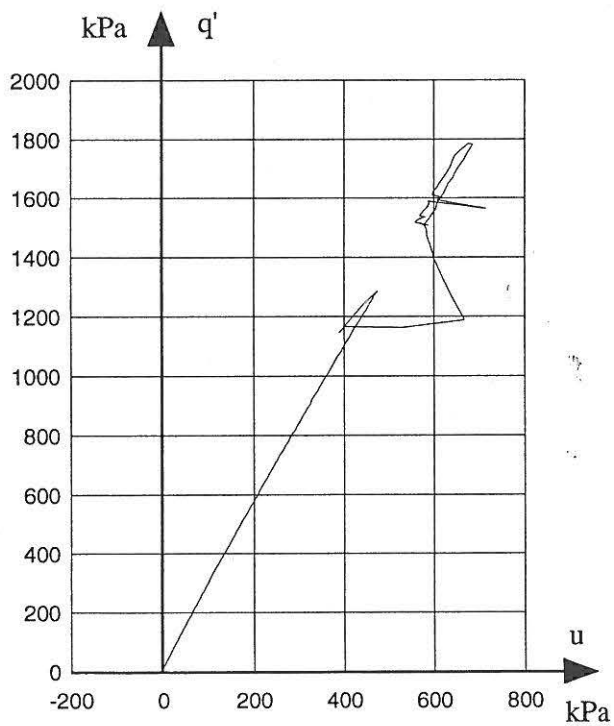
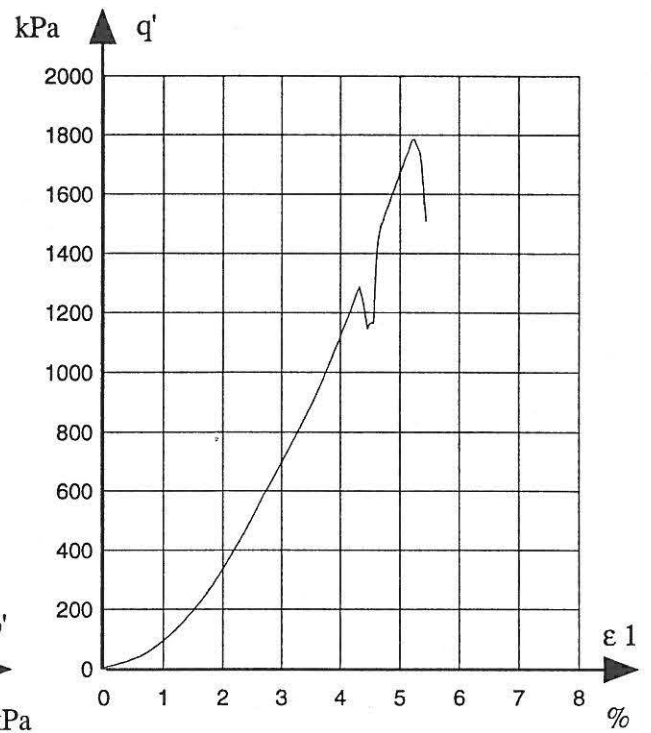
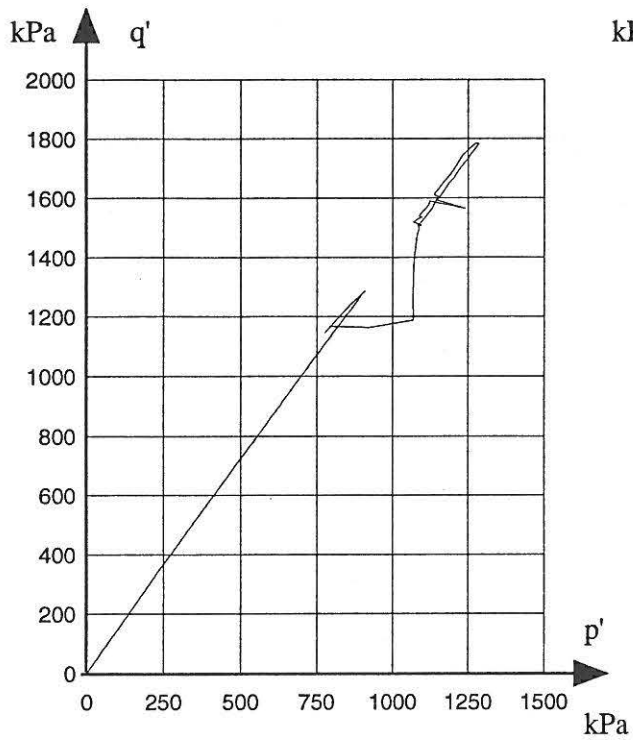
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	1 286.04	kPa	8.71	kPa
Mean normal stress	p'	907.94	kPa	6.16	kPa
Confining pressures	σ_3	479.26	kPa	3.25	kPa
Vertical strain	ϵ_1	4.32	%	0.09	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
-0.58	4.56	0.00	0.00
8.71	6.16	0.09	0.00
12.18	8.94	0.17	0.00
30.08	20.91	0.46	0.00
62.84	42.71	0.79	0.00
116.64	78.29	1.13	0.00
186.68	125.78	1.46	0.00
272.23	184.94	1.79	0.00
374.29	253.87	2.12	0.00
487.56	332.78	2.44	0.00
608.00	417.71	2.76	0.00
728.74	502.50	3.09	0.00
854.29	593.14	3.41	0.00
991.89	692.17	3.73	0.00
1143.69	801.94	4.04	0.00
1286.04	907.94	4.32	0.00
1472.18	1079.20	4.66	0.00
1649.72	1186.29	4.96	0.00
1618.04	1140.70	5.39	0.00
1573.79	1116.32	5.41	0.00
1549.98	1095.37	5.42	0.00
1532.31	1089.36	5.42	0.00
1523.44	1074.89	5.43	0.00
1508.00	1092.14	5.43	0.00

Job:	Encl. No
Lund No 0	3
Exc:	Check:
LB1, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.084$

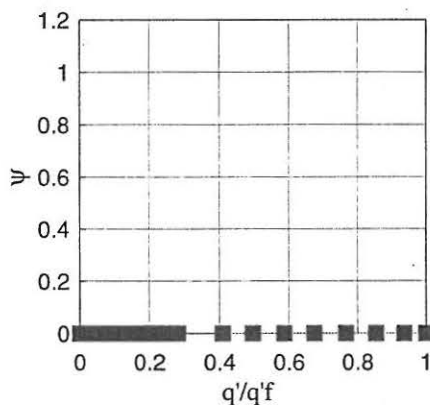
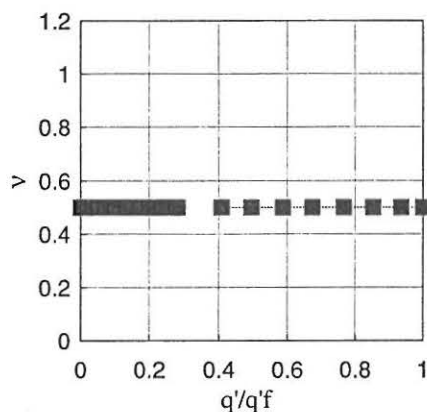


Job:	Encl. No
Lund No 0	4
Exc:	Check:
LBI, AMS	FRJ

Description of soil			Before test	At failure
Lund No 0		Water content %	25.4	
		Grain density	2.65	
Calibration file	Date	Void ratio	0.611	0.611
k2000_891	26.09.89	Saturation	1.10	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-0 kPa
		ϵ_1	-0.082 %
		ϵ_v	-0.355 %
	2. Undrained compression.		
Deformation rate:		5.5 % ph	

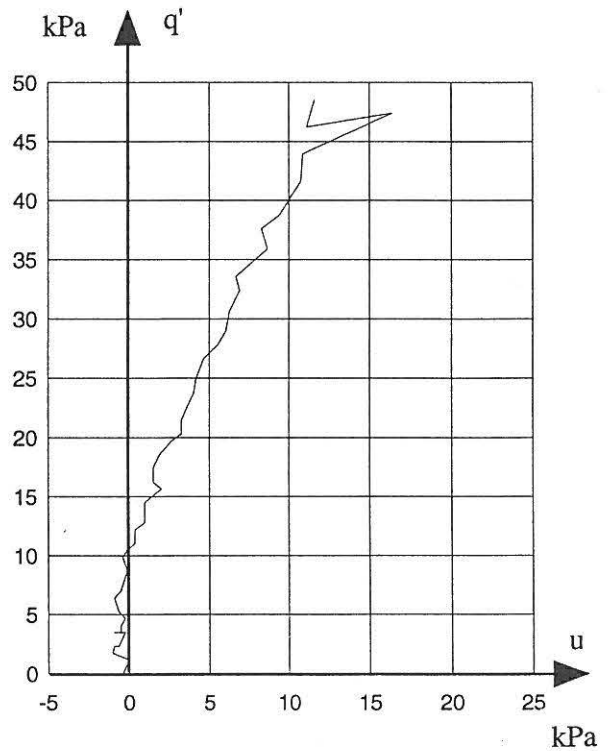
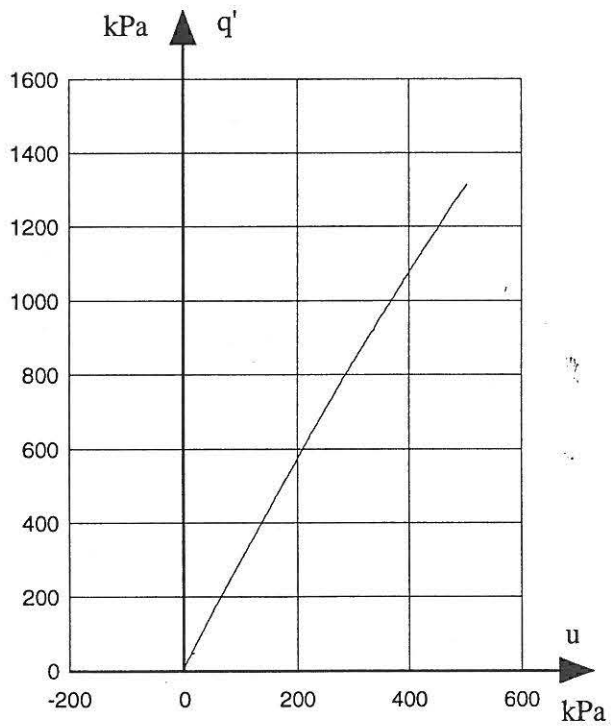
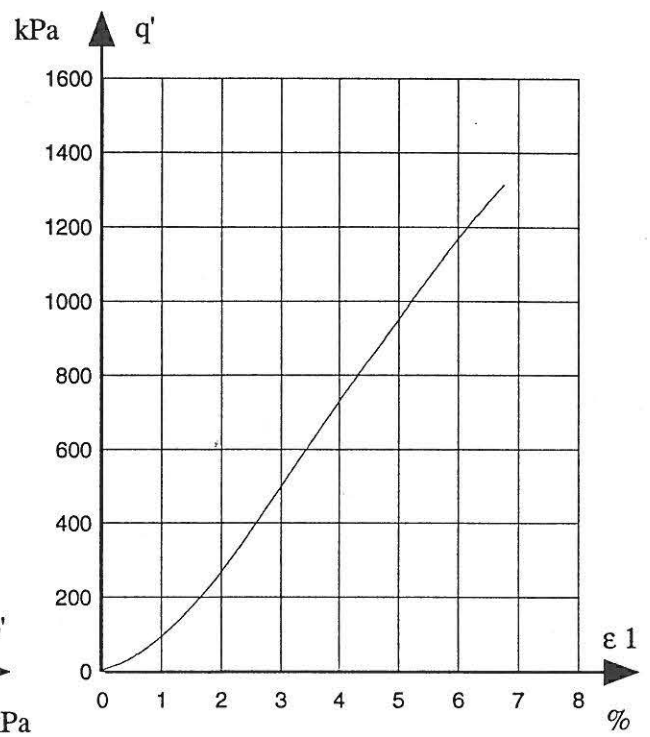
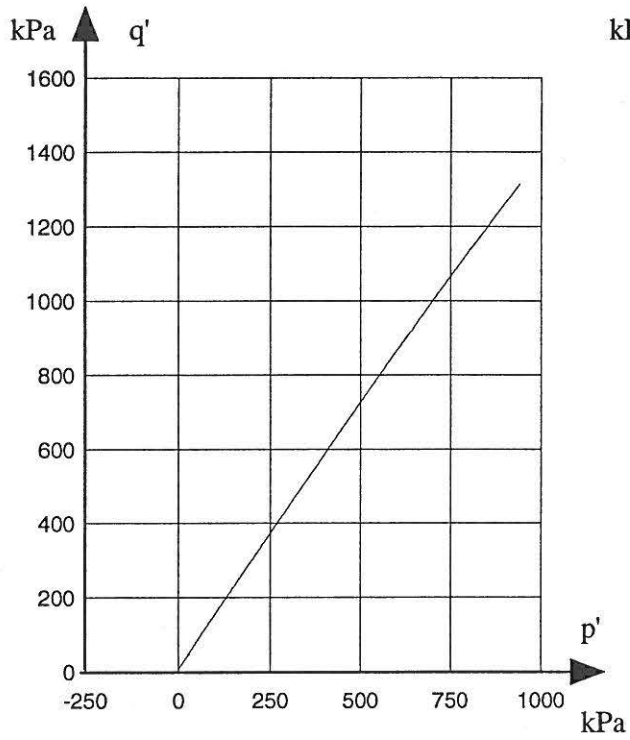
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	1 315.19	kPa	1.74	kPa
Mean normal stress	p'	941.17	kPa	-0.54	kPa
Confining pressures	σ_3	502.78	kPa	-1.13	kPa
Vertical strain	ϵ_1	6.76	%	0.01	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
-0.58	-0.32	0.00	0.00
1.74	-0.54	0.01	0.00
3.49	0.29	0.02	0.00
8.71	2.65	0.09	0.00
17.39	7.17	0.24	0.00
28.94	15.53	0.42	0.00
46.21	26.41	0.61	0.00
70.33	41.71	0.81	0.00
99.50	61.81	1.04	0.00
134.84	85.60	1.27	0.00
174.02	111.42	1.51	0.00
217.57	142.08	1.74	0.00
264.33	174.43	1.98	0.00
315.40	209.47	2.22	0.00
369.64	247.94	2.45	0.00
369.64	247.94	2.45	0.00
536.92	365.25	3.16	0.00
650.69	445.70	3.65	0.00
770.73	531.88	4.18	0.00
884.96	615.37	4.71	0.00
1007.28	707.56	5.24	0.00
1121.56	793.06	5.77	0.00
1230.12	875.29	6.31	0.00
1315.19	941.17	6.76	0.00

Job:	Encl. No
Lund No 0	5
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.058$

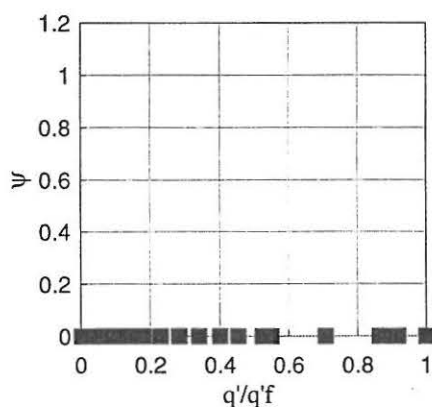
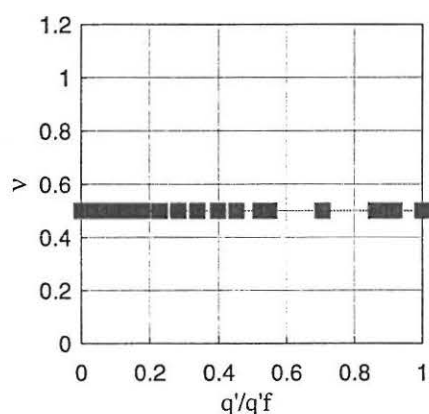


Job:	Encl. No
Lund No 0	6
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	24.7	
Calibration file	Date	Void ratio	0.617	
k2000_891	28.09.89	Saturation	1.06	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.	σ_3	100-5 kPa	
		ϵ_1	-0.104 %	
		ϵ_v	-0.374 %	
	2. Undrained compression.			
Deformation rate:		5.5 % ph		

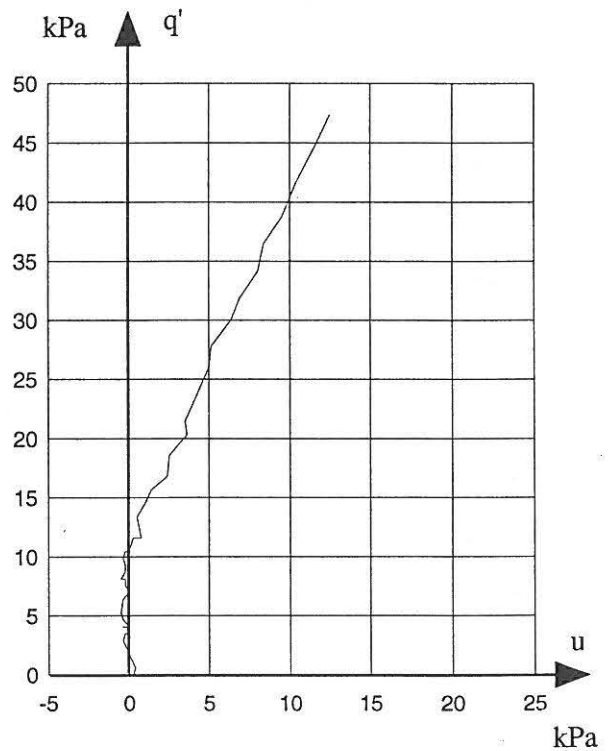
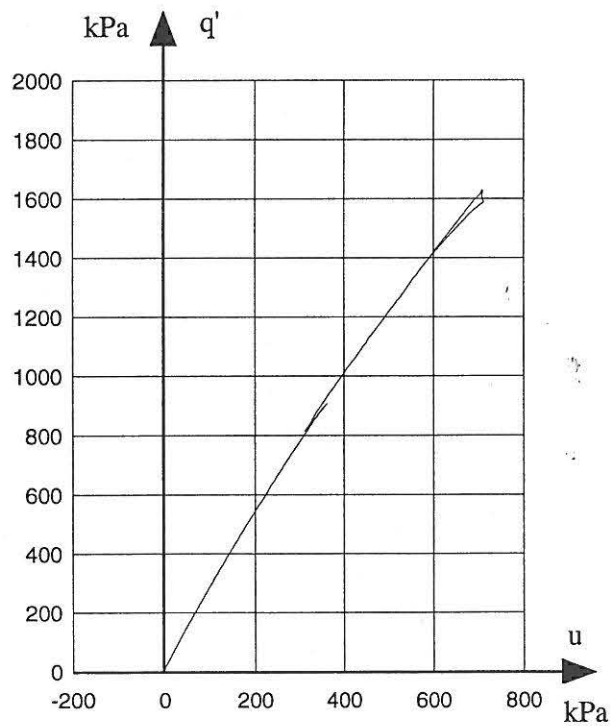
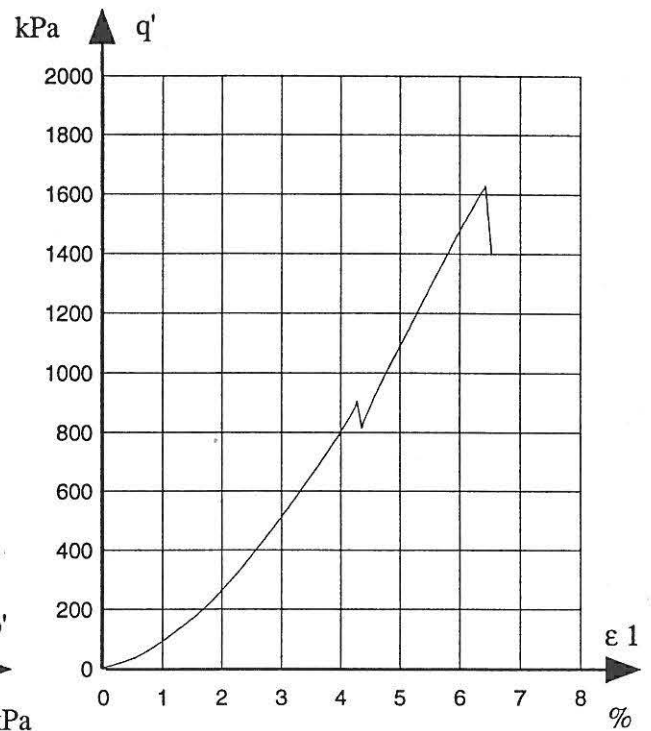
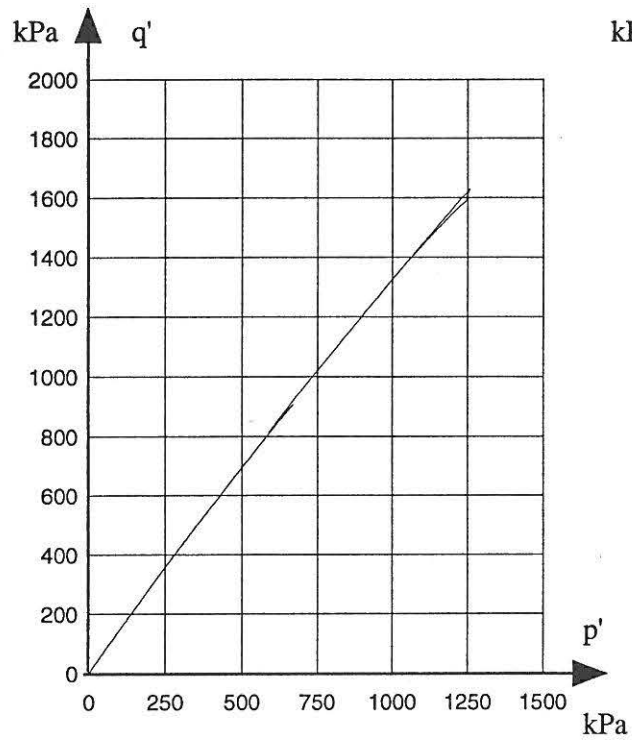
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	1 628.69	kPa	8.13	kPa
Mean normal stress	p'	1 256.09	kPa	6.96	kPa
Confining pressures	σ_3	713.20	kPa	4.25	kPa
Vertical strain	ϵ_1	6.43	%	0.09	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
-0.58	4.56	0.00	0.00
2.91	5.35	0.01	0.00
4.07	5.73	0.03	0.00
8.13	6.96	0.09	0.00
16.81	12.74	0.26	0.00
36.43	25.28	0.53	0.00
66.89	46.44	0.81	0.00
109.22	74.69	1.11	0.00
159.84	109.33	1.44	0.00
220.37	152.15	1.79	0.00
293.00	203.37	2.13	0.00
373.62	261.65	2.47	0.00
460.46	324.12	2.81	0.00
551.76	391.84	3.15	0.00
649.16	464.71	3.48	0.00
736.05	531.46	3.79	0.00
850.82	619.63	4.15	0.00
893.69	647.68	4.51	0.00
1151.75	855.92	5.16	0.00
1405.09	1066.22	5.81	0.00
1628.69	1256.09	6.43	0.00
1495.08	1150.01	6.48	0.00
1448.55	1106.73	6.50	0.00
1417.92	1078.88	6.51	0.00

Job:	Encl. No
Lund No 0	7
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.044$

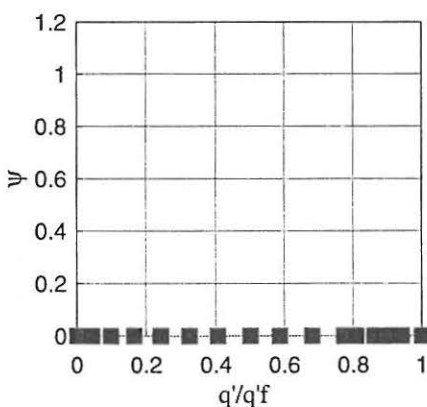
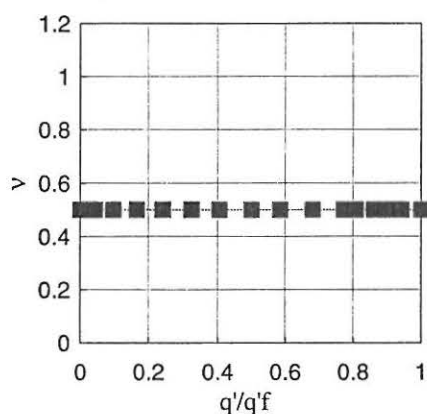


Job:	Encl. No
Lund No 0	8
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23	
Calibration file	Date	Void ratio	2.65	
kal90_01	11.12.90	Saturation	0.613	0.613
		Dimension H mm	1.01	
		D mm	71.5	
			69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	40-20 kPa
		ϵ_1	-0.087 %
		ϵ_v	0.000 %
	2. Undrained compression.		
Deformation rate:			3.1 % ph

		Maximum values	Minimum values for σ_3
Deviator stress	q'	2 311.31 kPa	25.31 kPa
Mean normal stress	p'	1 767.34 kPa	22.94 kPa
Confining pressures	σ_3	996.90 kPa	14.50 kPa
Vertical strain	ϵ_1	6.99 %	0.12 %
Volumetric strain	ϵ_v	0.00 %	0.00 %

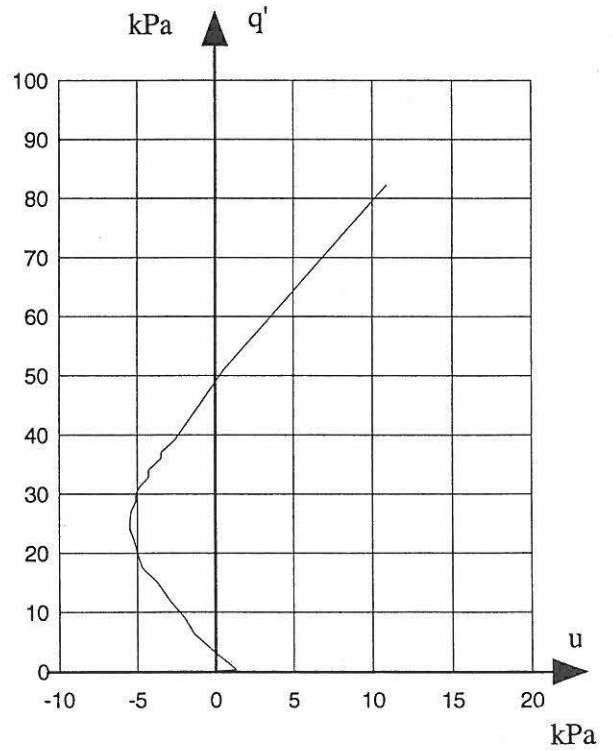
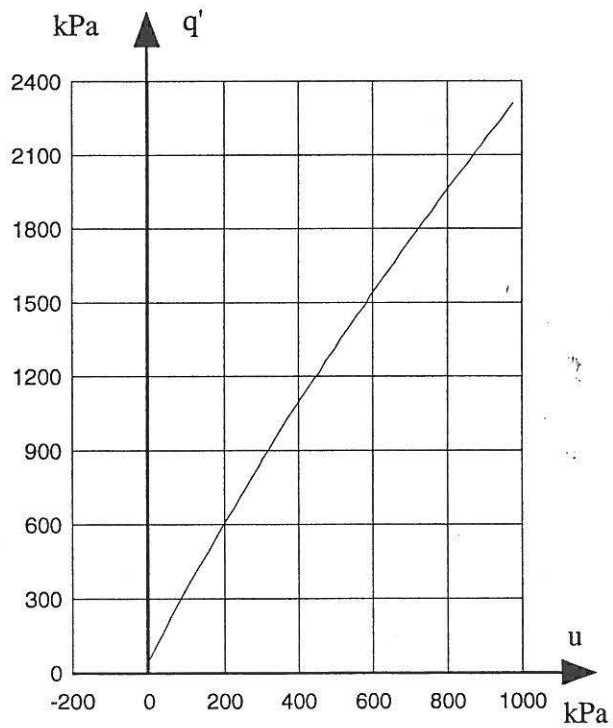
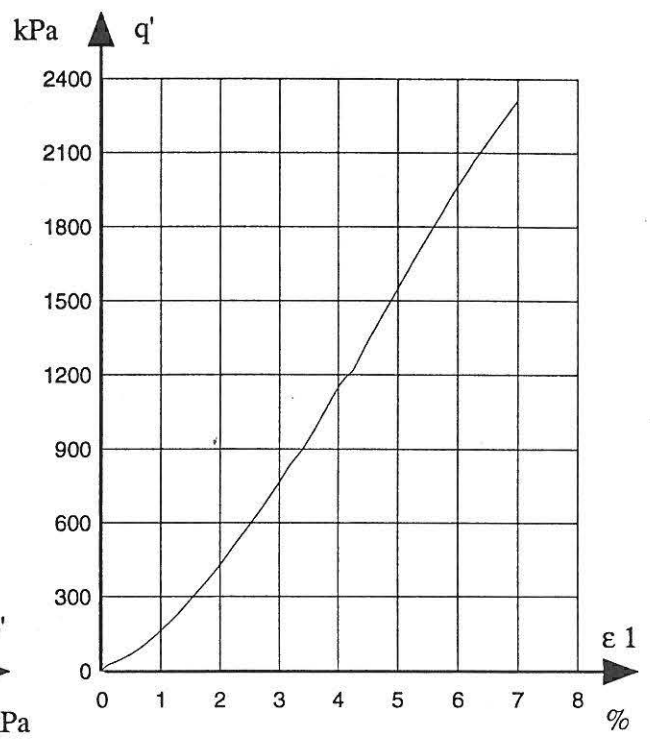
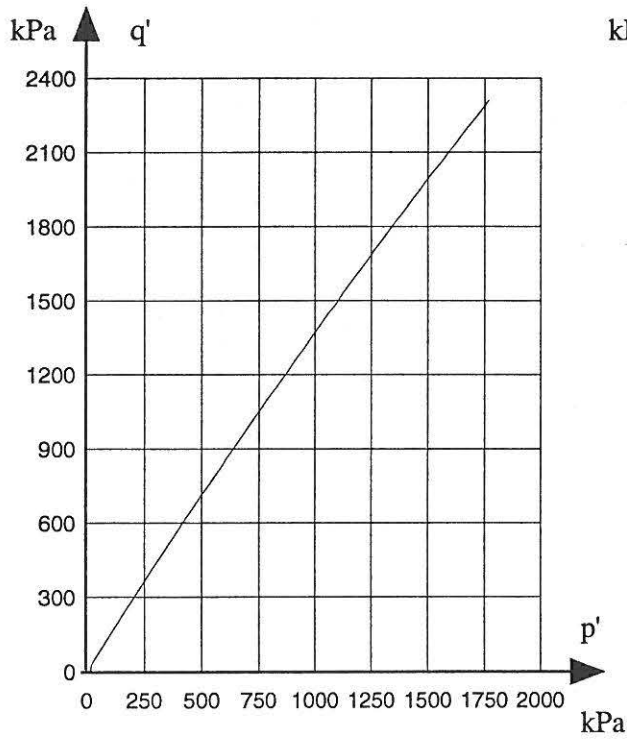


q'	p'	ϵ_1	ϵ_v
0.00	20.00	0.00	0.00
0.00	20.20	0.00	0.00
3.38	21.03	0.01	0.00
9.12	21.04	0.03	0.00
15.20	21.27	0.05	0.00
21.94	22.11	0.09	0.00
25.31	22.94	0.12	0.00
30.02	24.91	0.16	0.00
37.08	28.86	0.24	0.00
100.98	71.26	0.70	0.00
225.50	153.47	1.27	0.00
382.72	262.37	1.84	0.00
558.97	388.02	2.40	0.00
749.21	527.14	2.96	0.00
937.75	665.68	3.50	0.00
1155.21	832.17	4.01	0.00
1352.69	985.10	4.54	0.00
1573.21	1159.60	5.05	0.00
1786.55	1332.02	5.56	0.00
1867.39	1399.36	5.77	0.00
1990.46	1497.39	6.07	0.00
2066.87	1563.76	6.28	0.00
2176.20	1652.90	6.59	0.00
2311.31	1767.34	6.99	0.00

Job:	Encl. No
Lund No 0	9
Exc:	Check:
LBI, AMS	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1 = -0.110$
 Volumetric strain not measured during isotropic consolidation.

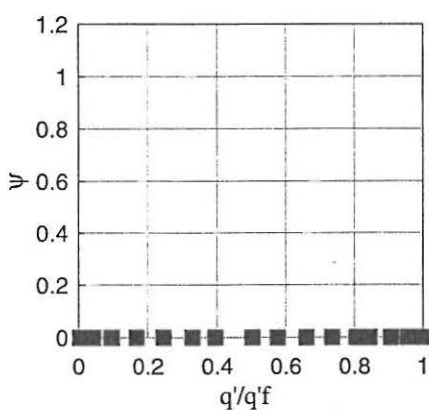
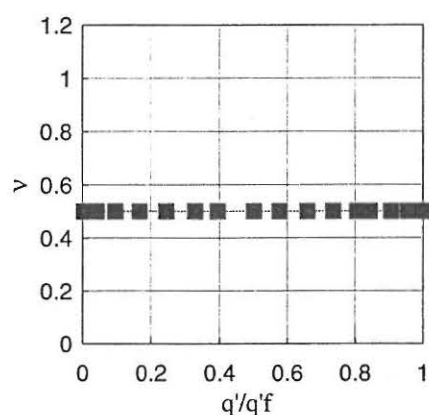


Job:	Encl. No
Lund No 0	10
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	24.6	Before test	At failure
		Grain density	2.65		
Calibration file	Date	Void ratio	0.610		
kal90_01	06.12.90	Saturation	1.07		
		Dimension H mm	71.5		
		D mm	69.7		

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.			
	1. Isotropic compression.		σ_3	80-20 kPa
			ϵ_1	-0.064 %
			ϵ_v	-0.173 %
	2. Undrained compression.		Deformation rate: 3.4 % ph	

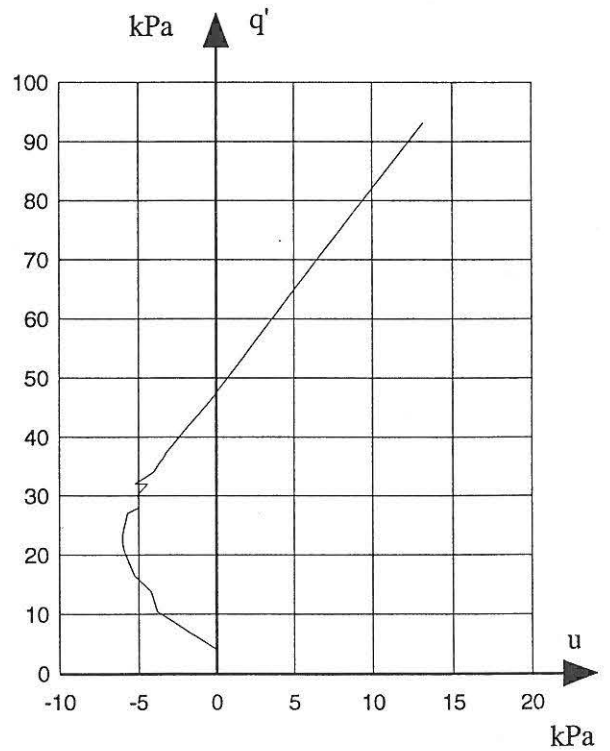
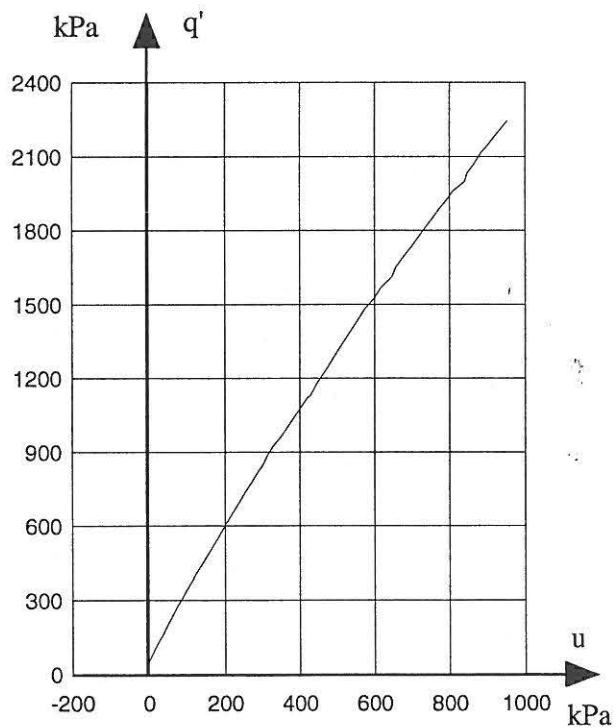
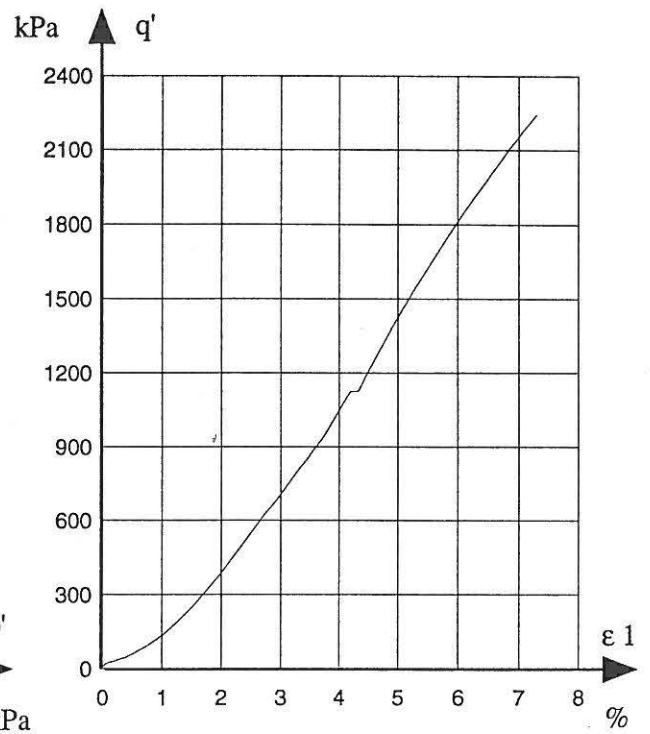
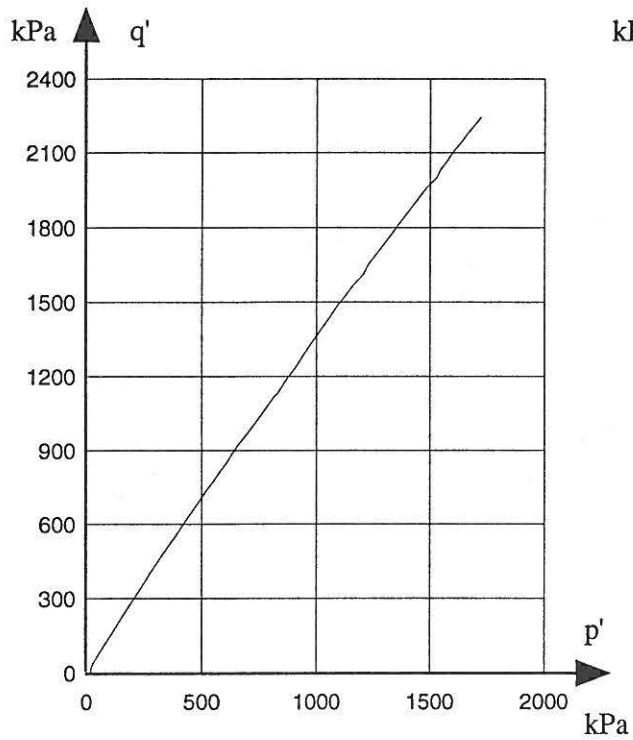
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 244.34	kPa	23.27	kPa
Mean normal stress	p'	1 722.01	kPa	23.26	kPa
Confining pressures	σ_3	973.90	kPa	15.50	kPa
Vertical strain	ϵ_1	7.29	%	0.10	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.34	21.61	0.00	0.00
0.34	21.61	0.00	0.00
10.46	21.19	0.02	0.00
16.54	21.71	0.05	0.00
20.58	22.46	0.07	0.00
23.27	23.26	0.10	0.00
26.97	24.79	0.15	0.00
29.99	26.40	0.19	0.00
36.37	30.22	0.27	0.00
93.16	65.75	0.75	0.00
212.82	145.74	1.36	0.00
374.74	258.31	1.96	0.00
552.05	386.32	2.51	0.00
738.49	521.46	3.10	0.00
886.09	629.56	3.56	0.00
1123.94	817.65	4.19	0.00
1292.42	946.01	4.69	0.00
1480.14	1088.58	5.14	0.00
1650.35	1228.22	5.57	0.00
1808.45	1358.72	5.99	0.00
1890.61	1425.30	6.22	0.00
2031.42	1544.04	6.63	0.00
2142.59	1635.20	6.96	0.00
2244.34	1722.01	7.29	0.00

Job:	Encl. No
Lund No 0	11
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.128$

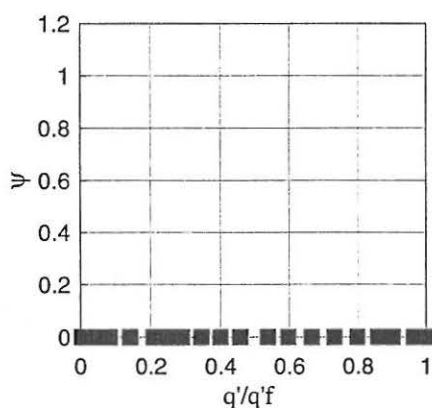
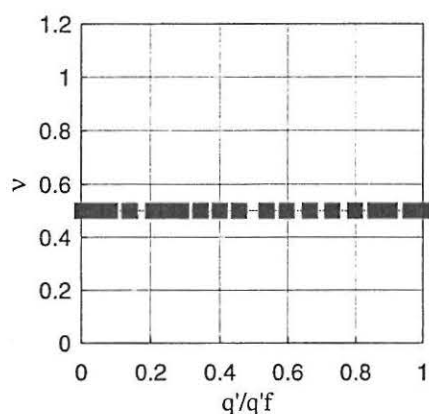


Job:	Encl. No
Lund No 0	12
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.3	
Calibration file	Date	Void ratio	0.611	
kal90_01	06.12.90	Saturation	1.01	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	80-500 kPa
		ϵ_1	0.225 %
		ϵ_v	0.671 %
	2. Undrained compression.		
	Deformation rate:		3.4 % ph

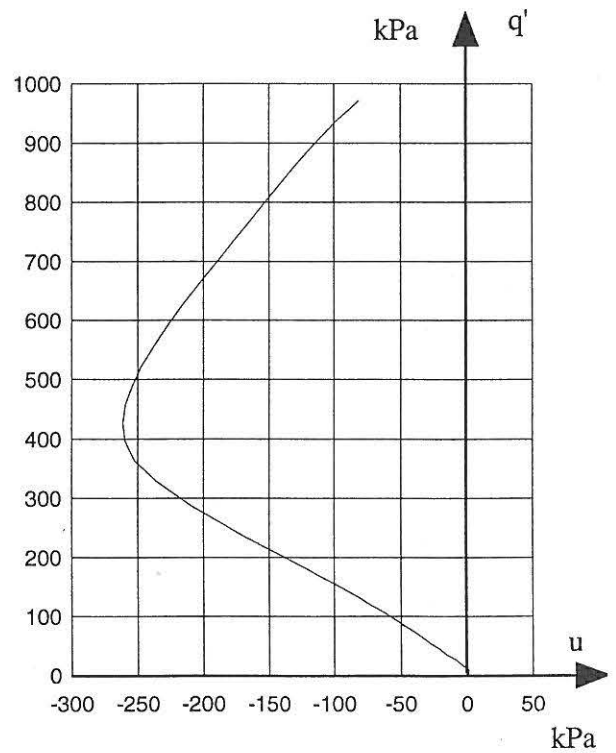
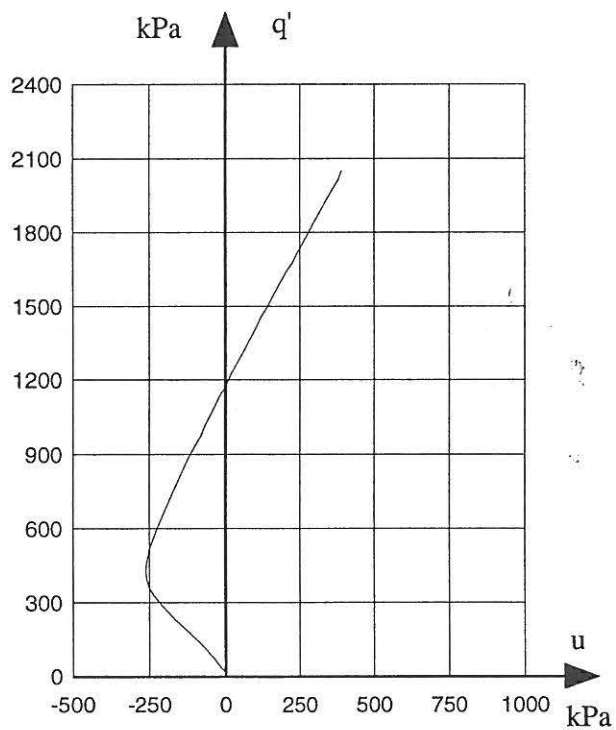
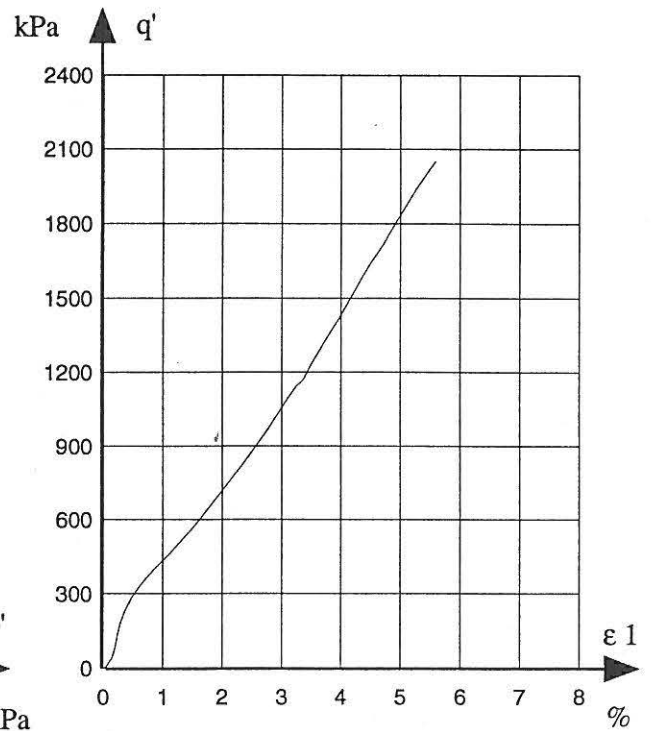
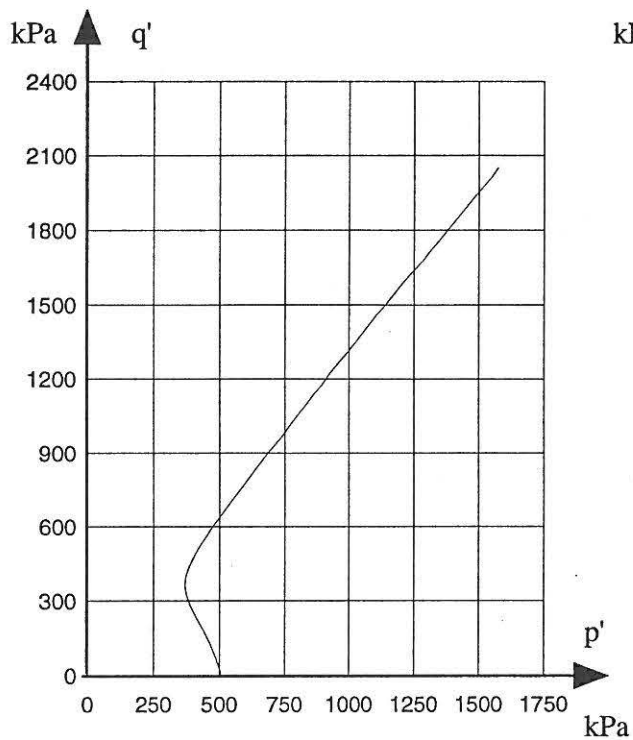
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 051.65	kPa	427.08	kPa
Mean normal stress	p'	1 575.18	kPa	381.36	kPa
Confining pressures	σ_3	891.30	kPa	239.00	kPa
Vertical strain	ϵ_1	5.59	%	0.98	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.00	500.40	0.00	0.00
7.79	504.60	0.06	0.00
15.58	502.89	0.08	0.00
25.06	500.95	0.11	0.00
54.16	490.75	0.17	0.00
71.41	485.80	0.19	0.00
105.90	473.50	0.22	0.00
168.05	445.22	0.28	0.00
286.48	387.19	0.49	0.00
427.08	381.36	0.98	0.00
522.50	427.37	1.35	0.00
592.26	471.02	1.59	0.00
705.70	548.33	1.96	0.00
819.65	628.12	2.33	0.00
934.97	713.06	2.66	0.00
1100.39	836.30	3.12	0.00
1222.66	925.25	3.48	0.00
1363.64	1036.15	3.83	0.00
1498.35	1139.65	4.16	0.00
1636.65	1247.65	4.49	0.00
1759.71	1348.87	4.82	0.00
1848.70	1418.23	5.04	0.00
1974.82	1518.37	5.37	0.00
2051.65	1575.18	5.59	0.00

Job:	Encl. No
Lund No 0	13
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.114$

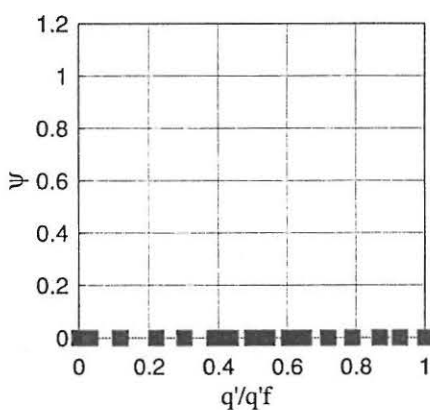
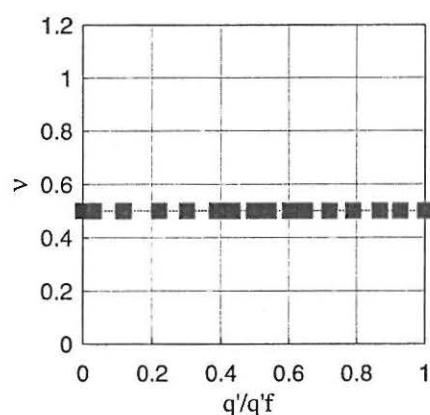


Job:	Encl. No
Lund No 0	14
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	23.2	
Calibration file	Date	Void ratio	0.610	0.611
kal90_01	08.01.91	Saturation	1.01	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	80-1000 kPa
		ϵ_1	-0.013 %
		ϵ_v	1.001 %
	2. Undrained compression.		
	Deformation rate:		3.1 % ph

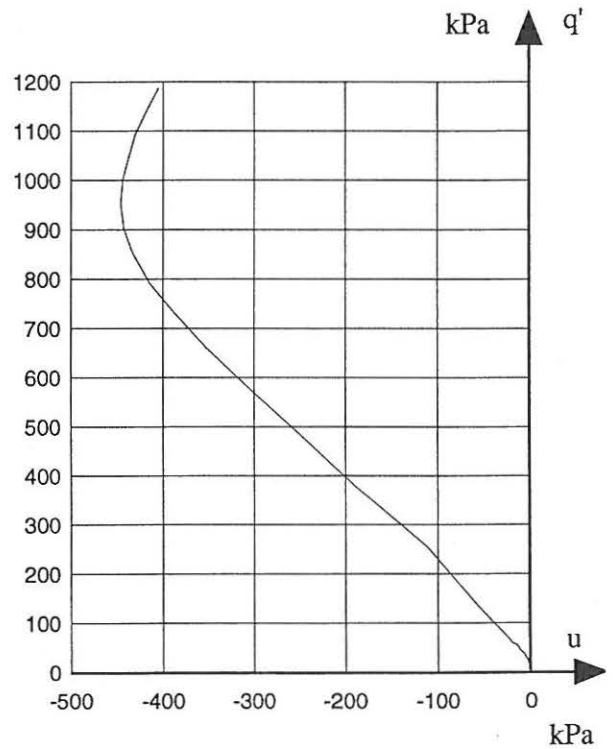
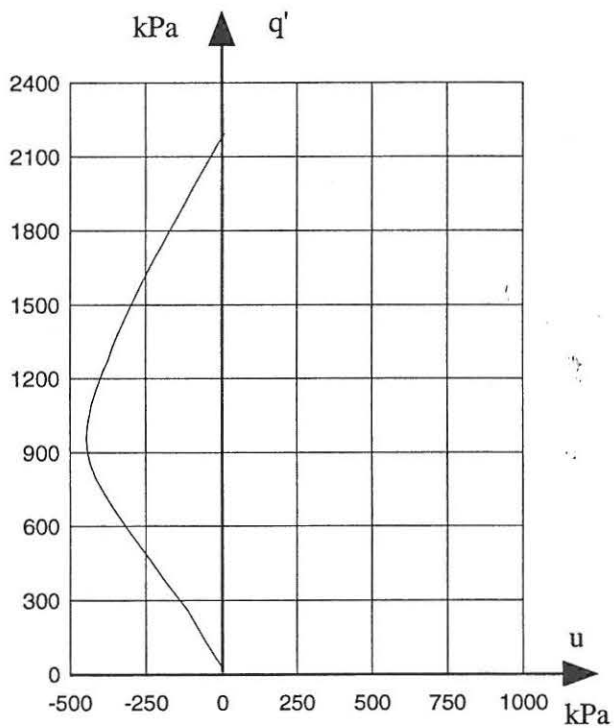
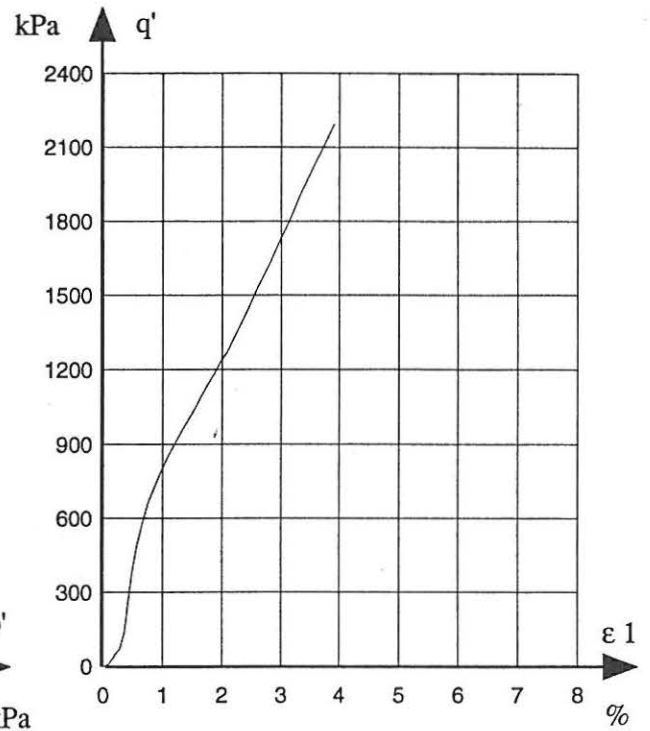
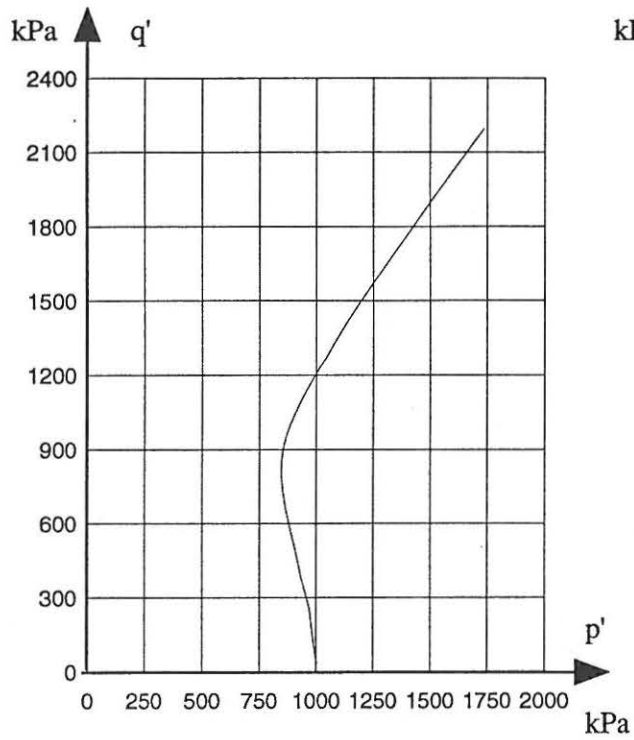
		Maximum values	Minimum values for σ_3
Deviator stress	q'	2 194.83 kPa	953.64 kPa
Mean normal stress	p'	1 735.91 kPa	868.08 kPa
Confining pressures	σ_3	1 004.30 kPa	550.20 kPa
Vertical strain	ϵ_1	3.91 %	1.33 %
Volumetric strain	ϵ_v	0.00 %	0.00 %



q'	p'	ϵ_1	ϵ_v
0.68	996.73	0.00	0.00
2.73	997.31	0.06	0.00
7.50	998.50	0.09	0.00
10.56	1000.12	0.11	0.00
20.09	1000.50	0.13	0.00
27.24	1001.28	0.16	0.00
36.76	1002.65	0.19	0.00
48.32	1000.21	0.22	0.00
55.79	1000.00	0.24	0.00
69.37	995.52	0.29	0.00
256.38	969.46	0.42	0.00
485.23	909.44	0.57	0.00
662.62	863.47	0.76	0.00
851.69	847.30	1.09	0.00
953.64	868.08	1.33	0.00
1092.39	930.03	1.65	0.00
1187.31	987.17	1.87	0.00
1322.94	1075.58	2.19	0.00
1421.99	1142.60	2.39	0.00
1579.72	1257.07	2.70	0.00
1732.01	1371.74	3.00	0.00
1905.34	1506.61	3.32	0.00
2033.45	1606.22	3.58	0.00
2194.83	1735.91	3.91	0.00

Job:	Encl. No
Lund No 0	15
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = 0.020$

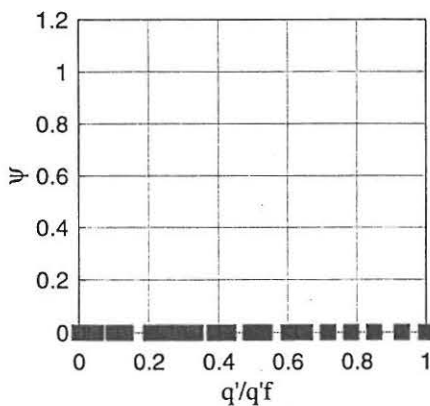
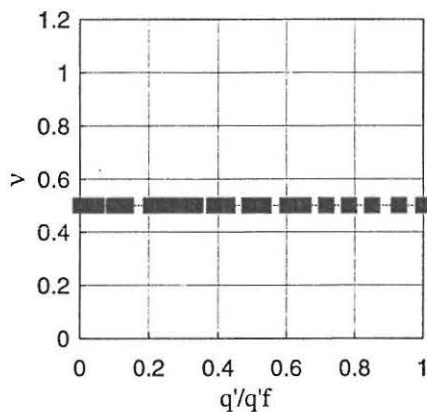


Job:	Encl. No
Lund No 0	16
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
Calibration file kal90_01	Date 09.01.91	Grain density	2.65	0.602
		Void ratio	0.602	
		Saturation		
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	80-100 kPa
		ϵ_1	0.069 %
		ϵ_v	-0.036 %
	2. Undrained compression.		
Deformation rate:			3.9 % ph

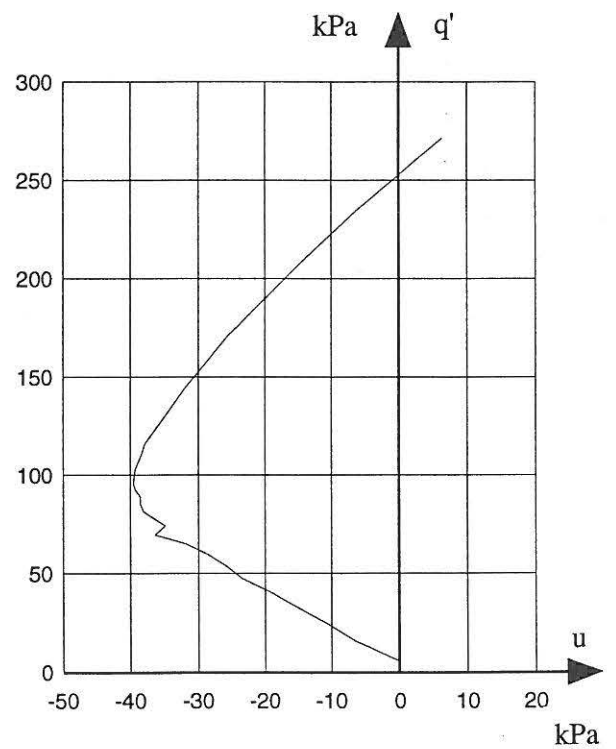
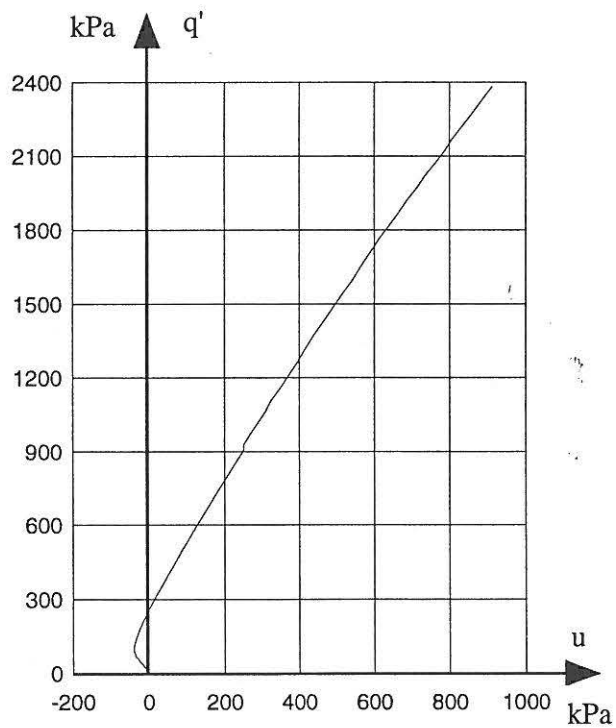
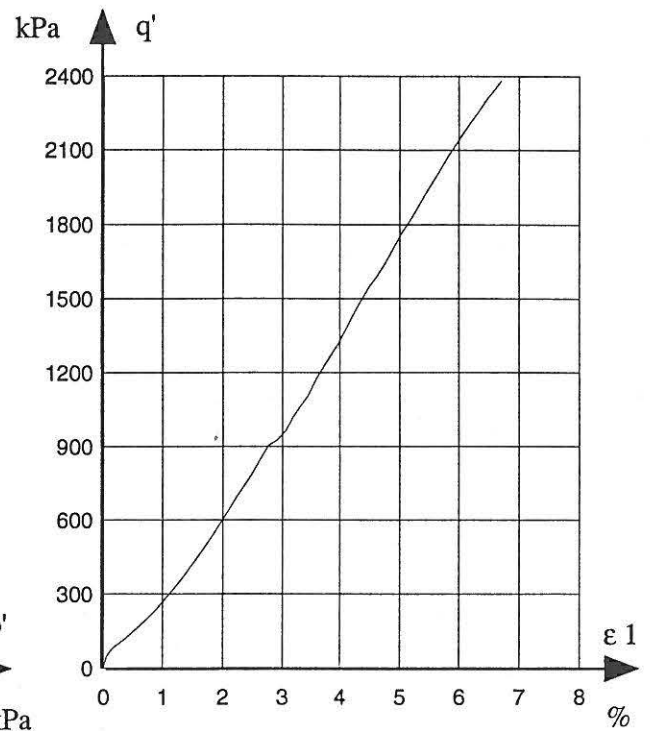
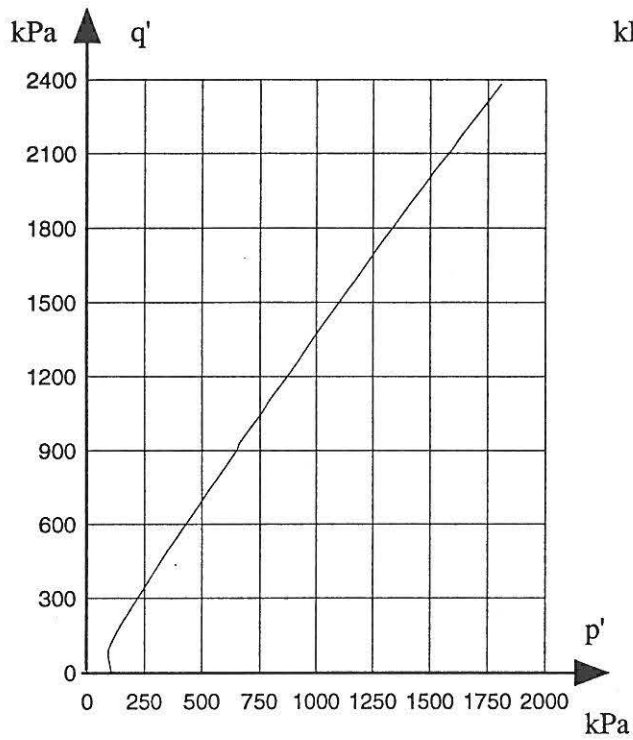
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 383.20	kPa	95.20	kPa
Mean normal stress	p'	1 809.30	kPa	94.43	kPa
Confining pressures	σ_3	1 014.90	kPa	62.70	kPa
Vertical strain	ϵ_1	6.71	%	0.24	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.34	102.41	0.00	0.00
5.39	104.10	0.01	0.00
24.61	99.80	0.03	0.00
53.92	94.37	0.07	0.00
78.12	91.64	0.15	0.00
95.20	94.43	0.24	0.00
112.26	101.52	0.34	0.00
234.64	174.21	0.88	0.00
313.97	228.26	1.16	0.00
490.92	352.04	1.70	0.00
588.14	421.85	1.96	0.00
693.99	499.43	2.23	0.00
794.99	573.10	2.50	0.00
924.86	659.59	2.92	0.00
1015.42	728.67	3.18	0.00
1169.30	845.87	3.57	0.00
1265.11	920.80	3.83	0.00
1433.35	1047.58	4.22	0.00
1545.66	1136.12	4.49	0.00
1705.47	1258.99	4.90	0.00
1864.57	1385.52	5.29	0.00
2024.04	1513.58	5.68	0.00
2213.25	1669.15	6.20	0.00
2383.20	1809.30	6.71	0.00

Job:	Encl. No
Lund No 0	17
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.003$
Saturation not measured.

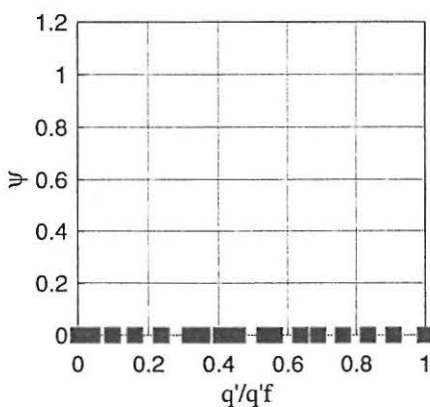
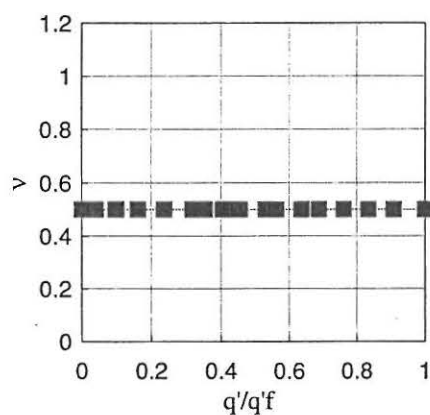


Job:	Encl. No
Lund No 0	18
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	20.9	
Calibration file	Date	Void ratio	0.553	0.553
kal90_01	28.02.91	Saturation	1.00	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	100-100 kPa
		ϵ_1	-0.154 %
		ϵ_v	0.000 %
	2. Undrained compression.		
	Deformation rate:		3.1 % ph

		Maximum values	Minimum values for σ_3
Deviator stress	q'	2 581.35 kPa	102.79 kPa
Mean normal stress	p'	1 897.75 kPa	95.96 kPa
Confining pressures	σ_3	1 037.30 kPa	61.70 kPa
Vertical strain	ϵ_1	6.42 %	0.64 %
Volumetric strain	ϵ_v	0.00 %	0.00 %

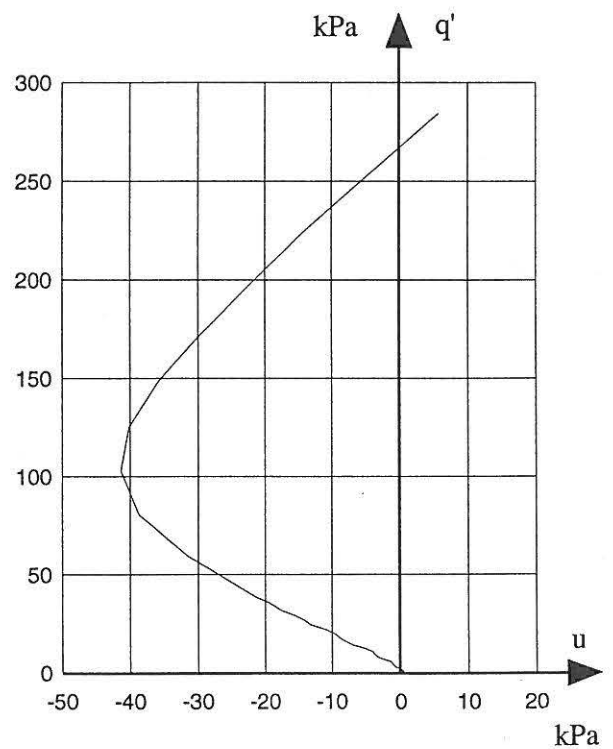
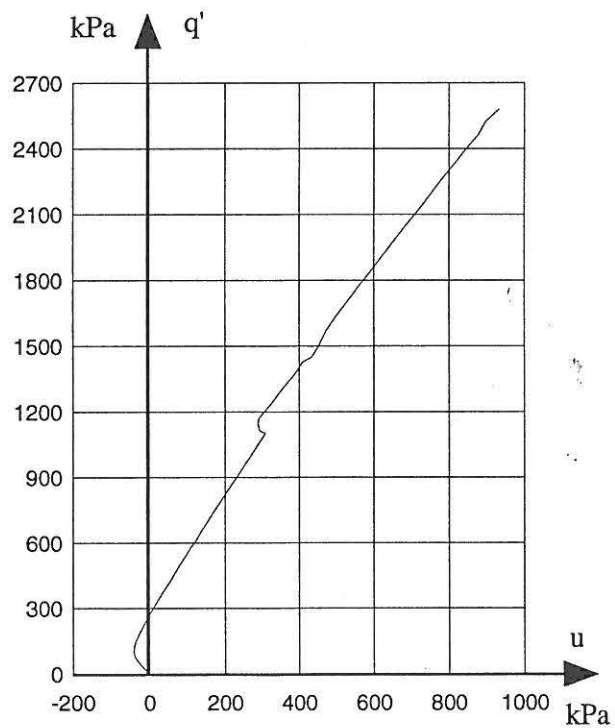
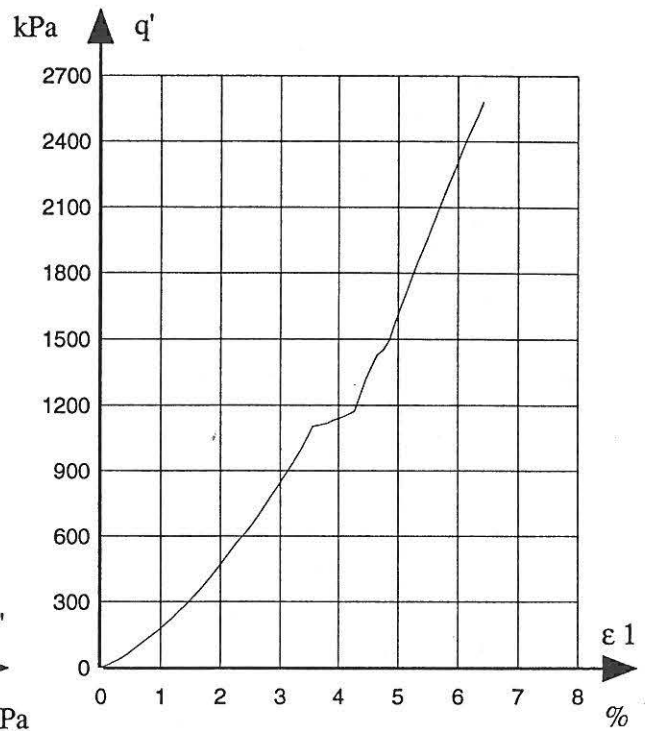
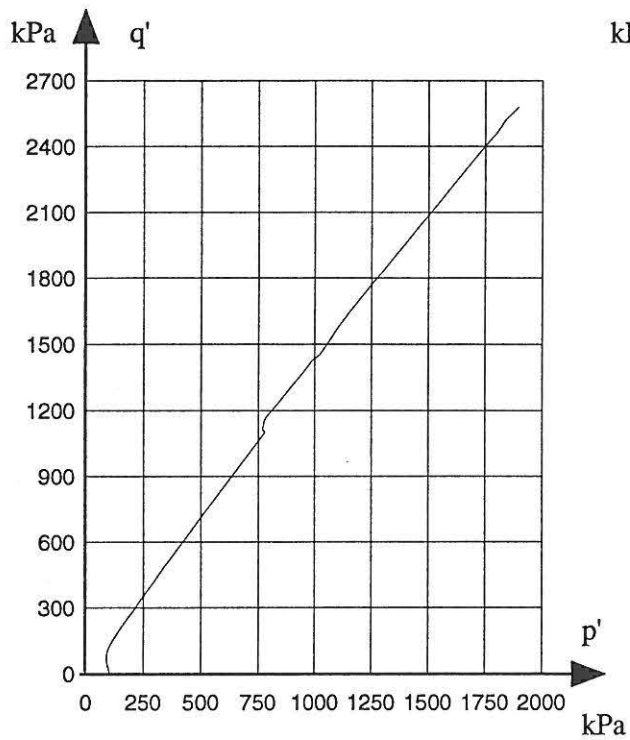


q'	p'	ϵ_1	ϵ_v
1.01	103.44	0.00	0.00
2.70	103.40	0.02	0.00
6.76	102.95	0.06	0.00
14.18	100.93	0.13	0.00
24.29	98.10	0.21	0.00
36.07	95.42	0.30	0.00
102.79	95.96	0.64	0.00
252.28	182.29	1.30	0.00
415.51	293.00	1.84	0.00
611.78	430.53	2.40	0.00
821.68	576.99	2.94	0.00
911.58	642.56	3.15	0.00
1049.81	740.84	3.45	0.00
1109.65	769.48	3.67	0.00
1141.83	771.81	4.03	0.00
1171.72	784.87	4.27	0.00
1369.67	946.26	4.55	0.00
1449.82	1020.77	4.75	0.00
1640.75	1150.92	5.04	0.00
1774.43	1254.28	5.22	0.00
1962.63	1402.81	5.49	0.00
2151.64	1553.21	5.77	0.00
2343.36	1703.82	6.05	0.00
2581.35	1897.75	6.42	0.00

Job:	Encl. No
Lund No 0	19
Exc:	Check:
LBI, AMS	FRJ

Remark:

Preparation [%] $\Delta \epsilon_1 = -0.019$
Volumetric strain not measured during isotropic consolidation.

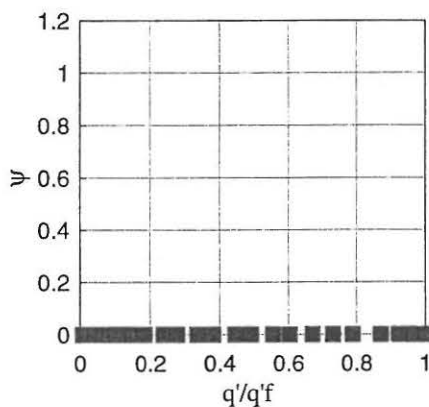
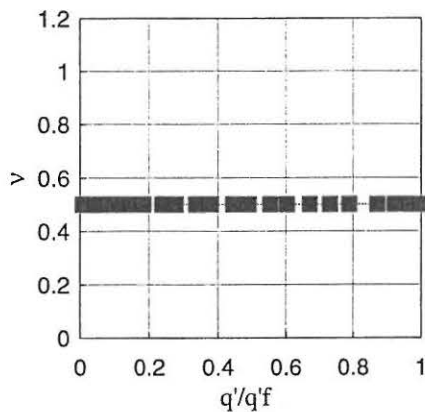


Job:	Encl. No
Lund No 0	20
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
Calibration file kal90_01	Date 02.03.91	Grain density	2.65	0.560
		Void ratio	0.560	
		Saturation		
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-250 kPa
		ϵ_1	0.153 %
		ϵ_v	0.264 %
	2. Undrained compression.		
Deformation rate:			3.0 % ph

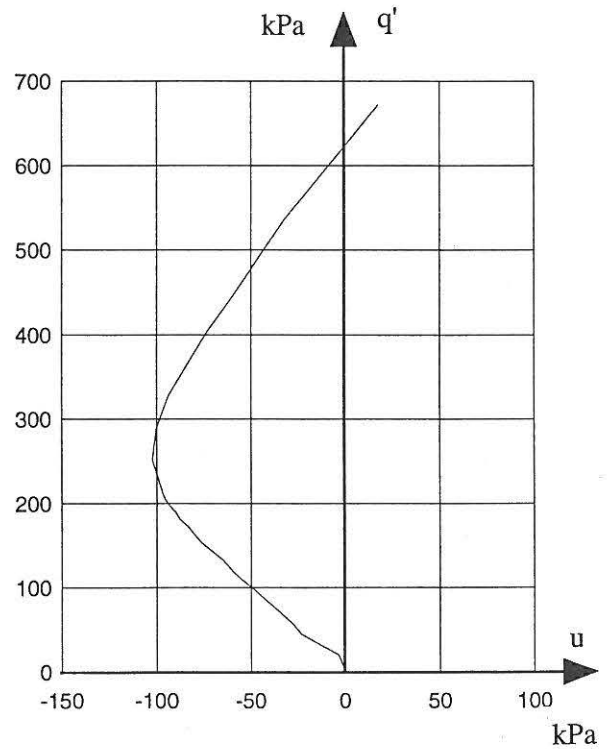
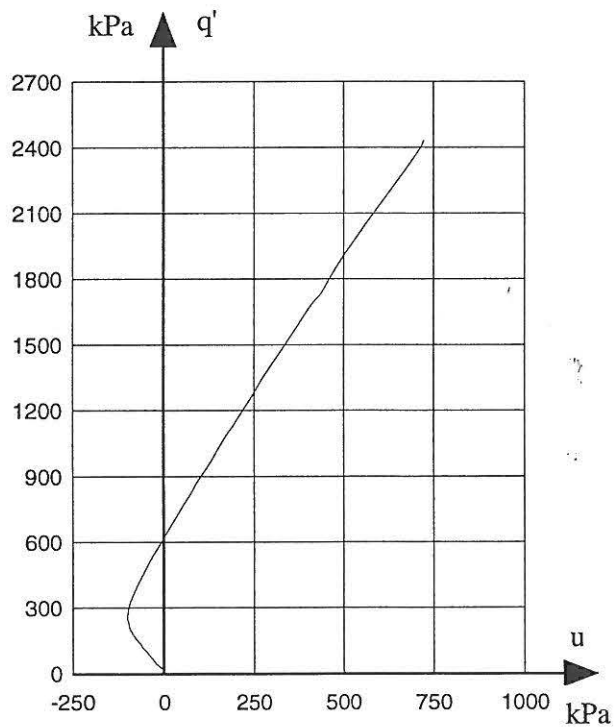
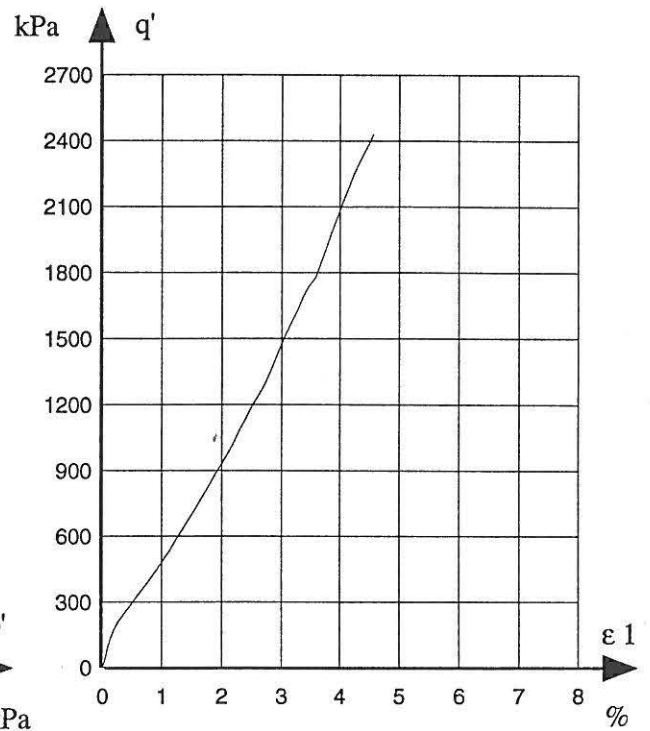
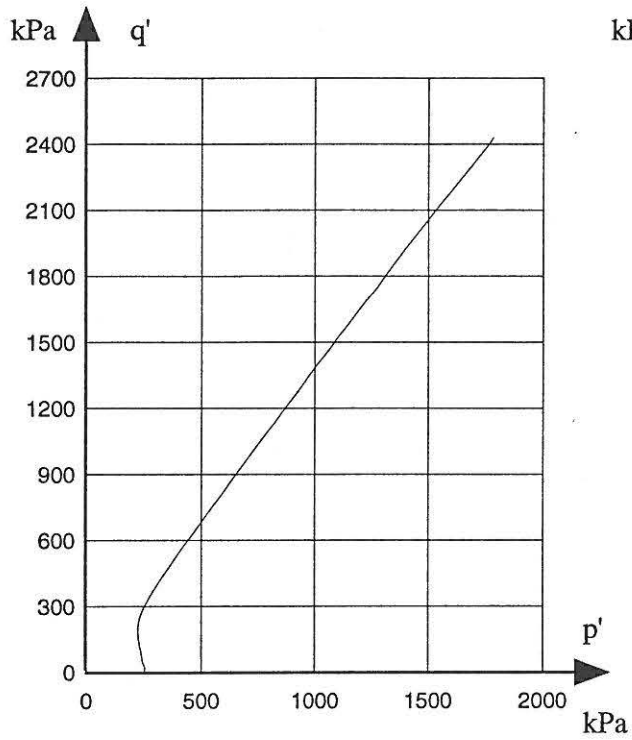
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 430.65	kPa	251.47	kPa
Mean normal stress	p'	1 783.62	kPa	231.42	kPa
Confining pressures	σ_3	973.40	kPa	147.60	kPa
Vertical strain	ϵ_1	4.56	%	0.38	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.00	250.10	0.00	0.00
20.27	253.66	0.03	0.00
45.26	242.09	0.06	0.00
109.37	231.46	0.12	0.00
163.60	224.03	0.19	0.00
204.93	222.91	0.26	0.00
251.47	231.42	0.38	0.00
365.09	287.90	0.70	0.00
446.67	339.59	0.92	0.00
581.39	428.90	1.24	0.00
672.13	492.04	1.44	0.00
818.04	594.38	1.76	0.00
921.53	667.08	1.98	0.00
1080.46	780.25	2.28	0.00
1187.91	859.17	2.49	0.00
1344.15	970.95	2.79	0.00
1463.86	1059.65	2.98	0.00
1628.51	1181.24	3.27	0.00
1775.77	1294.62	3.58	0.00
1914.80	1392.67	3.76	0.00
2114.48	1544.93	4.04	0.00
2238.97	1642.02	4.22	0.00
2348.98	1727.09	4.42	0.00
2430.65	1783.62	4.56	0.00

Job:	Encl. No
Lund No 0	21
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = -0.010$
Error during saturation.

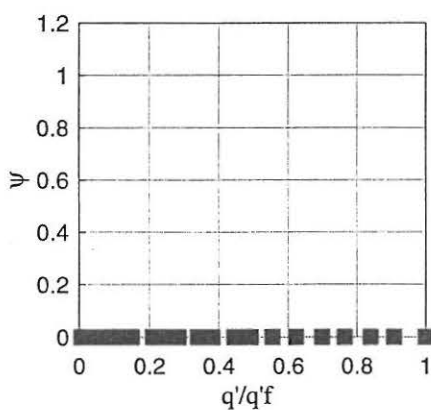
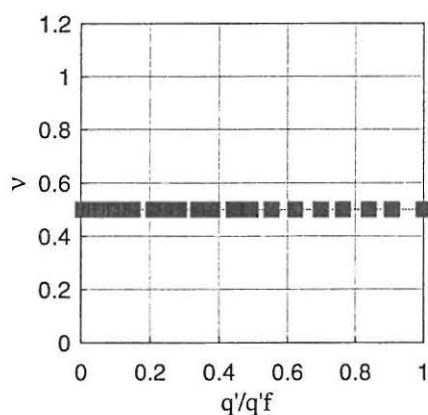


Job:	Encl. No
Lund No 0	22
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	21.2	At failure 0.553
		Grain density	2.65	
Calibration file	Date	Void ratio	0.553	
kal90_01	04.03.91	Saturation	1.02	
		Dimension H mm	71.5	
		D mm	69.7	

TEST-PROGRAM CU - Triaxial test. free ends	Drained compression.		
	1. Isotropic compression.	σ_3	100-500 kPa
		ϵ_1	0.132 %
		ϵ_v	0.508 %
	2. Undrained compression.	Deformation rate: 3.0 % ph	

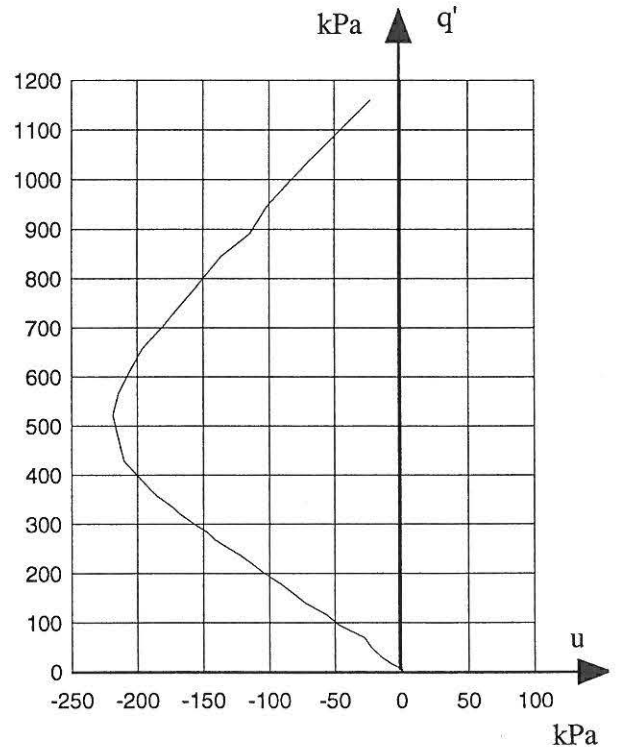
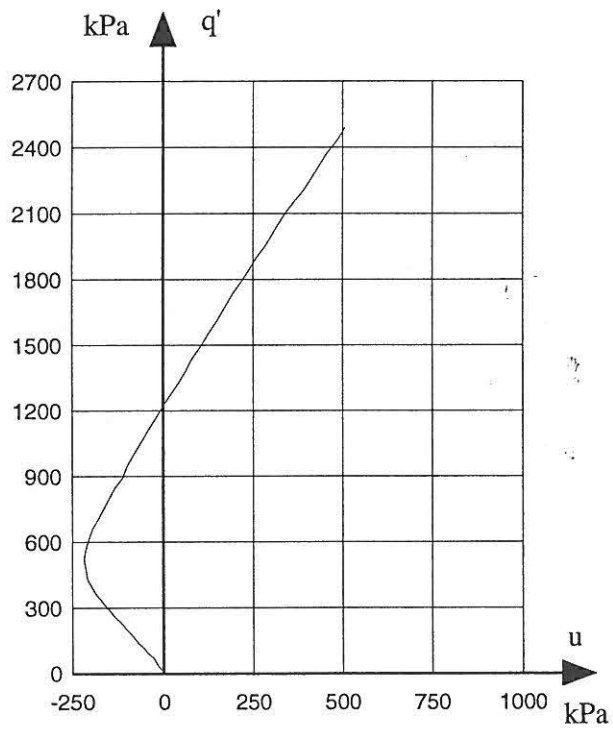
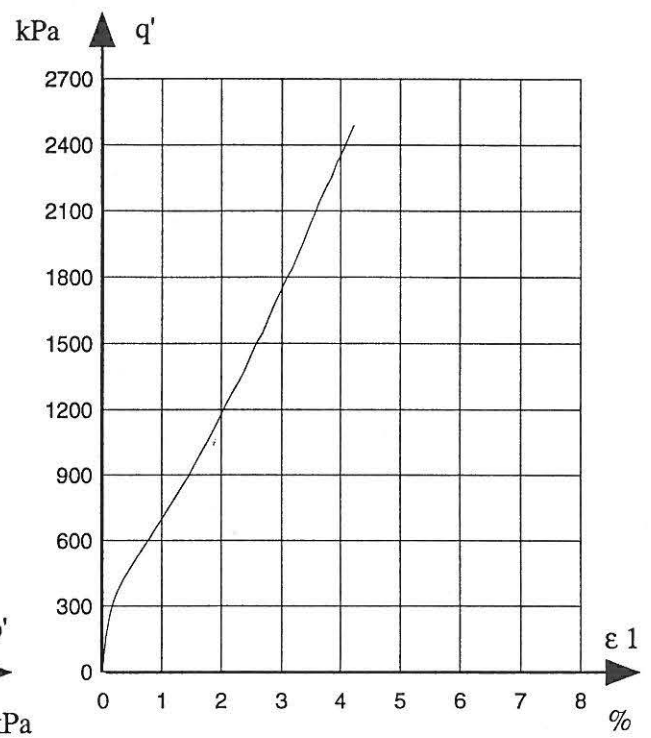
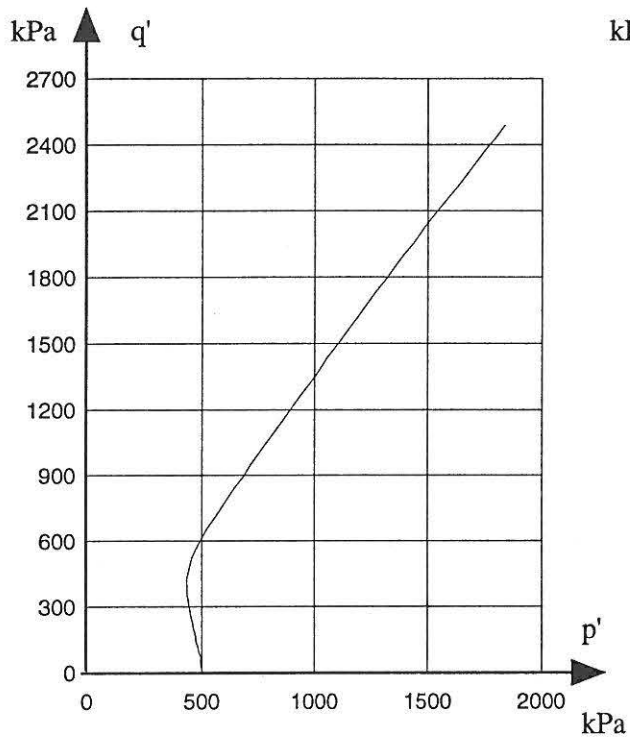
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 487.34	kPa	521.80	kPa
Mean normal stress	p'	1 837.01	kPa	455.93	kPa
Confining pressures	σ_3	1 007.90	kPa	282.00	kPa
Vertical strain	ϵ_1	4.22	%	0.58	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
-0.34	500.49	0.00	0.00
16.60	498.23	0.00	0.00
69.79	495.66	0.02	0.00
116.52	483.04	0.03	0.00
160.86	472.22	0.05	0.00
219.04	460.61	0.09	0.00
253.50	452.30	0.11	0.00
298.39	443.56	0.16	0.00
324.34	439.71	0.19	0.00
368.74	434.11	0.25	0.00
521.80	455.93	0.58	0.00
611.09	498.40	0.80	0.00
707.31	557.57	1.02	0.00
846.12	646.64	1.34	0.00
945.38	713.73	1.55	0.00
1102.68	823.76	1.86	0.00
1219.47	904.29	2.07	0.00
1375.71	1017.57	2.38	0.00
1547.77	1139.52	2.68	0.00
1736.37	1273.69	2.98	0.00
1899.32	1393.61	3.27	0.00
2087.19	1531.53	3.55	0.00
2255.68	1664.39	3.85	0.00
2487.34	1837.01	4.22	0.00

Job:	Encl. No
Lund No 0	23
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = 0.051$

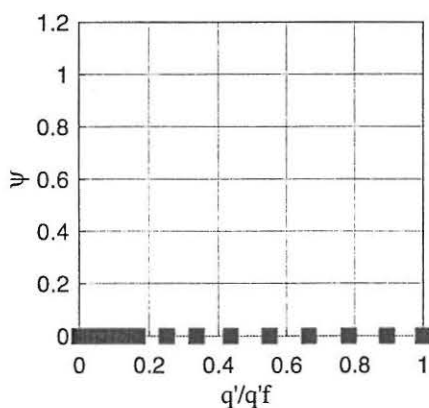
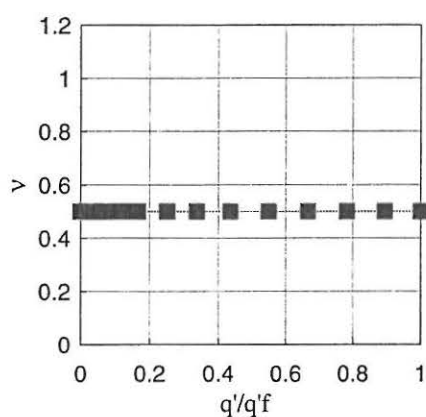


Job:	Encl. No
Lund No 0	24
Exc:	Check:
LBI, AMS	FRJ

Description of soil Lund No 0		Water content %	Before test	At failure
		Grain density	21	
Calibration file	Date	Void ratio	2.65	
kal90_01	10.01.90	Saturation	0.561	0.561
		Dimension H mm	1.01	
		D mm	71.5	
			69.7	

TEST-PROGRAM	Drained compression.		
CU - Triaxial test. free ends	1. Isotropic compression.	σ_3	20 kPa
		ϵ_1	%
		ϵ_v	%
	2. Undrained compression.		
	Deformation rate:		3.1 % ph

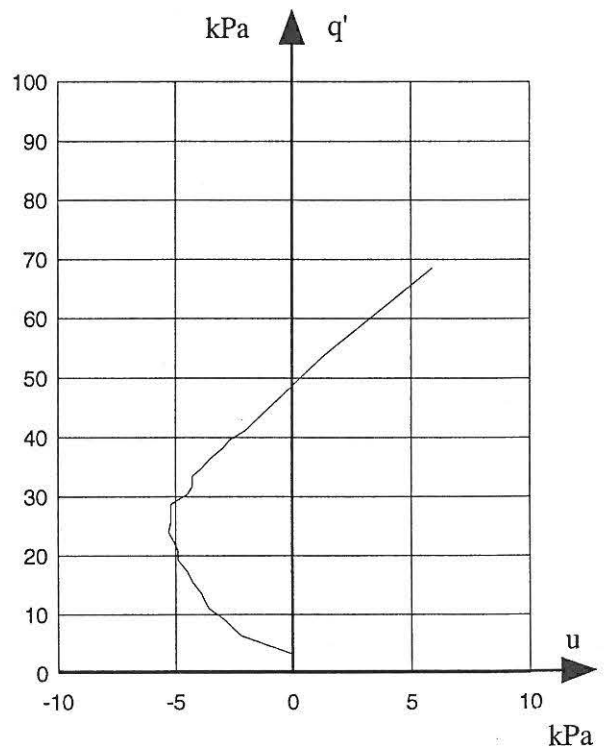
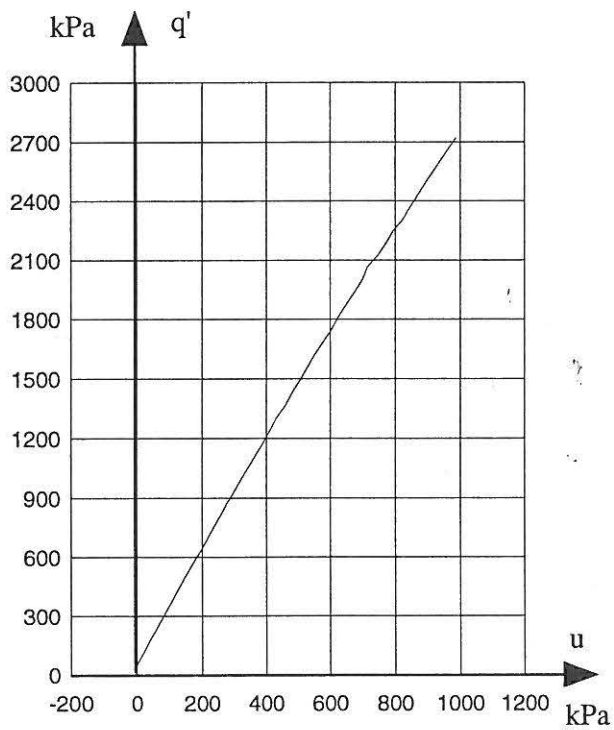
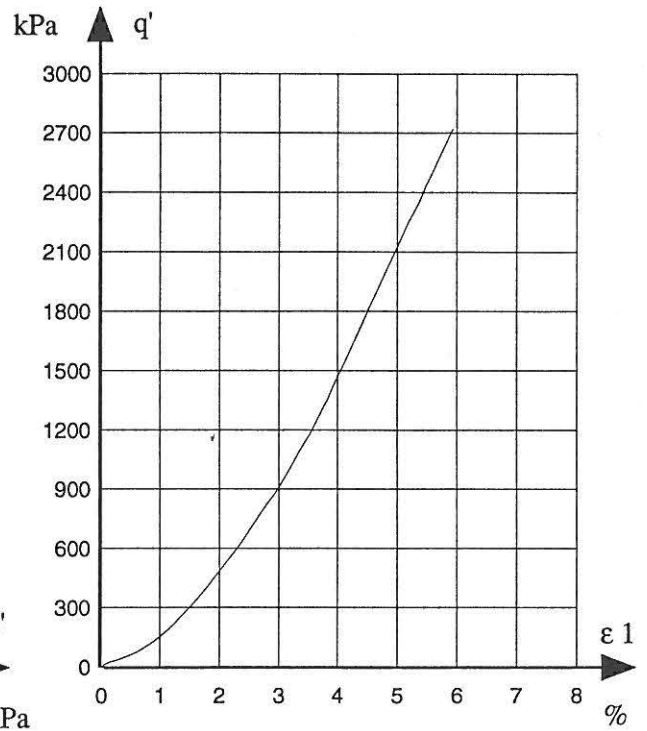
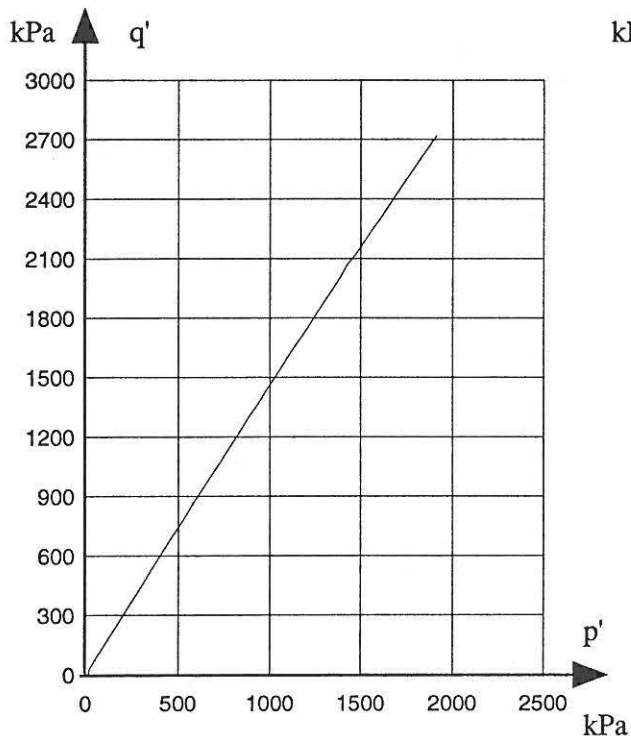
		Maximum values		Minimum values for σ_3	
Deviator stress	q'	2 720.00	kPa	23.97	kPa
Mean normal stress	p'	1 913.67	kPa	22.69	kPa
Confining pressures	σ_3	1 007.00	kPa	14.70	kPa
Vertical strain	ϵ_1	5.93	%	0.15	%
Volumetric strain	ϵ_v	0.00	%	0.00	%



q'	p'	ϵ_1	ϵ_v
0.68	20.23	0.02	0.00
3.30	21.10	0.03	0.00
11.14	20.11	0.06	0.00
15.53	20.88	0.08	0.00
19.24	21.51	0.11	0.00
23.97	22.69	0.15	0.00
27.34	23.91	0.18	0.00
31.73	26.28	0.23	0.00
34.77	27.69	0.27	0.00
41.18	31.73	0.33	0.00
68.53	48.74	0.56	0.00
109.70	75.87	0.81	0.00
159.70	108.33	1.03	0.00
255.20	171.97	1.36	0.00
326.40	219.40	1.58	0.00
451.70	302.97	1.92	0.00
683.90	460.77	2.49	0.00
917.20	620.03	3.03	0.00
1184.00	806.17	3.54	0.00
1494.00	1023.50	4.04	0.00
1811.00	1248.37	4.52	0.00
2128.00	1478.93	5.01	0.00
2432.00	1700.77	5.48	0.00
2720.00	1913.67	5.93	0.00

Job:	Encl. No
Lund No 0	25
Exc:	Check:
LBI, AMS	FRJ

Remark:
Preparation [%] $\Delta \epsilon_1 = 0.000$



Job:	Encl. No
Lund No 0	26
Exc:	Check:
LBI, AMS	FRJ

